and a	CITY OF BRIDGEPORT File No
SEAL OF	PLANNING & ZONING COMMISSION APPLICATION
1.	NAME OF APPLICANT: Dereck Pettway and Diverse Builders, LLC
2.	Is the Applicant's name Trustee of Record? Yes No X
	If yes, a sworn statement disclosing the Beneficiary shall accompany this application upon filing.
3.	Address of Property: 83 & 87 Primrose Avenue and 536 Peet Street / CT / 06606
	(number) (street) (state) (zip code)
4.	Assessor's Map Information: Block No. 73/2367 Lot No. 1, 7/A & 12
5.	Amendments to Zoning Regulations: (indicate) Article: <u>N/A</u> Section:
	(Attach copies of Amendment)
6.	Description of Property (Metes & Bounds): 80.00' x 100.00' x 120.00' x 140.00' x 200.00' x 240.00'. See submitted survey.
7.	Existing Zone Classification: <u>R-C</u>
	Zone Classification requested: N/A
9.	Describe Proposed Development of Property: Proposed construction of a three-family dwelling
	in connection with existing multi-family development at 536 Peet Street with associated Site improvements.
	Approval(s) requested: Special Permit and Site Plan Review
	Approvalo) requeeted.
	Signature: Date: 12/29/2021
	Print Name:
	If signed by Agent, state capacity (Lawyer, Developer, etc.) Signature:
	Print Name:
	Mailing Address: Chris Russo, Russo & Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06890
	Phone: 203-528-0590 Cell: 203-520-4603 Fax: 203-255-6618
	E-mail Address: Chris@russorizio.com
	\$Fee received Date: Clerk:
	THIS APPLICATION MUST BE SUBMITTED IN PERSON AND WITH COMPLETED CHECKLIST
	Completed & Signed Application Form     A-2 Site Survey     Building Floor Plans
	Completed Site / Landscape Plan
	Written Statement of Development and Use Property Owner's List Fee
	Cert. of Incorporation & Organization and First Report (Corporations & LLC's)
	PROPERTY OWNER'S ENDORSEMENT OF APPLICATION
	The Estate of Helen Pothanszky 12/29/2021
	Print Owner's Name Owner's Signature Date
	Diverse Builders, LLC     12/29/2021       Print Owner's Name     Owner's Signature     Date
	Print Owner's Name Owner's Signature Date

Rev. 6/18/2016



Colin B. Connor Robert G. Golger David K. Kurata Katherine M. Macol Leah M. Parisi William M. Petroccio\* Raymond Rizio\* Christopher B. Russo Robert D. Russo John J. Ryan Vanessa R. Wambolt (\*Also Admitted in NY)

December 23, 2021

Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, CT 06604 **HAND-DELIVERED** 

### Re: Petition for Special Permit and Site Plan Review – 83 & 87 Primrose Avenue and 536 Peet Street

Dear Mr. Buckley:

Please accept the following narrative and enclosed application materials as part of an application for the properties located at 83 & 87 Primrose Avenue (the "Site") for a Special Permit and Site Plan Review approval to construct a three-family dwelling with associated site improvements in the R-C Zone in connection with a previously approved multi-family development at adjacent 536 Peet Street.

### Narrative

The Petitioner requests a Special Permit and Site Plan Review under Sections 14-4 and 14-2 of the Zoning Regulations of the City of Bridgeport (the "Regulations") to construct a three-family dwelling on the Site. The Site is located on two (2) public street frontages –Woodmont Avenue and Primrose Avenue - in the R-C Zone. This project is attached to the recent development that was approved at 536 Peet Street. In combination with 536 Peet Street, the lot area consists of Thirty-six thousand square feet (36,000 SF), which is significantly oversized for the R-C Zone. The Petition is fully conforming to the Regulations and requires no variances.

83 Primrose Avenue currently contains a single-family dwelling. 87 Primrose Avenue is currently a vacant and buildable lot. The Petitioner proposes to significantly enhance the Site with landscaping, conforming parking and new sidewalks. Access to the Site is proposed through a single driveway on Primrose Avenue. The property known as 536 Peet Street was already approved for a tremendous development of Ten (10) residential dwelling units divided into a six (6) dwelling unit building located along Peet Street, a two (2) dwelling unit building along Glendale Avenue and another two (2) dwelling unit building along Woodmont Avenue. On the properties known as 83 & 87 Primrose Avenue, the Petitioner now proposes a single building containing Three (3) dwelling units. Two (2) entrances out to the sidewalk will be installed to Woodmont Avenue to match the design of 536 Peet Street. A parking area for Two (2) parking spaces will be created with each dwelling unit having an additional parking space, so the Petition will be completely conforming as to

> 10 Sasco Hill Road Fairfield, CT 06824

Tel 203-255-9928 Fax 203-255-6618 off-street parking. The entire street frontage of the Site will be surrounded by perimeter landscaping, which is also in conformity with the Regulations. The first floor of each dwelling unit will contain the single-car garage, a kitchen, living room and full bath. The second floor will contain three (3) bedrooms, full bath, closets, and washer/dryer.

### Special Permit and Site Plan Review

The Petition satisfies all Special Permit and Site Plan Review standards under Section 14-4 and 14-2 of the Regulations as the proposed improvements will develop a currently vacant and overgrown property and a dated single-family dwelling with a proposed three-family dwelling. The proposed use is in conformity with the neighborhood and the Regulations with condominiums located across Peet Street and Glendale Avenue and just down at the end of Marconi Avenue.

The Petition satisfies the intent of the Regulations and Master Plan of Conservation and Development by developing a vacant and overgrown vacant lot and creating new housing stock to an area that has an extensive aging housing stock, one of which is being replaced with this Petition. It will not impair the future development of the surrounding area, but instead spur development in the surrounding area by removing blight from a very visible corner of the neighborhood and creating new, quality housing stock for City residents. The project fully conforms to the standards of the Regulations. The Petition includes extensive perimeter landscaping to separate and contain the proposed use and the Site will adequately park the proposed use, so it will have no impact on the abutting properties. In addition, the proposed front access to sidewalks from the unit will orient the buildings toward the street and have a more low-density residential appearance. The proposed use will not depreciate nearby property values, but rather, enhance them by developing a vacant and overgrown lot and dated dwelling in this neighborhood.

For the reasons stated above, the Petitioner respectfully requests approval of the application for Special Permit and Site Plan Review.

Sincerely,

LIST OF PROPERTIES WITHIN 100' OF 83 & 87 PRIMROSE AVENUE

LOCATION	Owner	Address	Citv	State Zin	Zin
64 PRIMROSE AV	MALDONADO DIANE	34 ELM ST	DERBY	CT	06418
<b>82 PRIMROSE AV</b>	POLITE JESSE & PHYLISS	<b>82 PRIMROSE AVE</b>	BRIDGEPORT	-	016606
422 WOODMONT AV		41 HEDGEHOG RD	TRUMBULI.	5 E	06611
408 WOODMONT AV		408 WOODMONT AVE	BRIDGEPORT	-	06606
<b>110 PRIMROSE AV</b>	PEART DOREEN A	<b>110 PRIMROSE AVE</b>	BRIDGEPORT	CT	06606
<b>70 PRIMROSE AV</b>	MORALES DARCY	64 PRIMROSE AVE	BRIDGEPORT CT	CT	06606
<b>87 PRIMROSE AV</b>	POTHANSZKY HELEN	<b>59 PRIMROSE AVE</b>	BRIDGEPORT	CT	06606
<b>83 PRIMROSE AV</b>	POTHANSZKY HELEN	<b>59 PRIMROSE AVE</b>	BRIDGEPORT CT		06606
536 PEET ST #566	DIVERSE BUILDERS LLC	25 SAWYER ROAD	FAIRFIELD	CT	06824
54 PRIMROSE AV	MALDONADO DIANE	34 ELM ST	DERBY	CT	06418
<b>75 PRIMROSE AV</b>	MONTEIRO FILIPE	4 LAWRENCE COURT	MILFORD	CT	06460
<b>59 PRIMROSE AV</b>	ESTATE OF HELEN POTHANSZKY	301 TURKEY ROOST RD	MONROE	CL	06468
436 WOODMONT AV	VEGA HIPOLITO & CYNTHIA	436 WOODMONT AV	BRIDGEPORT	CT	06606

DEVELOPMENT STANDARDS	ZONE: R-C	EXISTING CONDITIONS #536 PEET #83-87 PRIMROSE	PROPOSED CONDITIONS #536 PEET #83-87 PRIMROSE	
LOT AREA, MINIMUM FRONTAGE, MINIMUM	9,000 60 FT.	36,000 SF		
LOT AREA PER DWELLING UNIT, MINIMUM PRINCIPAL BUILDING SETBACK	2,700 SF	10 UNITS	13 UNITS	
FRONT LOT LINE, MINIMUM FROM	15.0 FT.	15.3	15.3'	
SIDE LOT LINE, MINIMUM FROM	10.0 FT.	10.2	10.2	
BOTH SIDES REAR LOT LINE, MINIMUM	20.0 FT. 20.0% / 50.0 FT.	N/A N/A	N/A N/A	
FRONT LOT LINE SIDE LOT LINE	50% / 75 FT. 3 FT.	N/A N/A	N/A N/A	
FLOOR AREA MAXIMUM	3 FT. 50% OF 1ST FLOOR	N/A N/A	N/A N/A	
	60.0%	22 Ool	00 <u>5%</u>	
SITE COVERAGE, MAXIMUM	70.0%	52.3%	66.5%	
D				
MINIMUM	30.0%	47.2%	33.5%	
MID-POINT OF HIGHEST ROOF	N/A 45 FT.	24.9' 30.6'	25.0' 31.0'	
FLAT/ ROUNDED ROOF	12 FT. 15 FT.	N/A N/A	N/A N/A	
<b>GENERAL NOTES:</b> 1. This Map has been prepared pursuant to the Regulation of Connecticut State Agencies Section 20-300b-1 through 20-300b-20 and the "Standards for Surveys and Maps in the State Connecticut as adopted by the Connecticut Association of Land Surveyors, Inc. on Sept. 26, 1996.	Regulation of nrough Maps in the State of ociation of Land			
<ol> <li>This Survey conforms to Class A-2.</li> <li>The Type of survey performed is a Limited Property / Boundary</li> </ol>	operty / Boundary			
4. Boundary determination is based upon a Dependent Resurvey	endent Resurvey			
5. North Arrow is based on Map Reference # 1.				×
6. This map is NOT VALID without a LIVE SIGNATURE EMBOSSED SEAL.	VATURE and			Ą
7. This map is NOT VALID if altered or used by the one depicted in title block of this map.	any party other than			
8. Property Lines Established According to Rec	Record Deeds as exist.			
9. Physical Features Such as Stone Walls, Wire Fences, Monuments, Iron Pins or Pipes, Etc. taken under consideration to establish current deed lines.	consideration to			
10. Underground Utility, Structure and facility Lo and noted hereon have been compiled, in part,	cations depicted rom record			
mapping supplied by the respective utility companies or government agencies, from parole testimony and from other sources. These Locations must be considered as approximate in nature. Additionally, other such features may exist on the site, the existence of which are unknown to this firm. The size, Location and existence of all such features must be field determined and verified by the appropriate authorities prior to construction. CALL BEFORE YOU DIG 1-800-922-4455.	panies or nd from other s approximate in exist on the site, the exize, Location le size, Location letermined and onstruction. CALL			
11. Lot served by town sewer system and public water	water supply.			
12. Elevations are based on an Assumed Datum	ne city sewer line			
13. Proposed sanitary Lines on the property to the city sewer line are the sole responsibility of the association. The City of Bridgeport will not take responsibility for the upkeep and maintenance of the sewer line between the 2 manholes.	ne city sewer line City of Bridgeport aintenance of the			
NAP REFE				
2. RECORD MAP VOL. #30, MAP #47				

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PROPO TO MATCH

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PRO

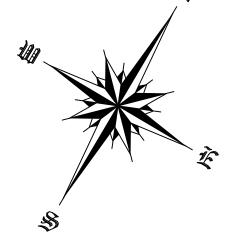
# PARKING CALCULATIONS

CURRENT PARKING

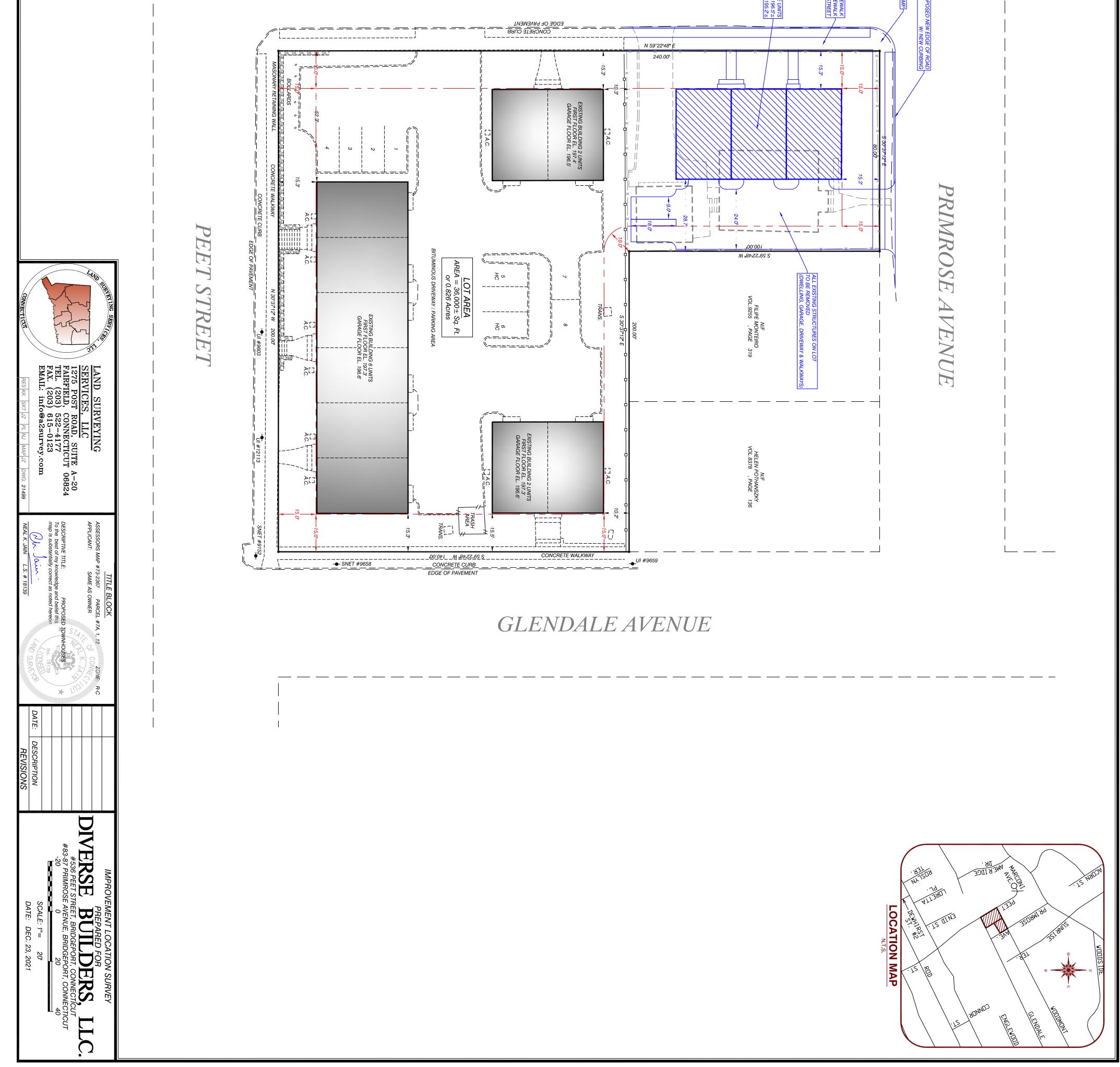
TOTAL UNITS = 10 TOTAL PARKING SPACES = 15 SPACES (10 INSIDE & 5 OUTSIDE)

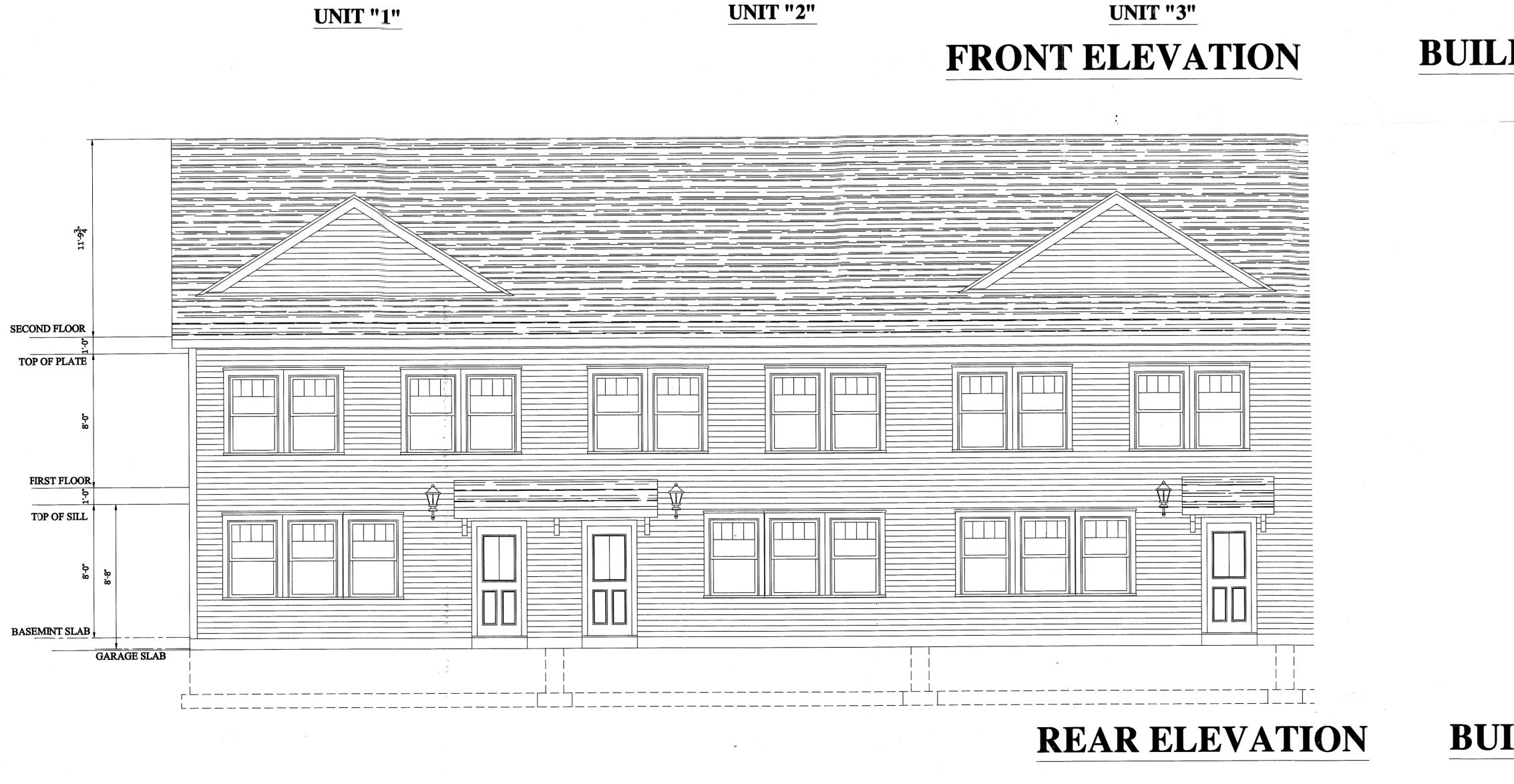
PROPOSED PARKING

PROPOSED TOTAL UNITS = 13 PROPOSED TOTAL PARKING = 20 SPACES (13 INSIDE & 7 OUTSIDE



WOODMONT AVENUE







**UNIT "1**"

**UNIT "2"** 

- 66,0-

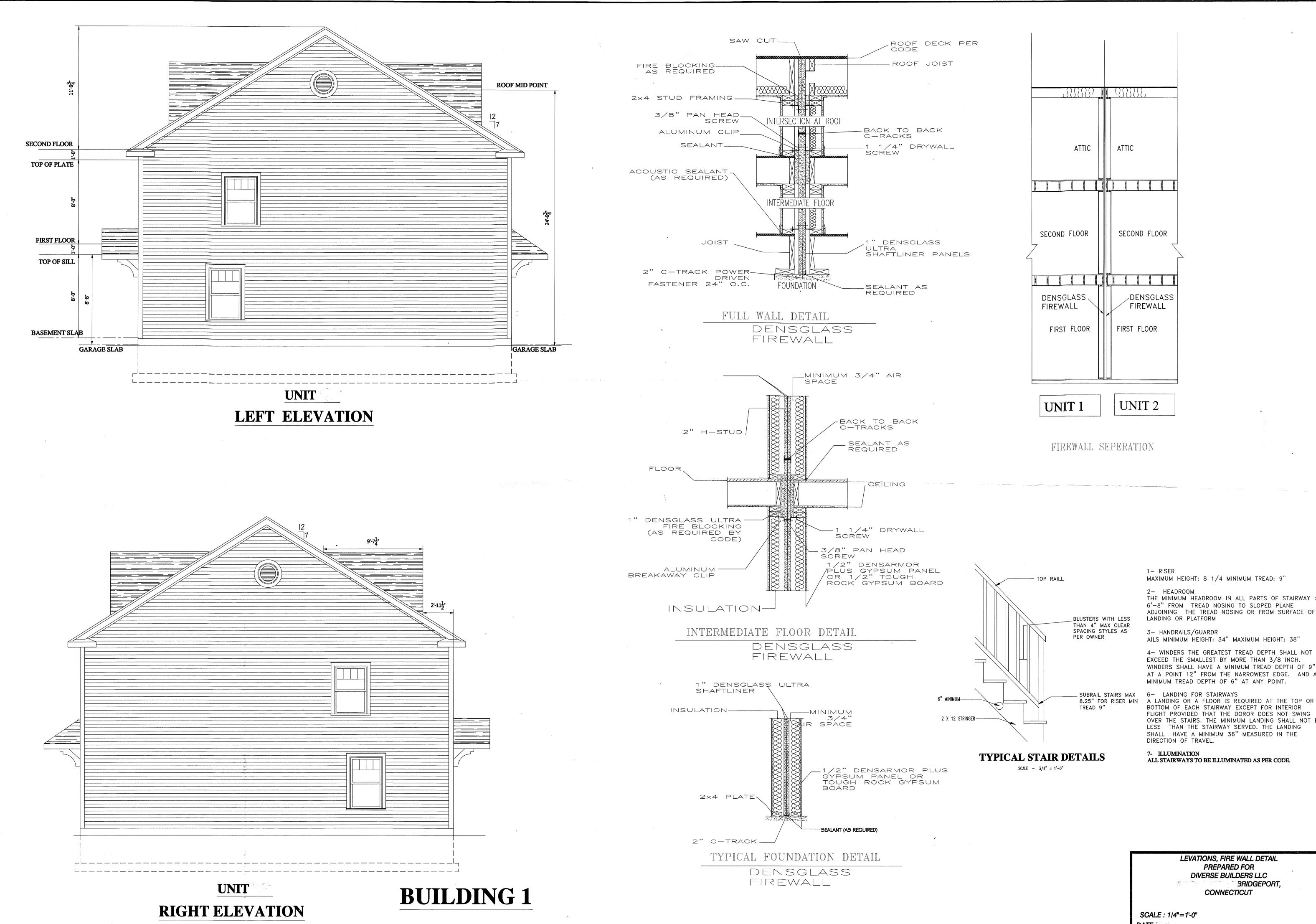
**BUILDING 1** 

**BUILDING 1** 

EAR ELEVATIONS PREPARED FOR DIVERSE BUILDERS LLC

BRIDGEPORT, CONNECTICUT

SCALE : 1/4"=1'-0" DATE



LEVATIONS, FIRE WALL DETAIL PREPARED FOR DIVERSE BUILDERS LLC **BRIDGEPORT**, Rent Frank CONNECTICUT

SCALE : 1/4"=1'-0" DATE

7- ILLUMINATION ALL STAIRWAYS TO BE ILLUMINATED AS PER CODE.

DIRECTION OF TRAVEL.

AT A POINT 12" FROM THE NARROWEST EDGE. AND A

MINIMUM TREAD DEPTH OF 6" AT ANY POINT.

BOTTOM OF EACH STAIRWAY EXCEPT FOR INTERIOR

LESS THAN THE STAIRWAY SERVED. THE LANDING

SHALL HAVE A MINIMUM 36" MEASURED IN THE

FLIGHT PROVIDED THAT THE DOROR DOES NOT SWING

OVER THE STAIRS. THE MINIMUM LANDING SHALL NOT BE

4- WINDERS THE GREATEST TREAD DEPTH SHALL NOT EXCEED THE SMALLEST BY MORE THAN 3/8 INCH. WINDERS SHALL HAVE A MINIMUM TREAD DEPTH OF 9"

3- HANDRAILS/GUARDR

AILS MINIMUM HEIGHT: 34" MAXIMUM HEIGHT: 38"

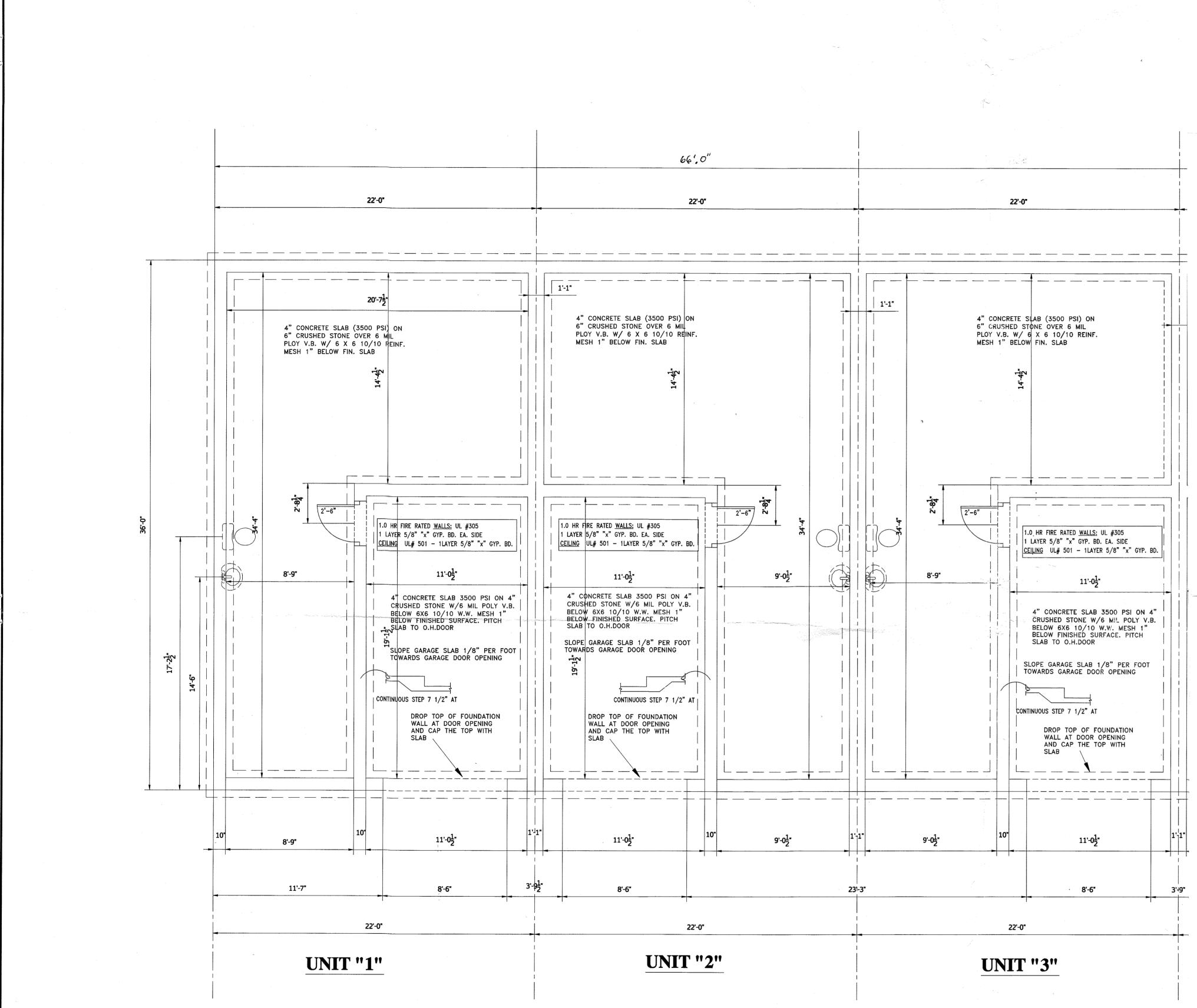
ADJOINING THE TREAD NOSING OR FROM SURFACE OF LANDING OR PLATFORM

2- HEADROOM THE MINIMUM HEADROOM IN ALL PARTS OF STAIRWAY : 6'-8" FROM TREAD NOSING TO SLOPED PLANE

MAXIMUM HEIGHT: 8 1/4 MINIMUM TREAD: 9"

1– RISER

()()()() ATTIC N NIN N N N SECOND FLOOR \_DENSGLASS FIREWALL FIRST FLOOR UNIT 2

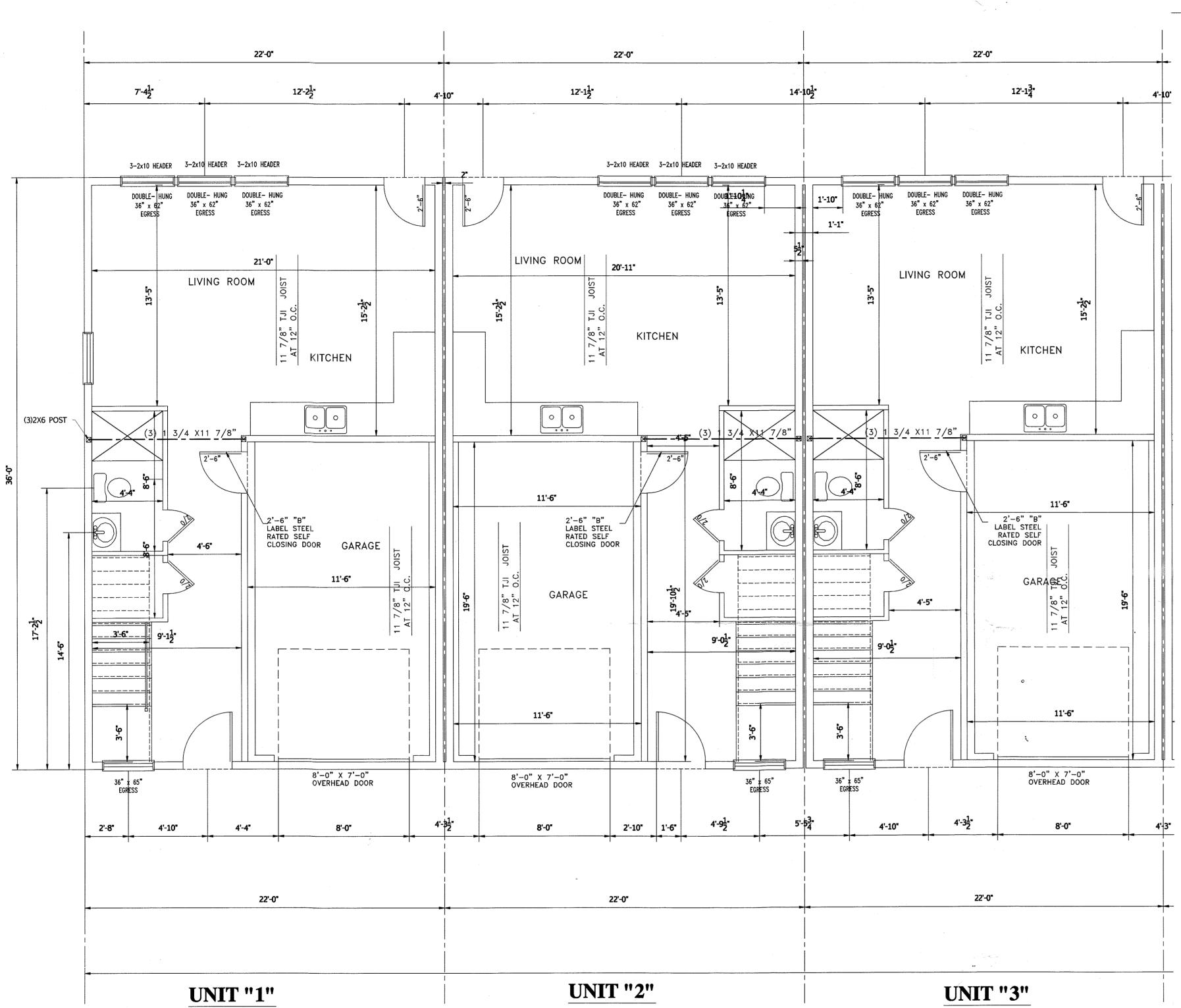


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## **BUILDING 1**

### FOUNDATION PLAN PLAN





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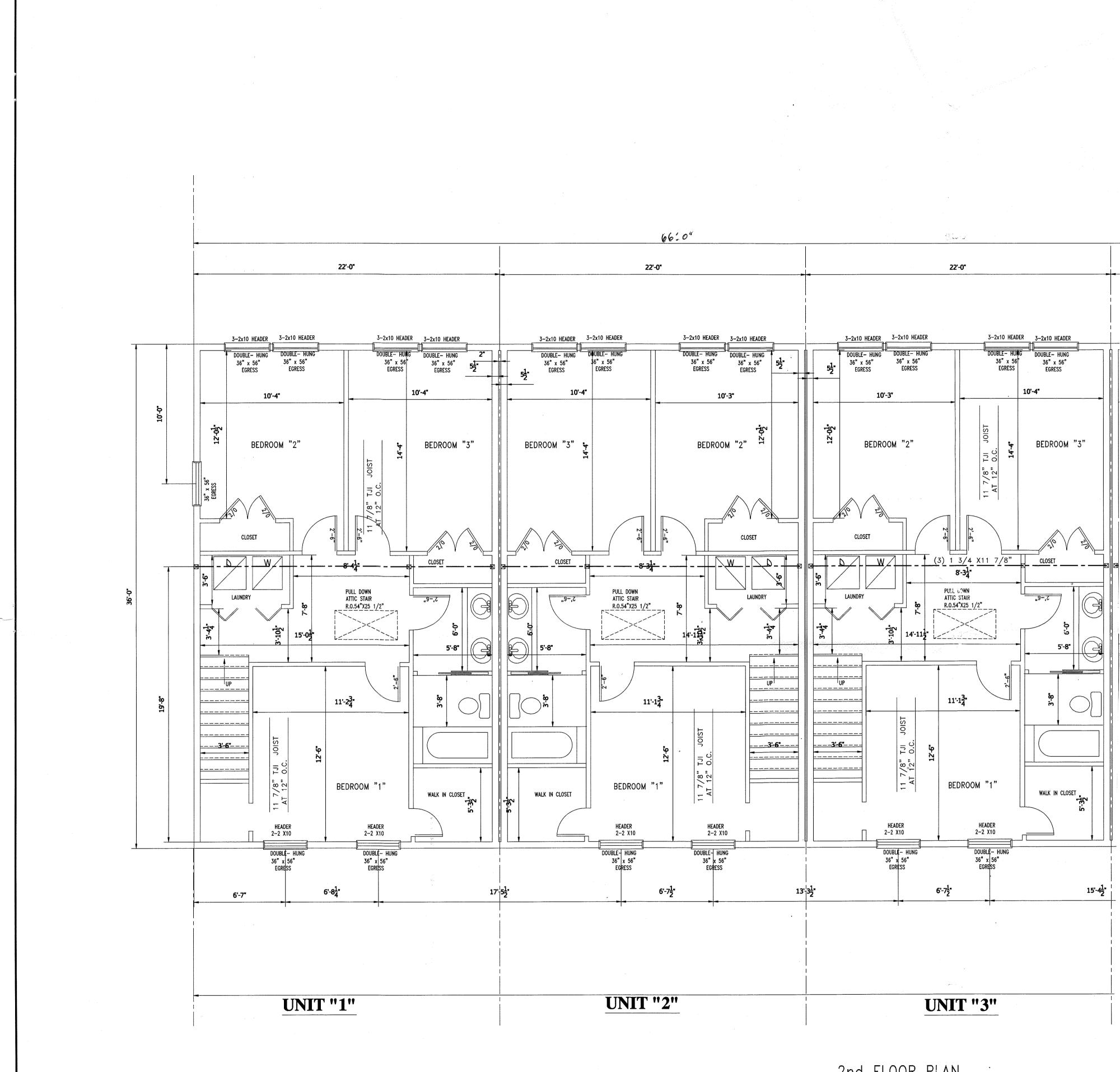
# **BUILDING 1**

3 1

### FIRST FLOOR PLAN

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# FIRST FLOOR PREPARED FOR DIVERSE BUILDERS LLC BRIDGEPORT, CONNECTICUT SCALE : 1/4"=1'-0" DATE



# **BUILDING 1**

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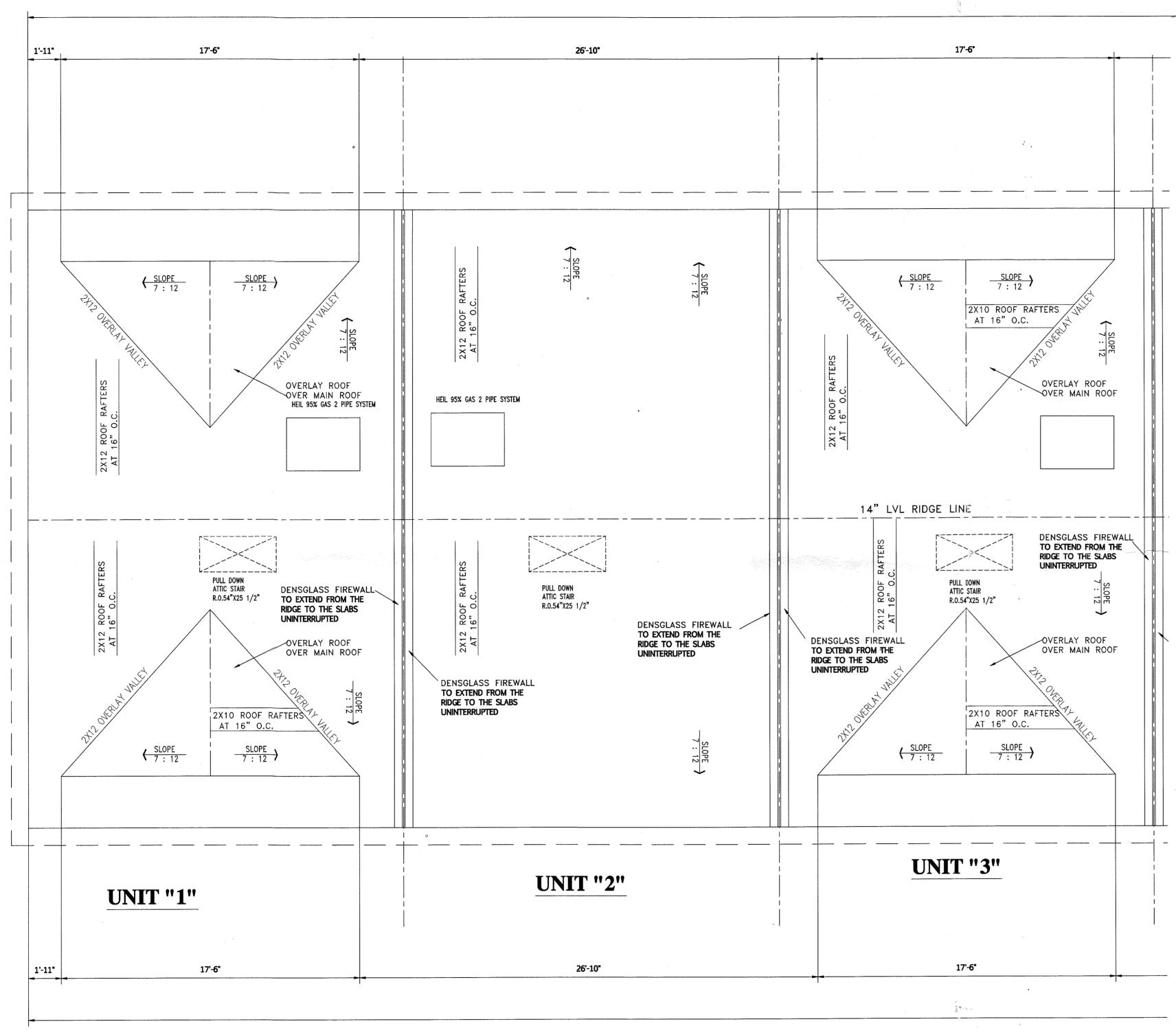
2nd FLOOR PLAN

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# 2ND FLOOR PREPARED FOR DIVERSE BUILDERS LLC

BRIDGEPORT, CONNECTICUT

SCALE : 1/4"=1'-0" DATE



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# **BUILDING 1**

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### ATTIC PREPARED FOR DIVERSE BUILDERS LLC

BRIDGEPORT, CONNECTICUT

SCALE : 1/4"=1'-0" DATE CITY OF BRIDGEPORT PLANNING & ZONING COMMISSION



### CHECKLIST FOR PUBLIC HEARING APPLICATIONS

- I. <u>REQUIRED INFORMATION</u> (except for <u>Fee & USB</u> submit an original & 16 copies of all below)
  - □ Completed & Signed Application & Checklist Form
  - □ Fee
  - □ Written Statement of Development Use
  - □ Completed Site Plan
  - Drainage Plan
  - □ Building Floor Plans
  - □ Property Owner's List
  - □ Cert. of Corporation/Org. of First Report
  - □ A-2 Site Survey
  - Building Elevations
  - □ Other Evidence/Testimonial Information
  - □ 1 USB MEMORY FLASH DRIVE STICK

### **NOTE:** Please provide 1 USB MEMORY FLASH DRIVE Stick:

- The information on the memory flash drive sticks must include the application, site plans, and all other hard copy information (landscaping, floor elevations, etc) that will be submitted. It also **must be labeled** with the property address, applicant name and date of hearing.
- All plans and paper work that is submitted to the zoning office must be FOLDED (11x17 or smaller) and Collated into 17 separate packets.

### II. SUPPLEMENTARY INFORMATION (Optional)

- □ Perspective Rendering
- □ Building and Site Sections
- □ Eight 8 x10 Color or Black/White Photos of the Current Premises' Condition
- □ Copies of Zoning Board of Appeals, or Historic District Commission Decisions
- Drainage Report
- □ Traffic Studies
- □ Environmental Impact Statement
- □ Real Estate Studies
- Department of Environmental Protection or Coastal Area Management reports
- □ Aerial Photographs

### III. OPTIONAL EXHIBITS (may be presented at the public hearing (16 copies not required)

- □ Color Rendering
- □ Models
- Material Sample
- OTHER: \_\_\_\_\_\_

### **CITY OF BRIDGEPORT**

### **PLANNING & ZONING COMMISSION**

### CHECKLIST FOR PUBLIC HEARING APPLICATIONS

The following requirements shall apply to all applications for public hearings before the Bridgeport Planning & Zoning Commission and for all agenda dates on or after December 23, 2011.

The following are required components for any and all applications for a **change of zone**; **site plan review**; **motor vehicle**; **sub-division**; **special permit**; **or coastal site plan reviews** applications. Except for the Fee & USB, the Petitioner shall submit **one (1) original and sixteen (16) copies of all materials described below in sections I & II pertinent to the application**. The agenda closing date shall be five (5) weeks prior to the public hearing. No materials submitted by the petitioner after the agenda closing date shall be accepted by the Clerk or by the Commission, unless exempted under Section III below. Failure to provide any of the components listed under Section I below may be deemed by the Commission to be grounds for denial due to incomplete information.

### I. <u>REQUIRED INFORMATION</u>

□ A Complete and signed application form. (The application must be signed by the current property owner)

- □ Fee
- □ A written statement, not to exceed one hundred (100) words, describing all proposed uses.
- The original plus sixteen (16) copies of a site plan prepared, signed and sealed by an engineer, architect or landscape architect registered and licensed to conduct business in the State of CT. Dated and meeting the following requirements:
  - The site plan must be drawn to a scale of 100 feet or less to the inch.
  - Proposed and existing structures and amenities, including, but not limited to, footprints of foundations, porches, decks, walkways, travel lanes, shall be indicated. Dimensions to property lines from structures and overall building dimensions shall also be shown. The dimensions of parking lot, including isle width and length, and width of parking spaces shall be shown.
  - All applicable (existing and proposed) Zone Development Standards.
  - Existing and proposed grades shall be shown at 2-foot intervals.
  - One or more benchmarks that can be used in the field to verify conditions shall be indicated.

- A drainage plan prepared by a professional engineer, showing all provisions for site runoff; on-site retentions; connections to city services; and any other pertinent information, including City Engineer's requirements.
- Building floor plans (all floors above and below grade) shall be prepared by a licensed architect, showing any and all proposed new construction or additions to existing structures. Additions and alterations shall be clearly delineated from existing work. Minimum scale 1/16" = 1"0.
- A list of names and addresses of all property owners within 100 feet of all property lines of the subject property shall be provided.
- If the petitioner is a corporation a copy of the "Certificate of Corporation" and "Organization and First Report" as filed with the Office of the Secretary of the State of CT must be filed with the application.
- □ An A-2 survey.
- □ For applications involving a building(s), the following shall be submitted:
  - Preliminary architectural plans, sections, and/or elevations at 1/4" or 1/8"
     = 1' showing exterior wall elevations, roof lines, façade materials or other features of proposed buildings or structures.
  - Drawings prepared by a registered architect, landscape architect or professional engineer licensed in the State of CT, each individually sealed and signed by the design professional, (except seals not required on residential projects of less than 5,000 square feet total).
- □ Any other evidence or testimonial information, which will be presented by the petitioner at a public hearing.
- <u>Note</u>: All of the above information shall be submitted at the time of filing. Applications with missing information will be deemed incomplete; will not be processed and will be immediately returned to the applicant.

### II. <u>SUPPLEMENTARY INFORMATION</u>

- Perspective renderings, either in black and white or in color, reproduced either photographically or by diazo print, showing print, showing principal street side view of the proposed development. Minimum size 8"x10" (for photos); Maximum size 30"x42". Color renderings may be presented at the public hearing provided diazo print or photo reproduction has been submitted to the Clerk for distribution before the agenda closing date.
- Building and site section drawings to show relationship of proposed development to existing adjacent streets and buildings.

- Not more than eight (8) 8"x10" color or black and white photographs showing existing site conditions or surrounding area. These may be reproduced xerographically for application filing.
- Copies of any pertinent actions by the Zoning Board of Appeals or Historic District Commission.
- Drainage reports, traffic studies, environmental impact studies and/or real estate studies.
- State Department of Environmental Protection (DEP) or Coastal Area Management (CAM) reports.
- □ Aerial photographs of subject parcel and surrounding environment.

### III. OPTIONAL EXHIBITS

The following items may be presented to the Commission at the time of the public hearing (16 copies not required) without need for filing on or before the agenda closing date:

- Color renderings (see Section II item) provided the Commission has received through the Clerk reduced photographic reproductions, or black and white versions of the renderings.
- $\Box$  Models of proposed building(s).
- □ Samples of materials and/or colors to be used in the proposed development.

**Note:** Staff reports or departmental correspondence (e.g. City Engineer, W.P.C.A., Fire Marshal, Design Review Coordinator, etc.) shall be received and distributed by the Clerk of the Commission on or before the date of the public hearing. Whether such reports or correspondence is received before the agenda closing date shall not pose any penalty to the Petitioner and shall be the responsibility of the staff.

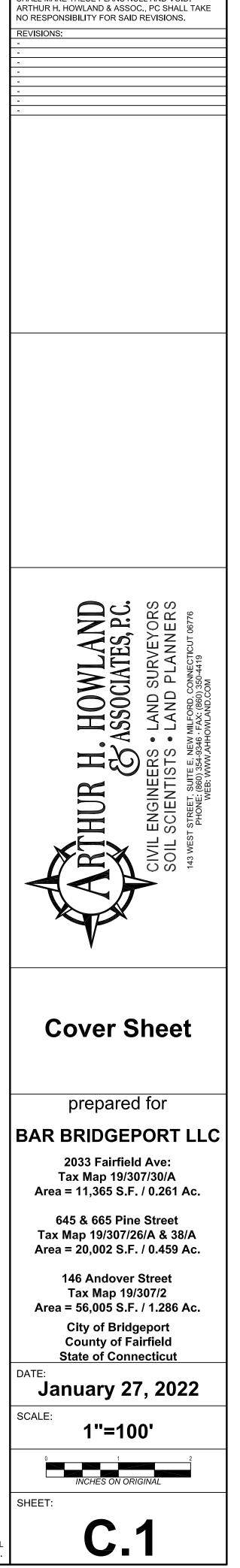


Underground utility, structure and facility locations shown have been determined from record maps provided by utility companies, governmental agencies, testimony, field locations, and other sources. Other utilities may exist on site or in the area shown. The size, location, and existence of all underground features must be field verified by the appropriate providers prior to construction Call Before You Dig, 1-800-922-4455.

# Site Development Plan For Proposed **Car Wash Facility**

C.1	Cover Sheet	1"=100'
EC.1	Existing Conditions Map	1"=20'
SD.1	Site Development Plan	1"=20'
GDU.1	Site Grading, Drainage & Utility Plan	1"=20'
LL.1	Site Lighting & Landscaping Plan	1"=20'
D.1	Detail Sheet	N.T.S.
D.2	Detail Sheet	N.T.S.

146 Andover Street Bridgeport, CT



COPIES NOT BEARING THE EMBOSSED SEAL

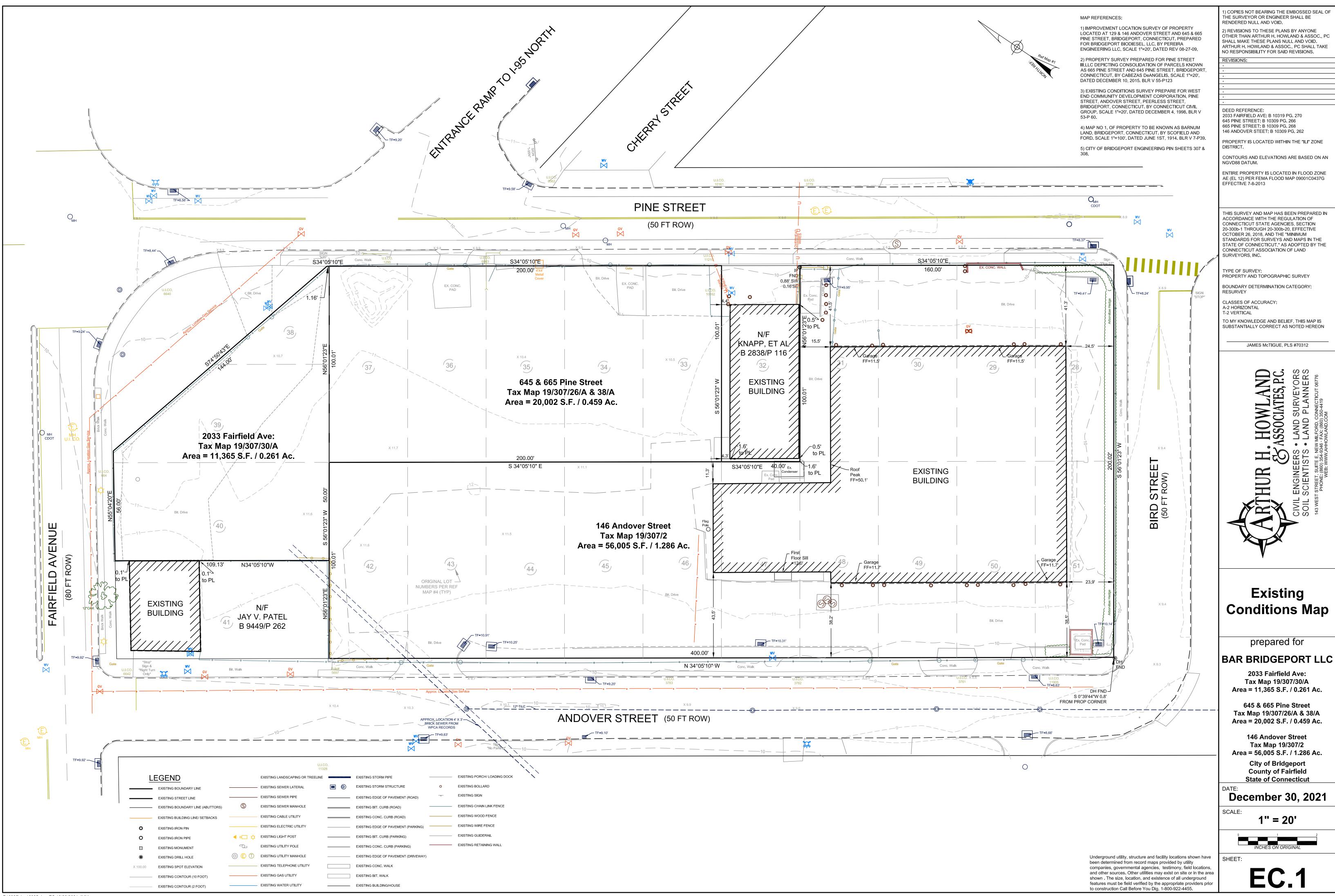
THE SURVEYOR OR ENGINEER SHALL

REVISIONS TO THESE PLANS BY ANYONE THER THAN ARTHUR H. HOWLAND & ASSOC., HALL MAKE THESE PLANS NULL AND VOID.

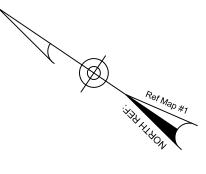
RENDERED NULL AND VOID.

1) THESE PLANS ARE INTENDED FOR APPROVAL PURPOSES AND ARE NOT FOR CONSTRUCTION.

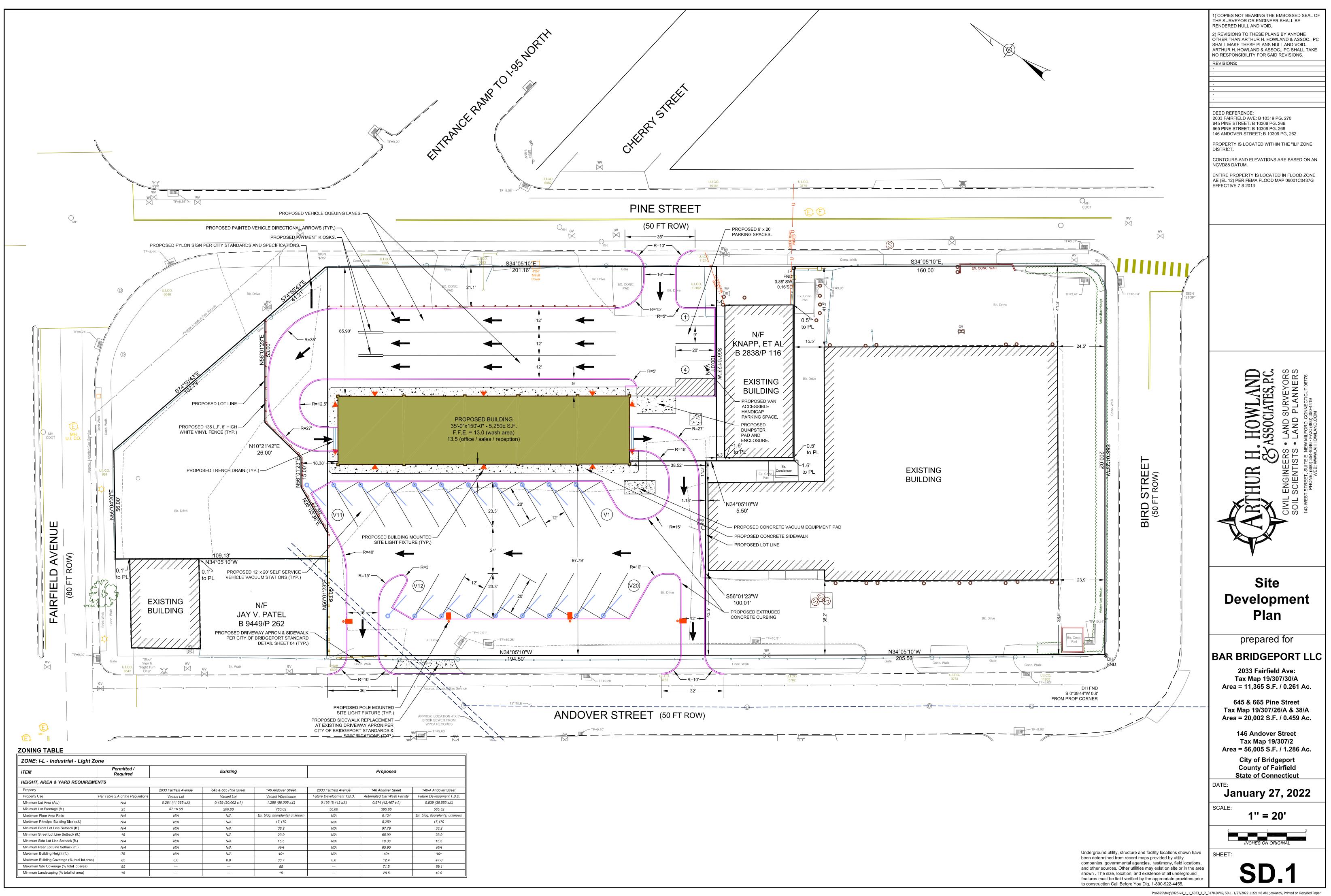
P:\6825\dwg\6825-v4\_1\_1\_6033\_1\_2\_3170.DWG, C.1, 1/27/2022 11:19:28 AM, joskandy, Printed on Recycled Paper!

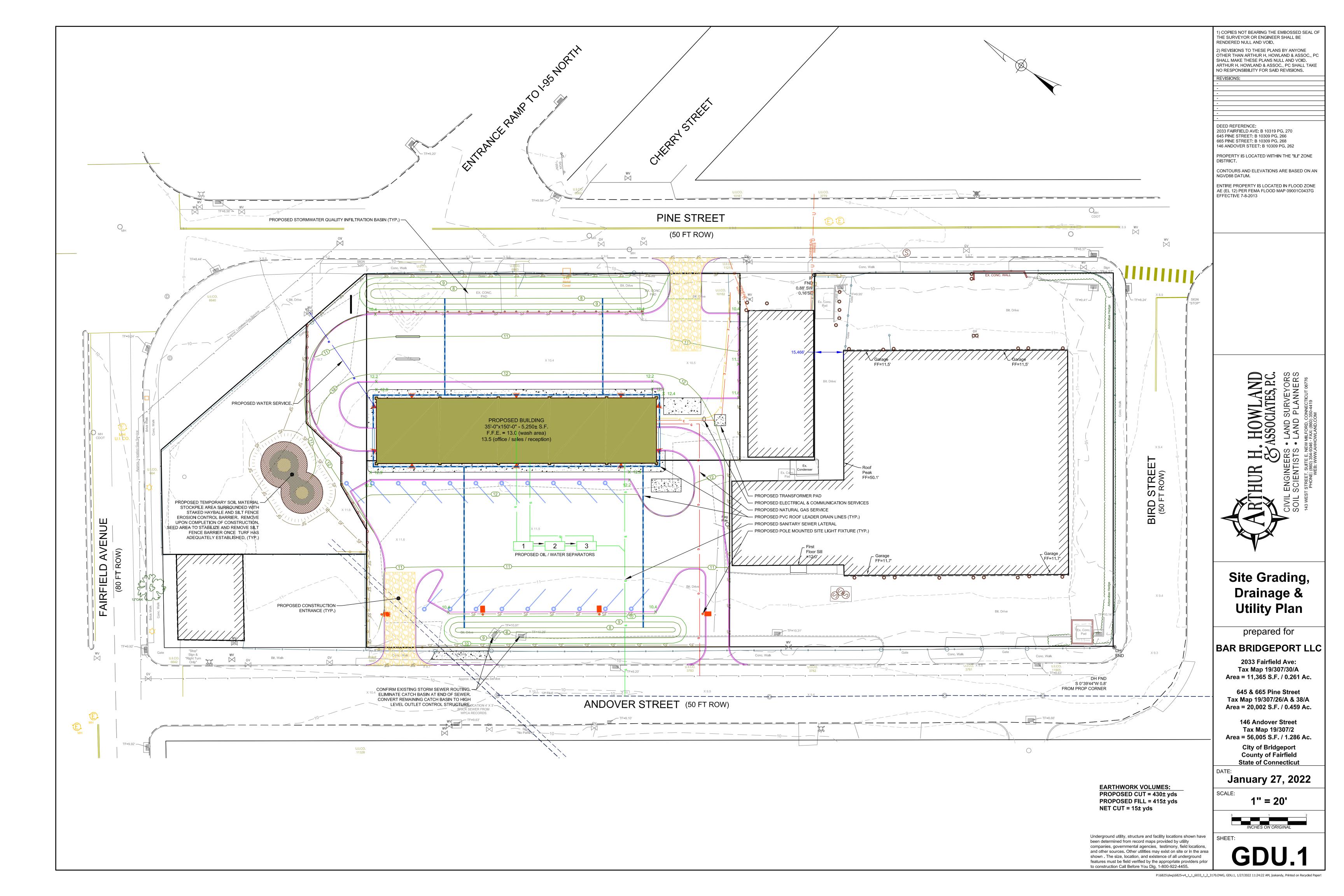


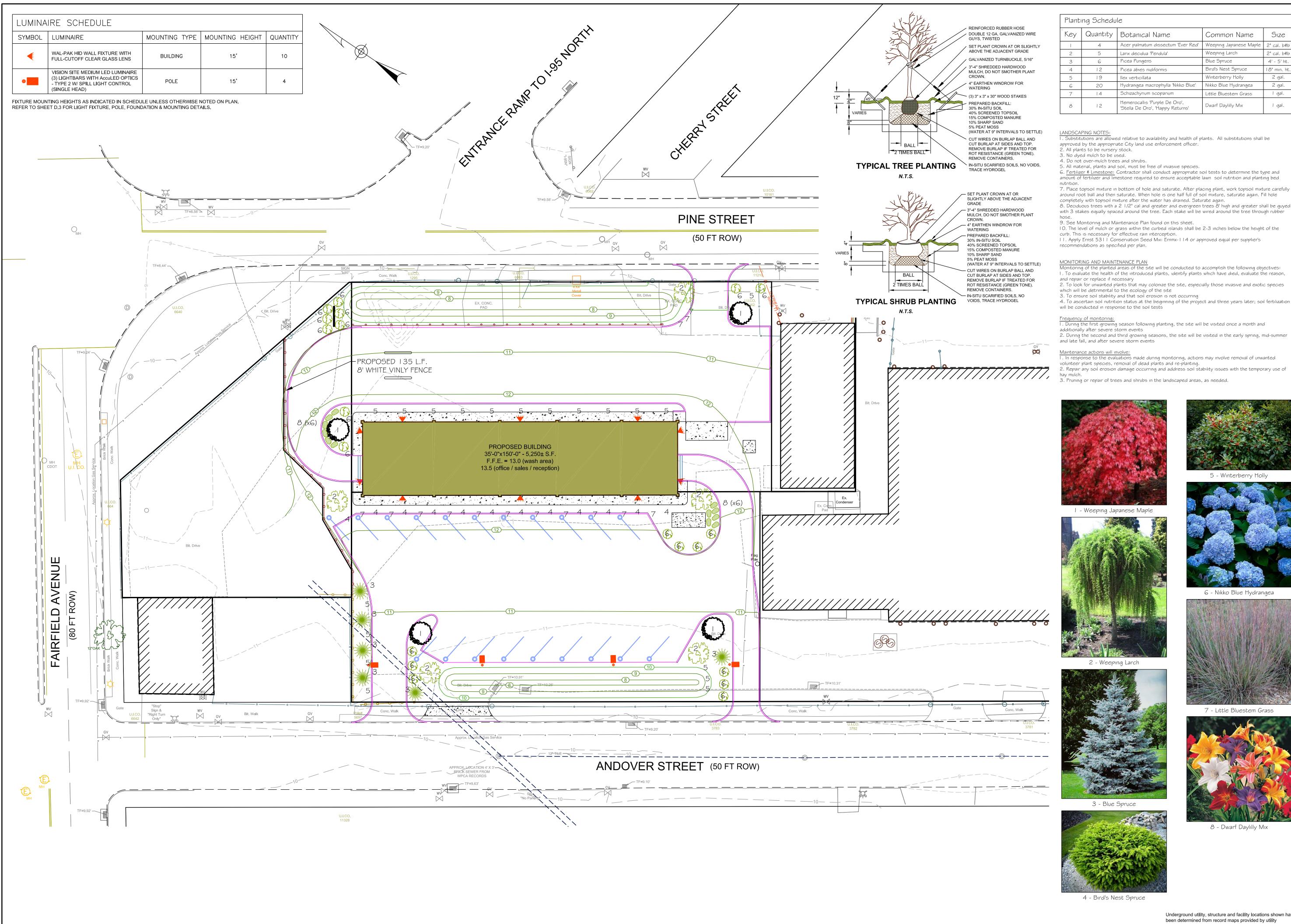
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### Key | Quantity | Botanical Name Common Name Sıze Acer palmatum dissectum 'Ever Red' Weeping Japanese Maple 2" cal. b¢b Veeping Larch Larix decidua 'Pendula' 2" cal. b¢b Blue Spruce 4' - 5' ht. Bird's Nest Spruce 18" min. ht Ninterberry Holly 2 gal. Nikko Blue Hydrangea Hydrangea macrophylla 'Nikko Blue' 2 aal. Schizachyrium scoparium lttle Bluestem Grass I gal. Hemerocallis 'Purple De Oro', warf Daylılly Mıx l gal. 'Stella De Oro', 'Happy Returns'

LANDSCAPING NOTES: 1. Substitutions are allowed relative to availability and health of plants. All substitutions shall be approved by the appropriate City land use enforcement officer.

5. All material, plants and soil, must be free of invasive species.

amount of fertilizer and limestone required to ensure acceptable lawn soil nutrition and planting bed

7. Place topsoil mixture in bottom of hole and saturate. After placing plant, work topsoil mixture carefully around root ball and then saturate. When hole is one half full of soil mixture, saturate again. Fill hole completely with topsoil mixture after the water has drained. Saturate again. 8. Deciduous trees with a 2 1/2" cal and greater and evergreen trees 8' high and greater shall be guyed

with 3 stakes equally spaced around the tree. Each stake will be wired around the tree through rubber 9. See Monitoring and Maintenance Plan found on this sheet.

10. The level of mulch or grass within the curbed islands shall be 2-3 inches below the height of the curb. This is necessary for effective rain interception. II. Apply Ernst 5311 Conservation Seed Mix: Ernmx-114 or approved equal per supplier's

oring of the planted areas of the site will be conducted to accomplish the following objectives: 1. To evaluate the health of the introduced plants, identify plants which have died, evaluate the reason, 2. To look for unwanted plants that may colonize the site, especially those invasive and exotic species

which will be detrimental to the ecology of the site 3. To ensure soil stability and that soil erosion is not occurring 4. To ascertain soil nutrition status at the beginning of the project and three years later; soil fertilization

Maintenance actions will involve: 1. In response to the evaluations made during monitoring, actions may involve removal of unwanted volunteer plant species, removal of dead plants and re-planting. 2. Repair any soil erosion damage occurring and address soil stability issues with the temporary use of



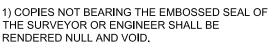


7 - Little Bluestem Grass



8 - Dwarf Daylilly Mix

Underground utility, structure and facility locations shown have been determined from record maps provided by utility companies, governmental agencies, testimony, field locations, and other sources. Other utilities may exist on site or in the area shown . The size, location, and existence of all underground features must be field verified by the appropriate providers prior to construction Call Before You Dig, 1-800-922-4455.



2) REVISIONS TO THESE PLANS BY ANYONE OTHER THAN ARTHUR H. HOWLAND & ASSOC., PC SHALL MAKE THESE PLANS NULL AND VOID. ARTHUR H. HOWLAND & ASSOC., PC SHALL TAKE NO RESPONSIBILITY FOR SAID REVISIONS. EVISIONS

### DEED REFERENCE:

2033 FAIRFIELD AVE: B 10319 PG. 270 645 PINE STREET: B 10309 PG. 266 665 PINE STREET: B 10309 PG. 268 146 ANDOVER STEET: B 10309 PG. 262

PROPERTY IS LOCATED WITHIN THE "ILI" ZONE DISTRICT.

CONTOURS AND ELEVATIONS ARE BASED ON AN NGVD88 DATUM.

ENTIRE PROPERTY IS LOCATED IN FLOOD ZONE AE (EL 12) PER FEMA FLOOD MAP 09001C0437G EFFECTIVE 7-8-2013



### Site Lighting & Landscaping Plan

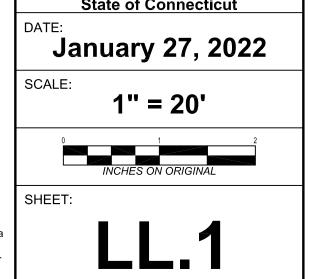
prepared for

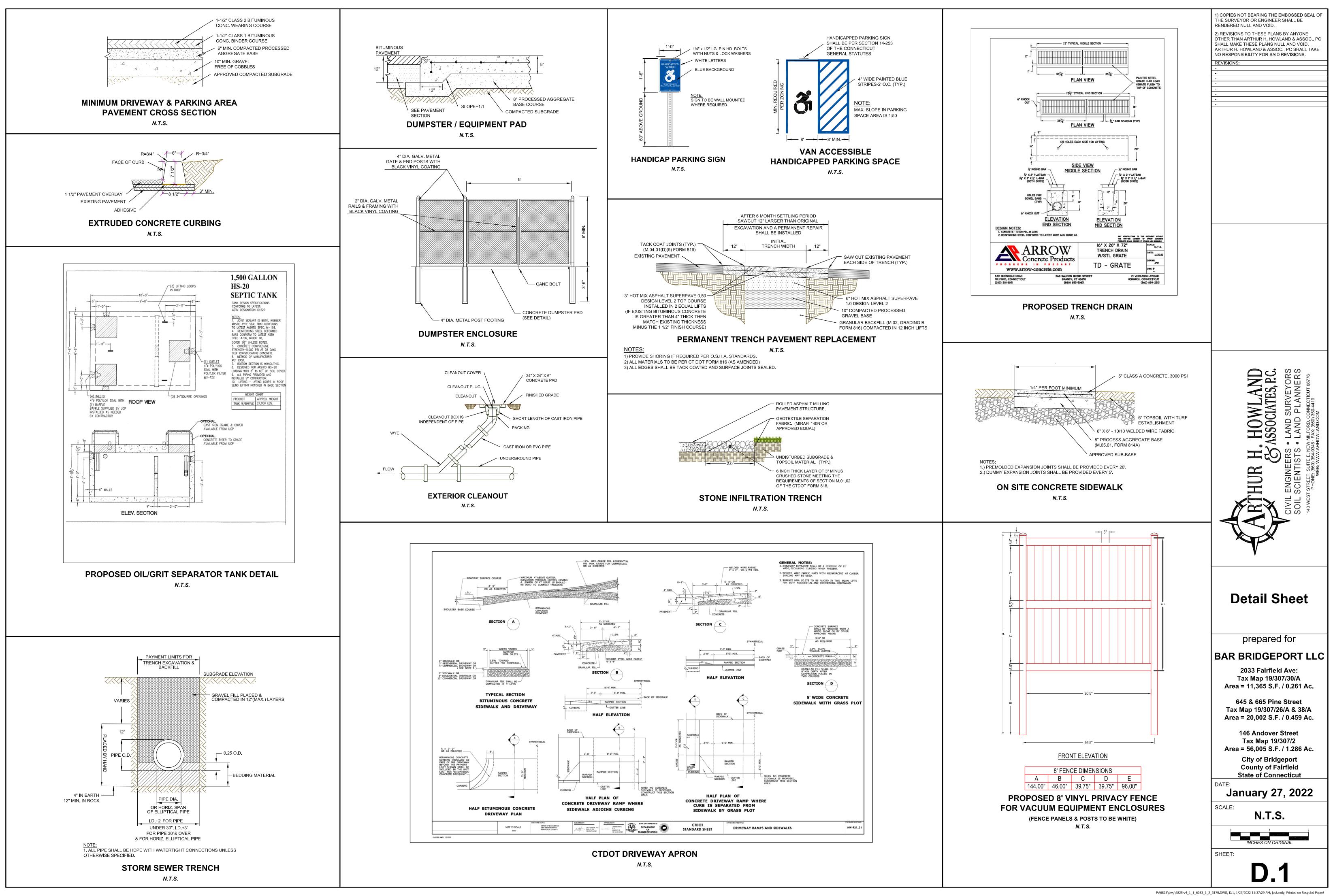
### BAR BRIDGEPORT LLC

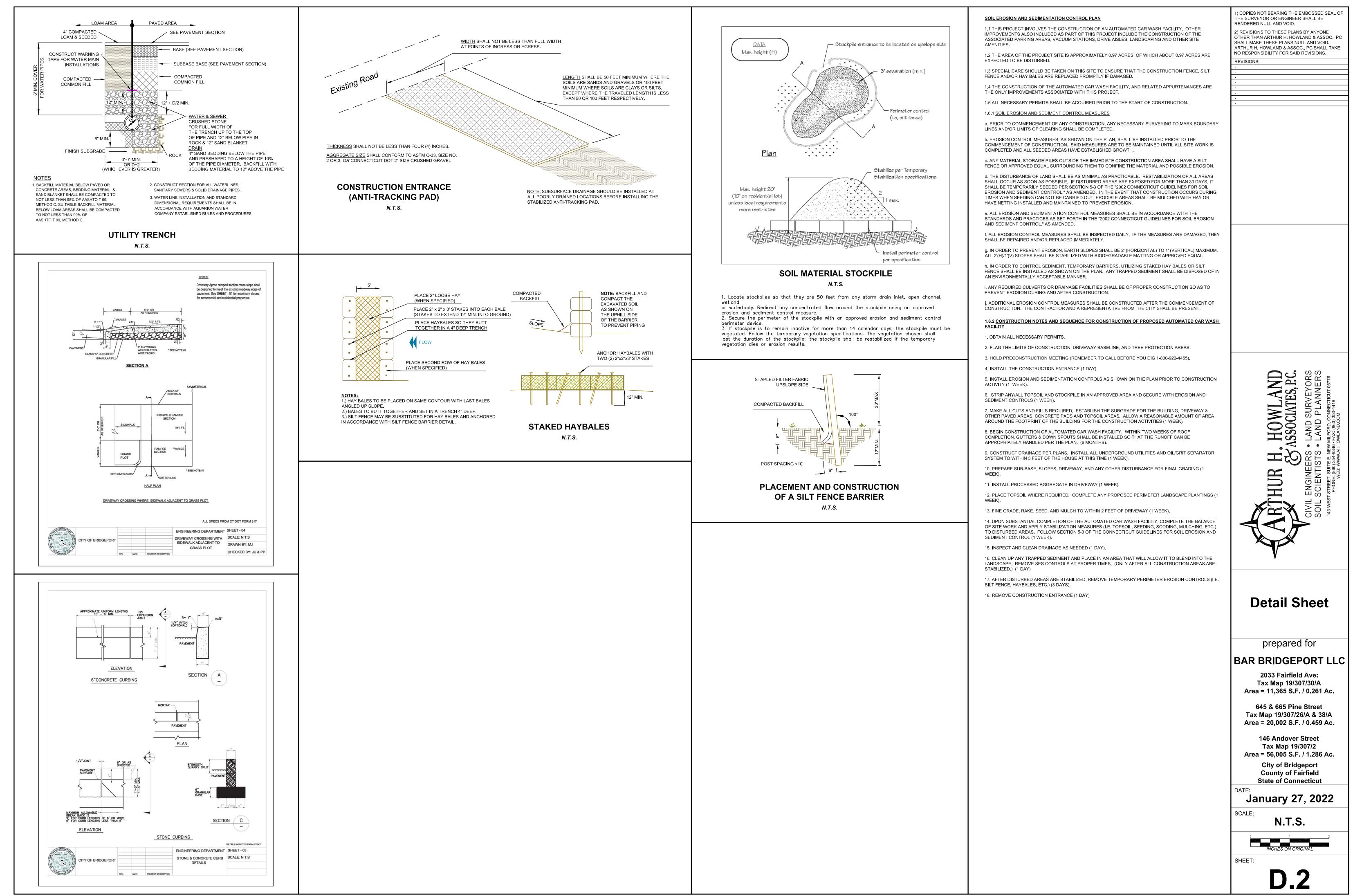
2033 Fairfield Ave: Tax Map 19/307/30/A Area = 11,365 S.F. / 0.261 Ac.

645 & 665 Pine Street Tax Map 19/307/26/A & 38/A Area = 20,002 S.F. / 0.459 Ac.

146 Andover Street Tax Map 19/307/2 Area = 56,005 S.F. / 1.286 Ac. City of Bridgeport County of Fairfield State of Connecticut







and and a	CITY OF BRIDGEPORT File No
L JO TNAS	PLANNING & ZONING COMMISSION APPLICATION
1.	NAME OF APPLICANT: Arthur H. Howland & Associates, PC
2.	V
	If yes, a sworn statement disclosing the Beneficiary shall accompany this application upon filing.
3.	Address of Property: 146 Andover Street CT 06605
	(number) (street) (state) (zip code)
4.	Assessor's Map Information: Block No19-307Lot No2
5.	Amendments to Zoning Regulations: (indicate) Article: N/A Section: N/A
	(Attach copies of Amendment)
6.	Description of Property (Metes & Bounds): Refer to sheets EC.1 & SD.1 for existing and proposed property boundary bearings, distances and areas.
7	Existing Zone Classification: Industrial - Light Zone (I-L)
8.	NI/A
9.	Describe Proposed Development of Property: Construction of an automated car wash facility with the
	associated self-service vacuum stations and other associated site amenities.
	Approval(s) requested: Special Permit & Site Plan Approval
	Signature: Date:
	Print Name:
	Phone:         (860) 354-9346         Cell:         N/A         Fax:         (860) 350-4419           E-mail Address:         pszymanski@ahhowland.com         Fax:         (860) 350-4419         Fax:         (860) 350-4419
	E-mail Address: pszymanski@annowiand.com
	\$Fee received Date: Clerk:
	THIS APPLICATION MUST BE SUBMITTED IN PERSON AND WITH COMPLETED CHECKLIST
	□ Completed & Signed Application Form □ A-2 Site Survey □ Building Floor Plans
	Completed Site / Landscape Plan     Drainage Plan     Building Elevations
	<ul> <li>Written Statement of Development and Use</li> <li>Property Owner's List</li> <li>Fee</li> </ul>
	<ul> <li>Cert. of Incorporation &amp; Organization and First Report (Corporations &amp; LLC's)</li> </ul>
	James E. Balise, Jr.
	Print Owner's Name Owner's Signature Date
	Print Owner's Name Owner's Signature Date

Rev. 6/18/2016



East Elevation



West Elevation

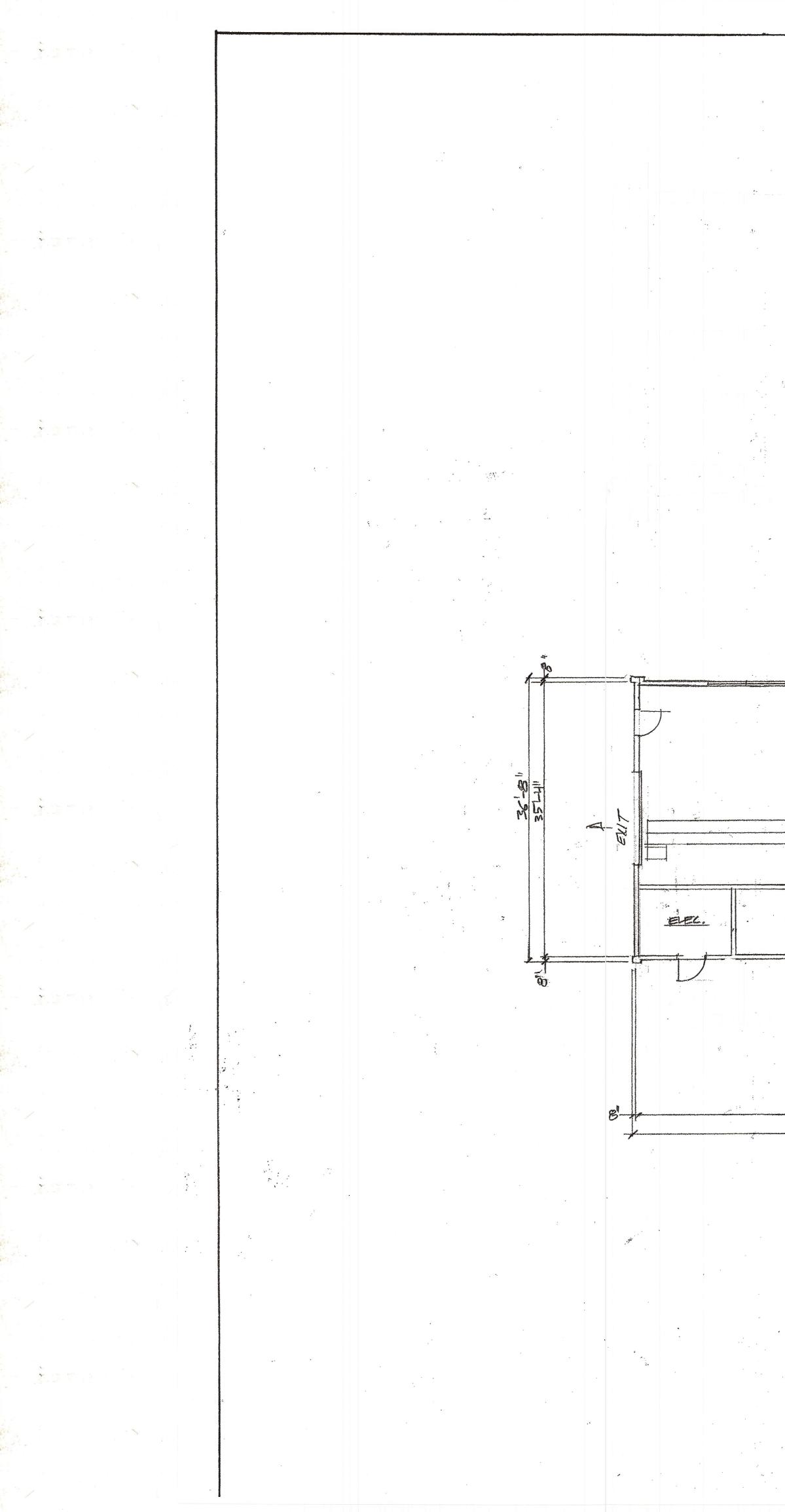


**South Elevation** 

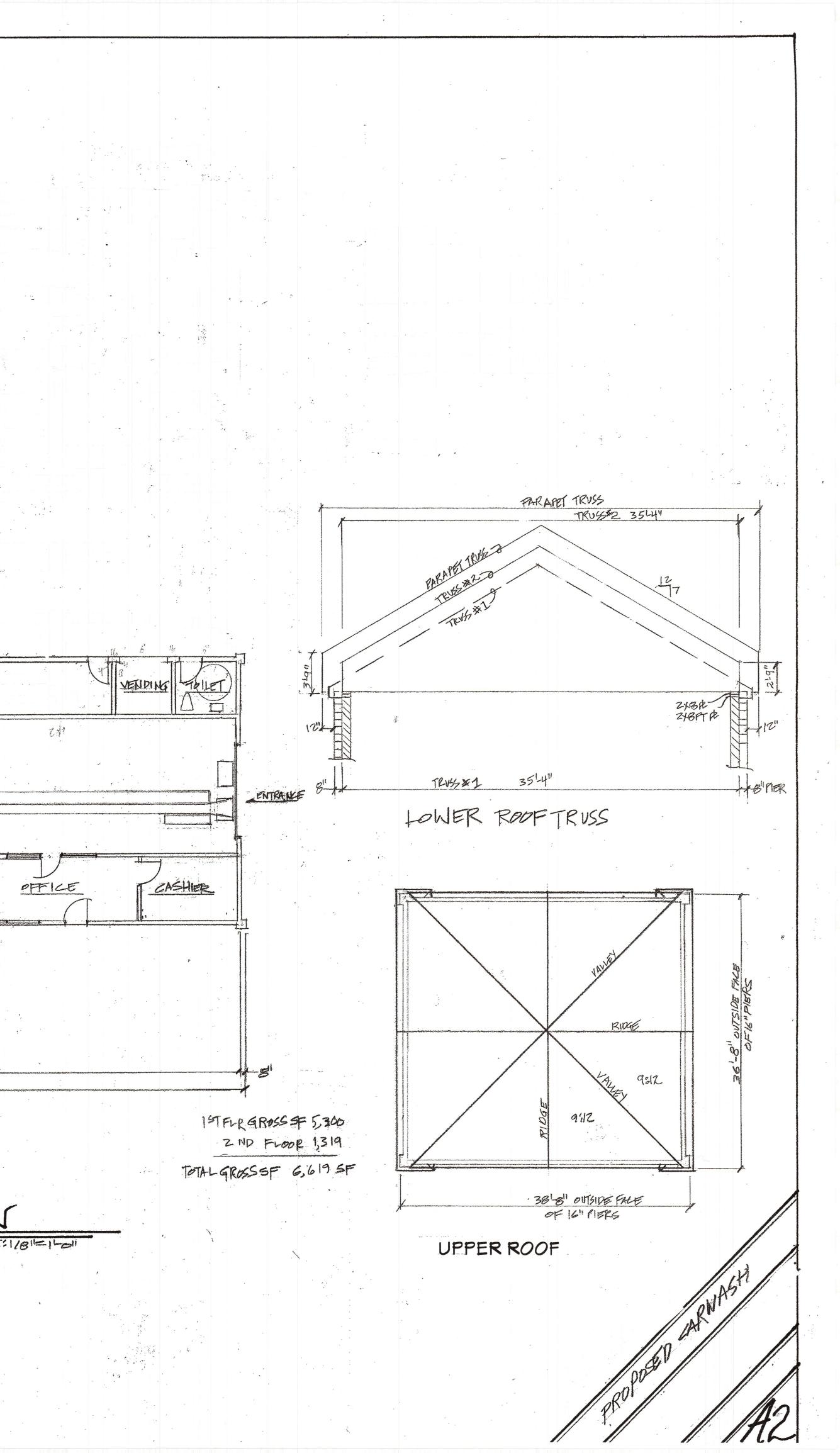


North Elevation

1) COPIES NOT BEARING THE EMBOSSED SEAL OF THE SURVEYOR OR ENGINEER SHALL BE RENDERED NULL AND VOID. 2) REVISIONS TO THESE PLANS BY ANYONE OTHER THAN ARTHUR H. HOWLAND & ASSOC., PC SHALL MAKE THESE PLANS NULL AND VOID. ARTHUR H. HOWLAND & ASSOC., PC SHALL TAKE NO RESPONSIBILITY FOR SAID REVISIONS. REVISIONS DEED REFERENCE: 2033 FAIRFIELD AVE: B 10319 PG. 270 645 PINE STREET: B 10309 PG. 266 665 PINE STREET: B 10309 PG. 268 146 ANDOVER STEET: B 10309 PG. 262 PROPERTY IS LOCATED WITHIN THE "ILI" ZONE DISTRICT. CONTOURS AND ELEVATIONS ARE BASED ON AN NGVD88 DATUM. ENTIRE PROPERTY IS LOCATED IN FLOOD ZONE AE (EL 12) PER FEMA FLOOD MAP 09001C0437G EFFECTIVE 7-8-2013 /EYOR HUR ENGINE Typical Proposed Building Elevations prepared for BAR BRIDGEPORT LLC 2033 Fairfield Ave: Tax Map 19/307/30/A Area = 11,365 S.F. / 0.261 Ac. 645 & 665 Pine Street Tax Map 19/307/26/A & 38/A Area = 20,002 S.F. / 0.459 Ac. 146 Andover Street Tax Map 19/307/2 Area = 56,005 S.F. / 1.286 Ac. City of Bridgeport County of Fairfield State of Connecticut DATE: January 27, 2022 SCALE: As Noted SHEET: ELEV.1



AUTOMATIC CARWASH TUNNEL ------SWD SUPPLY 150-0." Autor and a state of the state , 2 ° .° <u>FLOOR PLAN</u> SCALE: 1/BILEILOII





CIVIL ENGINEERS LAND SURVEYORS SOIL SCIENTISTS LAND PLANNERS

January 27, 2022

City of Bridgeport Planning & Zoning Commission 45 Lyon Terrace – Room 210 Bridgeport, Connecticut 06604

### Re: Statement of Development Use - Proposed Car Wash Facility 146 Andover Street - Bridgeport, Connecticut 06604

### **Proposed Site Development**

The proposed site development will involve the construction of a car wash facility including a 5,250 square foot automated wash tunnel building, 20 vacuum stations and the typical site amenities and landscaping as indicated in the plans submitted. The proposed traffic routing will be to enter the site from proposed driveway off of Pine Street, into the three vehicle queuing lanes, through the wash tunnel building, out to the vacuum station area and then out of the site onto Andover Street via the propped driveway there. Trash/refuse and recycling collection as well as deliveries to the site will be made via the Pine Street driveway access during off hours and will be the only two-way traffic occurring at this location.

If you have any questions regarding the proposed conceptual site development, please do not hesitate to contact me at (860) 354-9346.

Sincerely, Arthur H. Howland & Associates, P.C.

Christopher A. Francis Senior Civil Engineer / Project Manager

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### **146 ANDOVER ST**

Location	146 ANDOVER ST	Mblu	19/ 307/ 2/ /
Acct#	RE-0029285	Owner	BAR BRIDGEPORT LLC
Assessment	\$716,950	Appraisal	\$1,024,210
PID	2502	Building Count	1

### **Current Value**

	Appraisal					
Valuation Year	Improvements	Land	Total			
2020	\$701,710	\$322,500	\$1,024,210			
Assessment						
Valuation Year	Improvements	Land	Total			

\$491,200

\$225,750

\$716,950

### **Owner of Record**

2020

Owner	BAR BRIDGEPORT LLC	Sale Price	\$1,800,000
Co-Owner		Certificate	
Address	146 ANDOVER ST	Book & Page	10309/262
		Sale Date	11/10/2020
	BRIDGEPORT, CT 06605-2316	Instrument	00

### **Ownership History**

	Ownership	History			
Owner	Sale Price	Certificate	Book & Page	Instrument	Sale Date
BAR BRIDGEPORT LLC	\$1,800,000		10309/262	00	11/10/2020
ANDOVER STREET ASSOCIATES LLC	\$0		4420/0135		09/20/2000
WEST END COMMUNITY DEVELOP	\$0		3873/0128		02/26/1998
EDGE MICHAEL K	\$0		3180/0050		10/20/1993

### **Building Information**

### Building 1 : Section 1

Year Built:	2002
Living Area:	19,800
Replacement Cost:	\$901,575

### Building Percent Good: 69

**Replacement Cost** 

Less Depreciation:

\$622,090

**Building Attributes** 

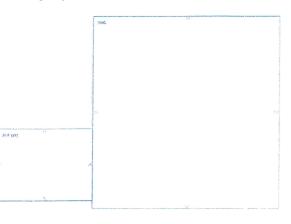
Field Description	
Style:	Pre-Eng Mfg
Model	Ind/Comm
Grade:	Average
Stories:	1
Occupancy:	1.00
Exterior Wall 1:	Pre-Finsh Metl
Exterior Wall 2:	Concr/CinderBI
Roof Struct:	Gable
Roof Cover:	Metal/Tin
Interior Wall 1:	Minim/Masonry
Interior Wall 2:	Drywall
Interior Floor 1:	Concr-Finished
Interior Floor 2:	Carpet
Heating Fuel:	Oil
Heating Type:	Hot Air-No Duc
АС Туре:	None
Struct Class	
Bldg Use:	Manufacturing
Ttl Rooms:	
Ttl Bedrms:	00
Ttl Baths:	0
Ttl Half Baths:	0
Itl Xtra Fix:	0
st Floor Use:	
leat/AC:	None
Frame Type:	Pre-Fab Metal
Baths/Plumbing:	Average
Ceiling/Wall:	Ceil & Min WI
Rooms/Prtns:	Average
Vall Height:	22.00
6 Comn Wall:	

### **Building Photo**



(http://images.vgsi.com/photos2/BridgeportCTPhotos/\00\08 \99\67.JPG)

### **Building Layout**



### (ParcelSketch.ashx?pid=2502&bid=2502)

Building Sub-Areas (sq ft)		Legend	
Code	Description	Gross Area	Living Area
BAS	First Floor	14,400	14,400
AOF	Office	5,400	5,400
		19,800	19,800

### Extra Features

Extra Features			Legend	
Code	Description	Size	Value	Bldg #
A/C	Air Conditioning	5400.00 SF	\$9,690	1
SPR1	Sprinklers-Wet	19800.00 SF	\$38,250	1

### Land

Land Use

### Land Line Valuation

Use Code	343	Size (Acres)	1.29
Description	Manufacturing	Frontage	0
Zone	ILI	Depth	0
Neighborhood	IND	Assessed Value	\$225,750
Alt Land Appr	No	Appraised Value	\$322,500
Category			

### Outbuildings

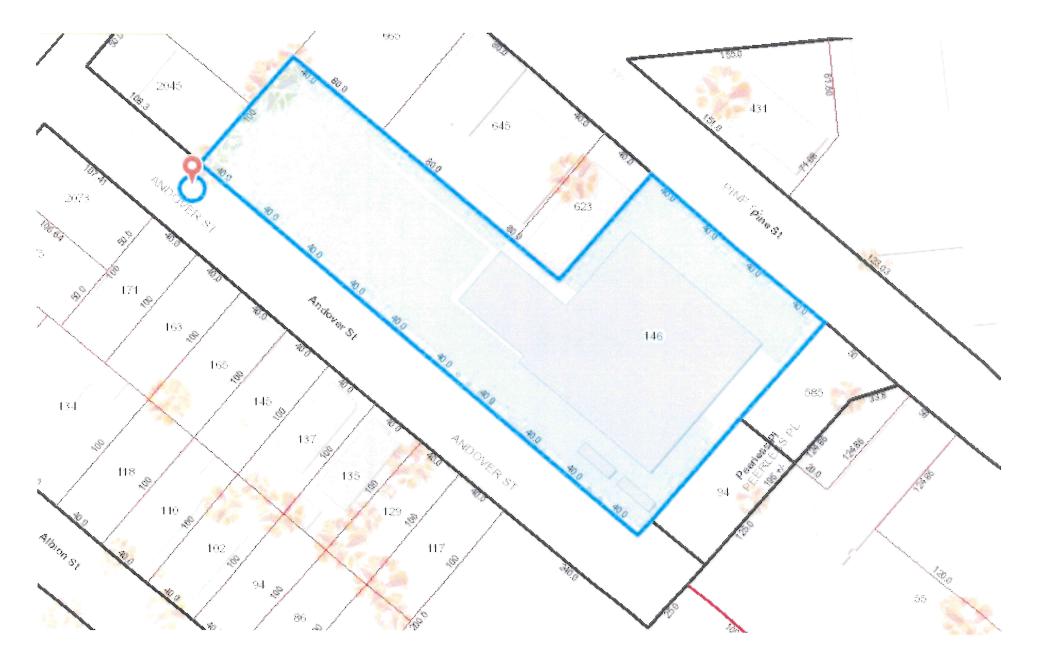
	Outbuildings <u>Legenc</u>			Legend		
Code	Description	Sub Code	Sub Description	Size	Value	Bldg #
FN5	Fence 10'	· · · · · · · · · · · · · · · · · · ·		300.00 LF	\$3,780	1
PAV1	Paving Asph			15000.00 SF	\$27,900	1

### Valuation History

Appraisal			
Valuation Year	Improvements	Land	Total
2019	\$715,680	\$258,000	\$973,680
2018	\$715,680	\$258,000	\$973,680
2017	\$715,680	\$258,000	\$973,680

Assessment			
Valuation Year	Improvements	Land	Total
2019	\$500,970	\$180,600	\$681,570
2018	\$500,970	\$180,600	\$681,570
2017	\$500,970	\$180,600	\$681,570

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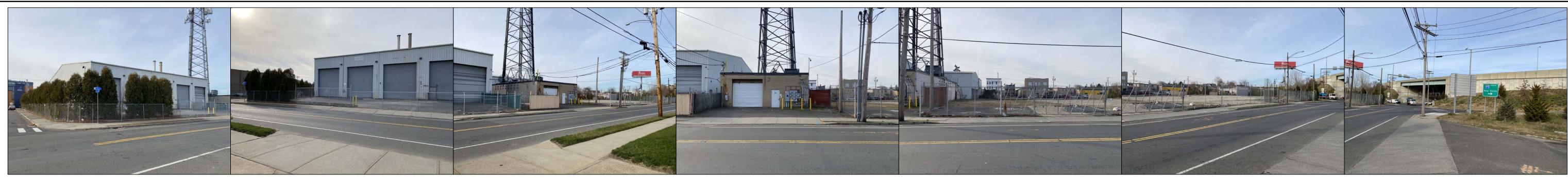


### ADJOINING PROPERTY OWNERS PREPARED FOR 146 ANDOVER STREET BRIDGEPORT, CONNECTICUT

19-307-2	BAR BRIDGEPORT, LLC 146 ANDOVER STREET BRIDGEPORT, CT 06605
19-308-10A	BPT HOUSING AUTHORITY 46 ALBION STREET #56 BRIDGEPORT, CT 06605
19-307-37	CITY OF BRIDGEPORT EXEMPT PARCEL BRIDGEPORT, CT 06605
19-307-10A	CITY OF BRIDGEPORT EXEMPT PARCEL BRIDGEPORT, CT 06605
19-307-1	JAY V PATEL 21 MANOR DRIVE TRUMBULL, CT 06611
19-307-30A	BAR BRIDGEPORT LLC 2033 FAIRFIELD AVE BRIDGEPORT, CT 06605
19-307-38A	BAR BRIDGEPORT LLC 665 PINE STREET BRIDGEPORT, CT 06605
19-307-26A	BAR BRIDGEPORT LLC 645 PINE STREET BRIDGEPORT, CT 06605
19-307-25	ANDREW & LILLIAN KNAPP 24 ROCKDALE ROAD WEST HAVEN , CT 06516
19-306-1	431 CHERRY STREET BRIDGE LLC 431 CHERRY STREET BRIDGEPORT, CT 06605

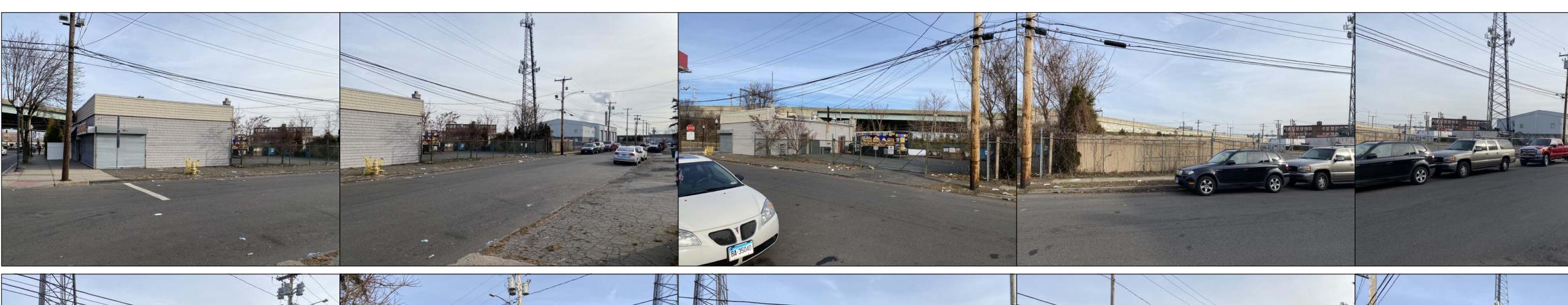
### SUBJECT PROPERTY

19-306-2A	AREC 40 LLC 2727 N CENTRAL AVE PHOENIX, AZ 85004
19-308-34	DRAGONE & SONS LLC 16 PAR LANE TRUMBULL, CT 06611
19-308-33	DRAGONE & SONS LLC 16 PAR LANE TRUMBULL, CT 06611
19-308-32	JOSE TROJILLO 1452 WOOD AVE BRIDGEPORT, CT 06604
19-308-31	WALDORF PROPERTIES LLC 478 ALBANY AVENUE #1 BROOKLY, NY 11203
19-308-36	ALPHA BLACK ROCK LLC 1700 DIXWELL AVE, BLG K, Ste K HAMDEN, CT 06514
19-308-30	ALPHA BLACK ROCK LLC 1700 DIXWELL AVE, BLG K, Ste K HAMDEN, CT 06514
19-308-29	ALPHA BLACK ROCK LLC 1700 DIXWELL AVE, BLG K, Ste K HAMDEN, CT 06514
19-308-37	BAR BRIDGEPORT LLC 129 ANDOVER STREET BRIDGEPORT, CT 06605
19-308-28	BAR BRIDGEPORT, LLC 117 ANDOVER STREET BRIDGEPORT, CT 06605
12-308-39D	JESAJ HOLDINGS LLC 885 CONKLIN STREET FARMINGDALE, NY 11753





Fairfield Avenue Site Frontage - Pine Street to Andover Street





Andover Street & Bird Street Site Frontage - Fairfield Avenue to Bird Street



South Side Andover Street Frontage - Bird Street to Fairfield Avenue

Bird Street & Pine Street Site Frontage - Bird Street to Fairfield Avenue

1) COPIES NOT BEARING THE EMBOSSED SEAL OF THE SURVEYOR OR ENGINEER SHALL BE RENDERED NULL AND VOID.

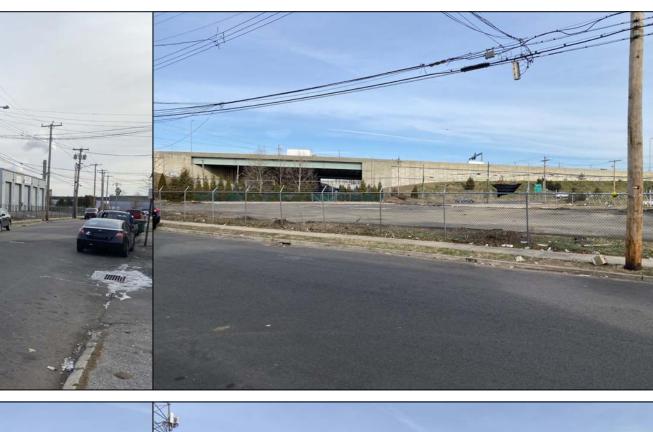
2) REVISIONS TO THESE PLANS BY ANYONE OTHER THAN ARTHUR H. HOWLAND & ASSOC., PC SHALL MAKE THESE PLANS NULL AND VOID. ARTHUR H. HOWLAND & ASSOC., PC SHALL TAKE NO RESPONSIBILITY FOR SAID REVISIONS. EVISION

DEED REFERENCE: 2033 FAIRFIELD AVE: B 10319 PG. 270 645 PINE STREET: B 10309 PG. 266 665 PINE STREET: B 10309 PG. 268 146 ANDOVER STEET: B 10309 PG. 262

PROPERTY IS LOCATED WITHIN THE "ILI" ZONE DISTRICT.

CONTOURS AND ELEVATIONS ARE BASED ON AN NGVD88 DATUM.

ENTIRE PROPERTY IS LOCATED IN FLOOD ZONE AE (EL 12) PER FEMA FLOOD MAP 09001C0437G EFFECTIVE 7-8-2013





**РНОТО.1** 



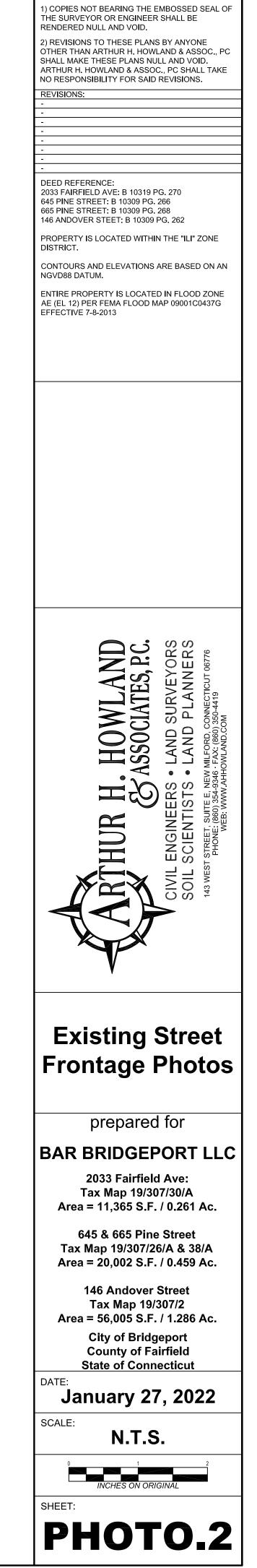
North Side Pine Street Frontage - Fairfield Avenue to Bird Street



West Side Fairfield Avenue Frontage - South to North

East Side Bird Street Frontage - Pine Street to Andover Street





January 25, 2022

BAR Bridgeport, LLC 146 Andover Street Bridgeport, Connecticut 06605

To Whom It May Concern:

Please be advised that the office of Arthur H. Howland & Associates, P.C. is authorized to represent me before any and all agencies and commissions of the City of Bridgeport for the purpose of obtaining approval of any and all land use applications and permits at 146 Andover Street, Bridgeport, Connecticut.

Sincerely Yours,

BAR Bridgeport, LLC Owner JAMES E Balise JR.

O HILL	CITY OF BRIDGEPORT File No
TWAS	PLANNING & ZONING COMMISSION APPLICATION
1.	NAME OF APPLICANT: The Bridgeport Roman Catholic Diocesan Corporation
2.	Is the Applicant's name Trustee of Record? Yes No
3.	If yes, a sworn statement disclosing the Beneficiary shall accompany this application upon filing. Address of Property: <u>238</u> Jewett Avenue / CT / 06606
	(number) (street) (state) (zip code)
4.	Assessor's Map Information: Block No. <u>65/2378</u> Lot No. <u>10/B</u>
	Amendments to Zoning Regulations: (indicate) Article: N/A Section:
	(Attach copies of Amendment)
6.	Description of Property (Metes & Bounds): See submitted survey; 479.48' x 110.08' x 148.33' x 216.41' x 651.33' x 303.65' x 123.00'
7.	Existing Zone Classification:
8.	Zone Classification requested: N/A
9.	Describe Proposed Development of Property: Proposed school use to be located within the existing building on the Site
	Approval(s) requested: Special Permit and Site Plan Review
	Signature:         Date:         12/24/202           Print Name:
	If signed by Agent, state capacity (Lawyer, Developer, etc.) Signature:
	Print Name:
	Mailing Address: Chris Russo, Russo & Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06824
	Phone:         203-528-0590         Cell:         203-520-4603         Fax:         203-255-6618           E-mail Address:         Chris@russorizio.com         Fax:         203-255-6618
	Fee received Date: Clerk:
	THIS APPLICATION MUST BE SUBMITTED IN PERSON AND WITH COMPLETED CHECKLIST
i	Completed & Signed Application Form
i	Completed Site / Landscape Plan
l	■ Written Statement of Development and Use ■ Property Owner's List □ Fee
l	Cert. of Incorporation & Organization and First Report (Corporations & LLC's)
a	PROPERTY OWNER'S ENDORSEMENT OF APPLICATION
-	The Bridgeport Roman Catholic Diocesan Corporation     12/29/2021       Print Owner's Name     Owner's Signature     Date
	Print Owner's Name Owner's Signature Date
-	Print Owner's Name Owner's Signature Date

Lisa S. Broder\* LBroder@russorizio.com

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David K. Kurata DKurata@russorizio.com

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Katherine M. Macol Kathy@russorizio.com

Victoria L. Miller<sup>4</sup> Victoria@russorizio.com

Anthony J. Novella\* Anovella@russorizio.com



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December 29, 2021

Leah M. Parisi Leah@russorizio.com

William M. Petroccio\* WPetro@russorizio.com

> Raymond Rizio\* Ray@russorizio.com

Christopher B. Russo Chris@russorizio.com

> Robert D. Russo<sup>4</sup> Rob@russorizio.com

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Jane Ford Shaw Jane@russorizio.com

Vanessa R. Wambolt Vanessa@russorizio.com

> \* Also Admitted in NY \* Also Admitted in VT \* Of Counsel

Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, CT 06604 **HAND-DELIVERED** 

### Re: Petition for Special Permit and Site Plan Review - 238 Jewett Avenue

Dear Mr. Buckley:

Please accept, on behalf of my client, The Bridgeport Roman Catholic Diocesan Corporation, (the "Petitioner"), the following narrative and enclosed application materials as part of an application for a Special Permit and Site Plan Review under the Bridgeport Zoning Regulations (the "Regulations") for the property located at 238 Jewett Avenue (the "Site") for the interior conversion of a portion of the existing building to support a school use with classrooms and administrative offices and existing off-street parking in the R-A Zone.

### **Narrative**

The Petitioner requests a Special Permit and Site Plan Review under the Regulations for the interior conversion of a portion of the existing building to support a school use with classrooms and administrative offices and existing off-street parking in the R-A Zone. The Site is located on Jewett Avenue in between Madison Avenue and Main Street. For decades, it has been the headquarters of the Petitioner. However, the Petitioner only utilizes a small portion of the existing building. Prior to its current use, the Site had a long history as the location of Notre Dame High School. The Petitioner proposes to return the majority of the existing building to that historical use while maintaining a small office area to support the Diocese. The Petitioner does not propose any physical changes to the Site or the footprint of the building. This change of use will entirely be an interior conversion. A school use is permitted in the R-A Zone.

The Petitioner, in conjunction with Fairfield University, proposes the establishment of a new **Bellarmine College** to offer an **Associate's Degree** to students from low-income and other historically underrepresented backgrounds, primarily in the surrounding Bridgeport region. This

unique model will allow the University to serve students for whom a Fairfield education has not been accessible and to strengthen support and advising services to help ensure the students' retention and debt-free graduation. The Petitioner anticipates that at full capacity, total enrollment will not exceed Two hundred (200) students. Faculty and students will occupy the existing classrooms within the building, which originally served as classrooms for Notre Dame High School.

The Petitioner proposes the creation of Bellarmine College as a new academic unit with its own dedicated faculty, administrative support, and Dean. Bellarmine College would recruit and serve students, primarily from Bridgeport, building upon Fairfield University's current student resources. Bellarmine College will propose a two-year Associate's Degree curriculum designed to provide students with maximum support, so that they will graduate debt-free, on time, and be fully prepared to transfer into a four-year institution should they so choose. The model further distinguishes its approach, following best practices for student retention, something community colleges are not designed to accommodate. Further, a cohesive Associate's Degree offered through Fairfield University guarantees that the majority – if not all – credits will transfer seamlessly into other four-year institutions. At Arrupe College LUC, the foundational model for the proposed Bellarmine College, 55% of students complete the Associate's Degree in two years, 88% of whom then proceed to a four-year institution with 75% of those completing their bachelor's degree.

While the primary goal of this program is to build a curriculum and support that provides students with transferrable credit and skills into a four-year institution, the curriculum is also designed to benefit the 20% of students who choose to stop with the Associate's Degree. Jobs requiring an Associate's Degree in 2017 offered a median income that was 46.3% higher than for jobs requiring a high school diploma. Internships, part-time work placement, and professional development training are built into the Bellarmine College model, providing students with real-world experience and income while completing their studies. Students choosing a career path at the end of their studies will be prepared to enter fields as potential paralegals, teaching assistants, medical or nursing assistants, and entry-level technicians.

Bellarmine College will be test blind and will have the ability to consider a student's potential beyond the typical statistics of a GPA and test score. For students who do not fit the profile of the most competitive students in the nation, Bellarmine College will consider other distinguishable factors that predict student success. All admitted students will be Pell Grant-eligible. Financial aid coupled with part-time employment guarantees that students will have the opportunity to graduate debt-free. While this level of financial support only covers the two-year program, the Bellarmine student support network provides transition guidance to helps graduates navigate and anticipate financial obligations at other institutions, including public v. private and in-state v. out-of-state tuition differentials.

The Site has a long history of serving the residents of the City of Bridgeport. From its days as Notre Dame High School to serving the Catholic community as the headquarters of the Diocese to the proposed Bellarmine College, the Site and its existing building have decades of history serving the local community. The Petition marks the next phase and a tremendous opportunity to improve the lives of Bridgeport residents and students. The Site already features a

large off-street parking area, which can support the proposed use in conformity with the Regulations. The Petitioner is merely looking to convert the interior use of the existing building.

The Petition satisfies the Site Plan Review and Special Permit standards of Sections and 14-2-5 and 14-4-4 of the Regulations. The Petition is in conformity with the Master Plan of Conservation and Development ("POCD"). The Petition proposes no changes to the exterior of the existing building, which has been there for decades. It revitalizes a Site that has become more underutilized as the Diocese's demand for its space has waned. POCD at 125. The building and the Site would benefit from the presence of a new use. The Petition will totally transform and revitalize the Site.

The Petition will not impair future development of the surrounding area, but it will actually stimulate the neighborhood as a landmark property at one of the historic Bridgeport properties. Bellarmine College will reinvigorate the area as it draws students and faculty. It is important to note that students will not be living at the Site. The Petition will clearly have no impact on the Long Island Sound and the proposed use conforms to the residential zone. The proposed use will only enhance surrounding property values as well as the character and operation of the neighborhood. The Site also features adequate off-street parking for the proposed use under the Regulations. The Fairfield University has a longstanding history as an institution of higher learning and, therefore, has the experience to operate the proposed Bellarmine College.

For the reasons stated above, the Petitioner respectfully requests approval of the Petition for a Special Permit.

Sincerely, istopher Russo

# LIST OF PROPERTY OWNERS WITHIN 100' OF 328 JEWETT AVENUE

LOCATION	OWNER	ADDRESS	CITY	STATE ZIP CODE	P CODE
401 JEWETT AV	FRAZIER TIMOTHY	401 JEWETT AVE	BRIDGEPORT	CT 06	06606
291 JEWETT AV	JOHNSON TYRONE A & DOTRICE M	291 JEWETT AVE	BRIDGEPORT	CT 06	06606
280 JEWETT AV	AH JEWETT ACQUISITION LLC C/O MATTHEW FINKLE	60 COLUMBUS CIRCLE	NEW YORK	NY 10	10023
488 PEET ST	WILLIAMS LISA M ET ALS	488 PEET ST	BRIDGEPORT	CT 06	06606
387 JEWETT AV	CANCELLIERI RONALD & MARY ANN	387 JEWETT AVE	BRIDGEPORT	CT 06	06606
311 JEWETT AV	KHAN SHER A & HASHMAT A KHAN	1522 OVERING ST	BRONX	NY 10	10461
444 PEET ST	NIESTEMSKI MAUREEN M	444 PEET ST	BRIDGEPORT	CT 06	06606
406 PEET ST	CAREY JULIE & TIMOTHY E	<b>175 WINDERMERE ST</b>	FAIRFIELD	CT 06	06825
375 JEWETT AV	АКТНЕК МАНАРНИЈА	375 JEWETT AVE	BRIDGEPORT	CT 06	06606
275 JEWETT AV	NORTH END PROPERTY LLC	<b>170 CORNHILL STREET</b>	BRIDGEPORT	CT 06	06606
238 JEWETT AV	BRIDGEPORT ROMAN CATHOLIC DIOCESAN CORPORATION	238 JEWETT AVE	BRIDGEPORT	CT 06	06606
456 PEET ST	UNDERHILL DERRICK & BRENDA M	456 PEET ST	BRIDGEPORT	CT 06	06606
415 JEWETT AV	MARRERO ROBERT SR & SYLVIA Z MARRERO	415 JEWETT AVE	BRIDGEPORT	CT 06	06606
345 GLENDALE AV #A02 VILLARREAL DAVID	VILLARREAL DAVID	345 GLENDALE AVE #A2	BRIDGEPORT	CT 06	06606
<b>380 PEET ST</b>	MIGUEL JOSE & FERNANDES SUSAN	380 PEET ST	BRIDGEPORT	CT 06	06606
405 GLENDALE AV #A03 BORGES BERNARDO	BORGES BERNARDO	485 SAINT JOHNS PL, APT 2A	BROOKLYN	NY 11	11238
287 JEWETT AV	MARTINS JOSE ET AL	287 JEWETT AVE	BRIDGEPORT	CT 06	06606
468 PEET ST	JARRIN JONATHAN P	468 PEET ST	BRIDGEPORT	CT 06	06606
325 JEWETT AV	MICKLE TERI RENE	325 JEWETT AVE	BRIDGEPORT	CT 06	06606
430 PEET ST	WESTPHAL ANA L	430 PEET ST	BRIDGEPORT	CT 06	06606
337 JEWETT AV	SCHNEIDER DAVID P & THERESA A SCHNEIDER	337 JEWETT AVE	BRIDGEPORT	CT 06	06606
418 PEET ST	WOOD PATRICIA BARRETT	418 PEET ST	BRIDGEPORT	CT 06	06606
347 JEWETT AV	SANGIORGI RICHARD	347 JEWETT AVE	BRIDGEPORT	CT 06	06606

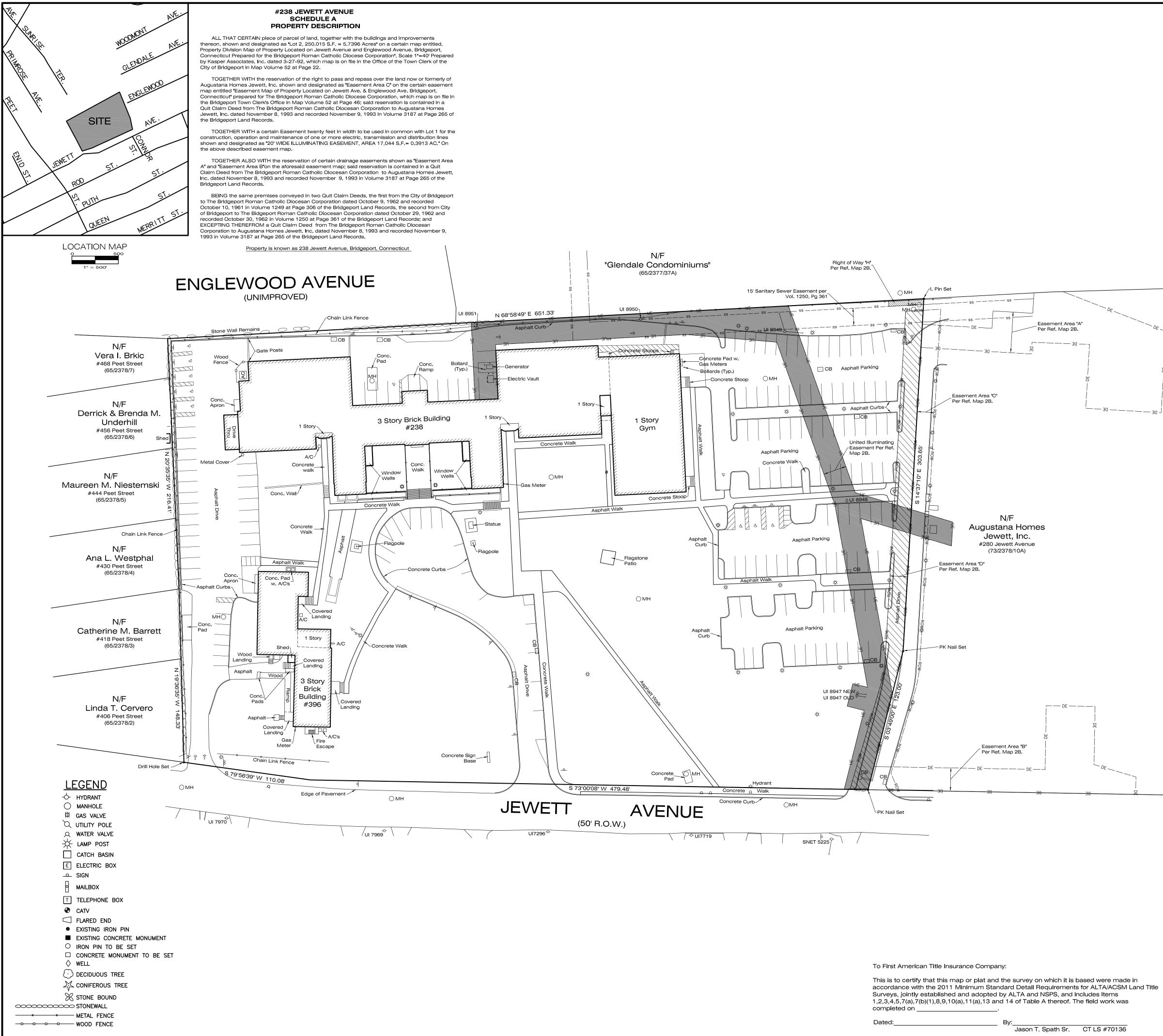
## BRIDGEPORT ROMAN CATHOLIC DIOCESAN CORPORATION, THE ACTIVE

No information provided

BUSINESS DETAILS	~
Business Details	^
General Information	_
Business Name BRIDGEPORT ROMAN CATHOLIC DIOCESAN CORPORATION, THE Business status ACTIVE	
Citizenship/place of formation Domestic/Connecticut	
Business address No information provided	
Annual report due	
NAICS code	
Business ALEI 0191547	
Date formed 11/27/1953	
Business type Religious	
Mailing address	
Last report filed	
NAICS sub code	

Principal Details	
None	
Agent details	_
None	
Filing History	^
Business Formation - Certificate of Incorporation OOOO112477 Filing date: 11/27/1953 Volume Type C Volume 380 Start page 185 Pages O Date generated 11/27/1953	

Name History	^
None	
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### NOTES:

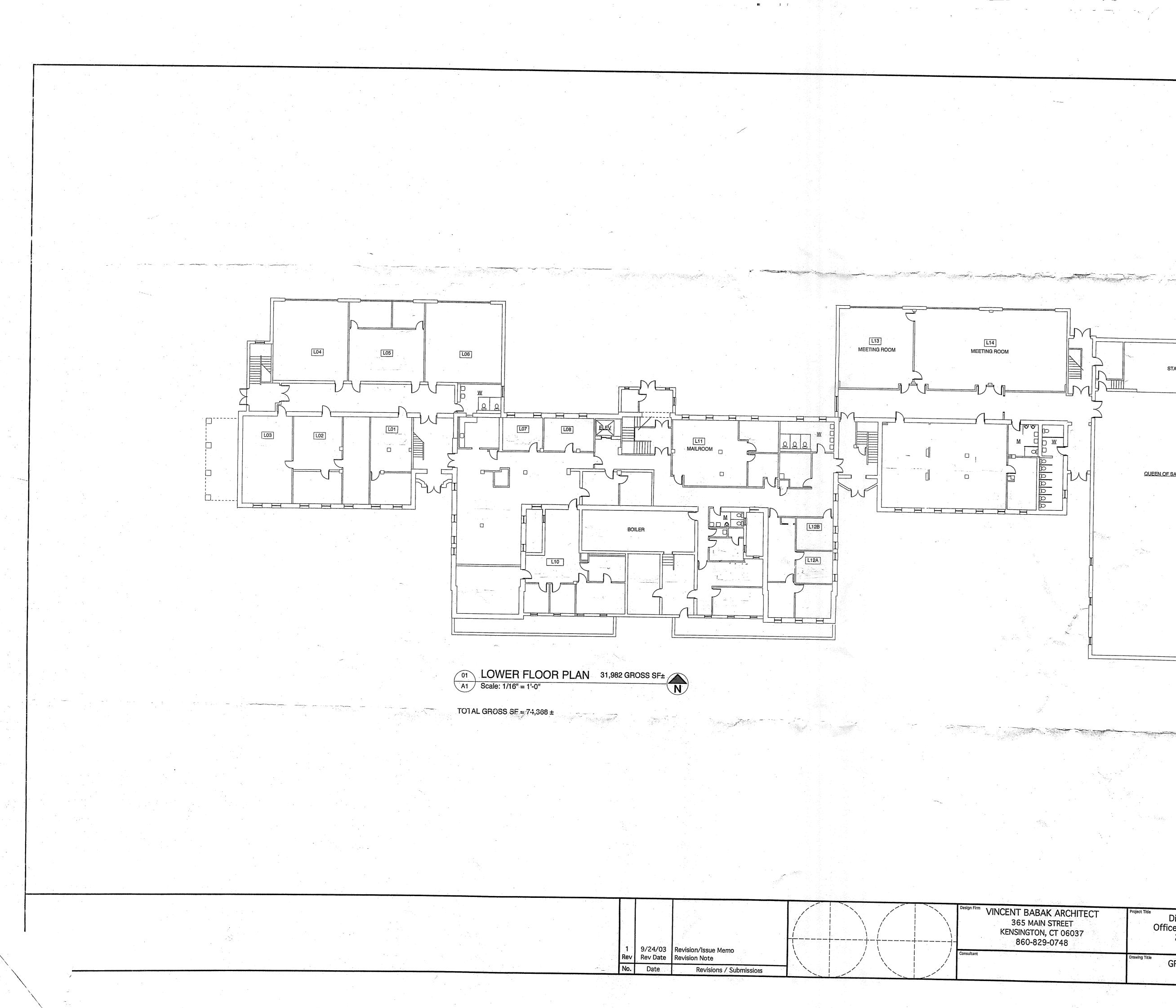
- 1. This survey and map has been prepared in accordance with the Sections 20-300b-1 through 20-300b-20 of the Regulations of Connecticut State Agencies - "Minimum Standards for Survey and Maps in the State of Connecticut<sup>®</sup> as endorsed by the Connecticut Association of Land Surveyors, Inc. It is a Property Survey based upon a Dependent Resurvey and conforms to Horizontal Accuracy Class A-2.
- 2. Reference is made to the following maps on file in the Bridgeport Town Clerk's Office:
  - A. Volume 52 Page 22.
  - B. Volume 52 Page 46.
- 3. Total Area = 5.739 ± Acres / 250,015 ± Sq. Ft.
- 4. Parcel is in Zone R-A.
  - Minimum Lot Area: 5,000 SF
  - Minimum Frontage:50 FT Minimum Depth: 100 FT
  - Minimum Setback:
    - Principal Building: 20 FT Front
  - 6 FT Side (20 FT Combined Sides) 20% of Lot Depth Rear/Minimum 20FT Accessory Structure: The Lesser of 50% of lot depth or 75 FT Front
    - 3 FT Side
  - 3 FT Rear
  - Maximum Building Coverage: 40% Not to exceed 3,000 SF Maximum Lot Coverage: 60%
  - Minimum Landscape Area: 40%
  - Height:
  - Principal Building Maximum: 28 FT to mid-point of highest roof 35 Ft to ridge
  - Accessory Structure Maximum: 12 FT for flat or rounded roof 15 FT to Ridge
- 5. Property is located in FEMA Zone X. Per Flood Insurance Rate Map #09001C0429F, Effective Date: June 18, 2010; Panel 429 of 626.
- 6. All monumentation found or set has been depicted hereon.
- 7. Reference is hereby made to Connecticut General Statute 8-13a, as amended, with regards to existing structures three or more years old.
- 8. This survey was made with the benefit of and is based on First American Title Insurance Company Commitment Order # CTST1795340
- 9. The underground utilities shown, if any, have been located from visible field survey information. The surveyor makes no guarantees that the underground utilities shown comprise all such utilities in the area either in service or abandoned. The surveyor further does not warrant that the underground utilities shown are in the exact location indicated although the surveyor does hereby declare that they are located as accurately as possible from information available. The surveyor has not physically located the underground utilities.
- 10. Property is served by public water supply and sanitary sewer.
- 11. Property has direct physical access to Jewett Avenue, a public street or highway.

Schedule B Exceptions to First American Title Insurance Company Commitment Order #CTST1795340 As to Parcel 3:

- 9. Reservation of a 15' Sewer Right of Way as set forth in Quit Claim Deed from the City of Bridgeport to The Bridgeport Roman Catholic Ciocesan Corporation dated October 29, 1962 and recorded October 30, 1962 in Volume 1250 at Page 361 of the Bridgeport Land Records.
- 10. Grant of Special Exception or Special Permit granted by the Zoning Board of Appeals of the City of Bridgeport dated and recorded March 4, 1980 in Volume 1625 at Page 178 of the Bridgeport Land Records.
- 11. Grant of Special Exception and Variance granted conditionally by the Zoning Board of Appeals of the City of Bridgeport dated August 5, 1992 and recorded August 13, 1992 in Volume 3031 at Page 71 of the Bridgeport Land Records.
- 12. Grant of Special Exception and Variance granted conditionally by the Zoning Board of Appeals of the City of Bridgeport dated September 22, 1993 and recorded September 27, 1993 in Volume 3172 at Page 175 of the Bridgeport Land Records.
- 13. Easement as contained in a Quit Claim Deed from The Bridgeport Roman Catholic Diocesan Corporation to Augustana Homes Jewett, Inc. dated November 8, 1993 and recorded November 9, 1993 in Volume 3187 at Page 265 of the Bridgeport Land Records for the right to pass and repass over "Easement Area D" as shown in Map Voume 52 at Page 46 of the Bridgeport Town Clerk's Office.
- 14. Notes and notations as shown on Map Volume 52 at Page 22 and Map Volume 52 at Page 46 on file in the Bridgeport Town Clerk's Office.

The word "certify" as used hereon is understood to be an expression of professional opinion by the Land Surveyor which is based on his best knowledge, information and belief and as such it constitutes neither a guarantee or warranty.

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<b>—</b>				#238 J BRIDGEPO	EWETT AVE RT, CONNE	_	
			DATE: 11-21-11	SCALE: 1"=40'	DRAFTER: MSS / SJR	JOB NUMBER: 14611	PROJECT #: 14611
NO.	DATE	DESCRIPTION	<b>HC</b>	Cons	UNTINGTON COM ulting Engineers & S Sherman Street, Fair 203.259.1091	Surveyors	1/1
			HCO INFO: M.547	2A P.5			2 2160



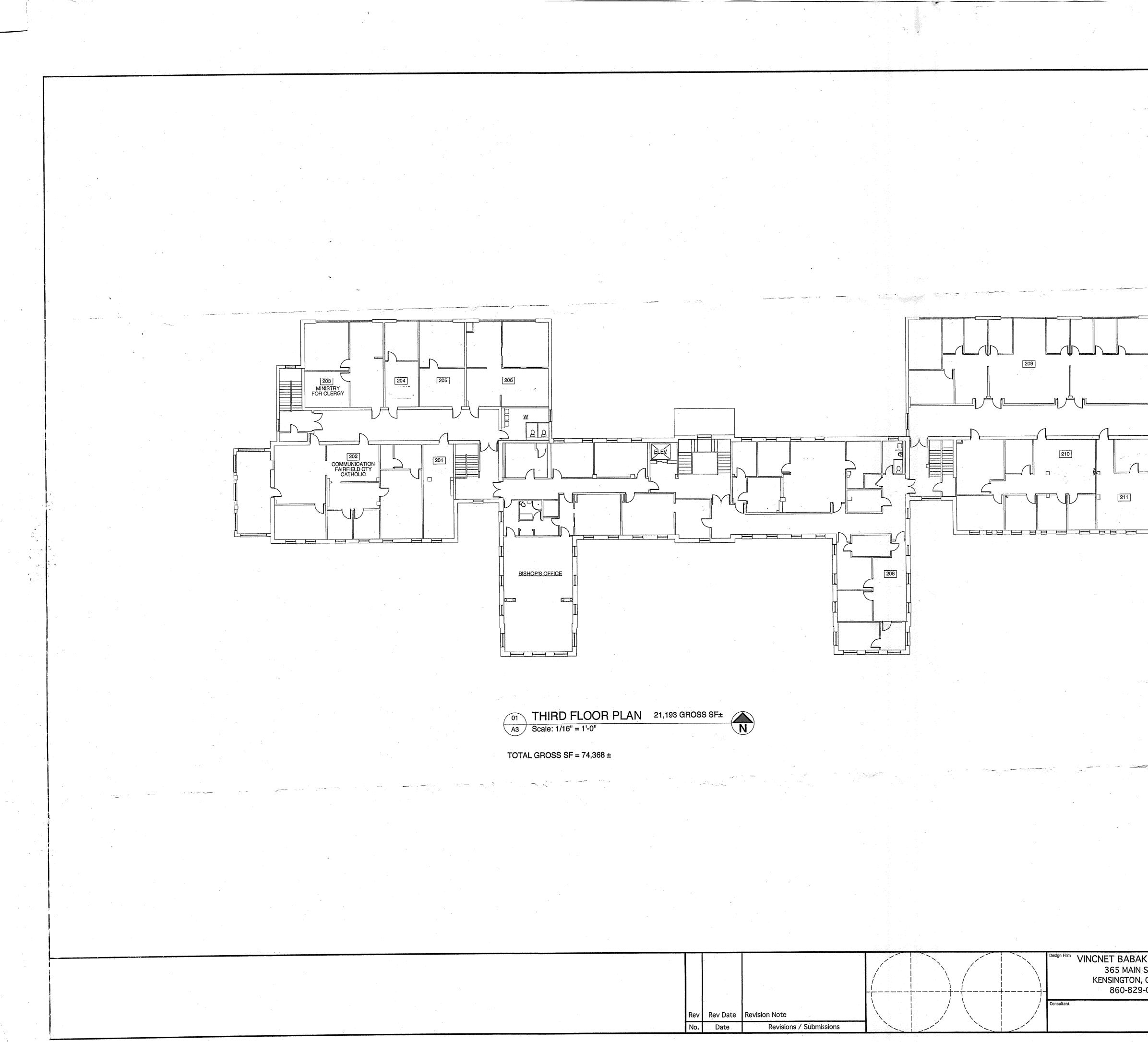
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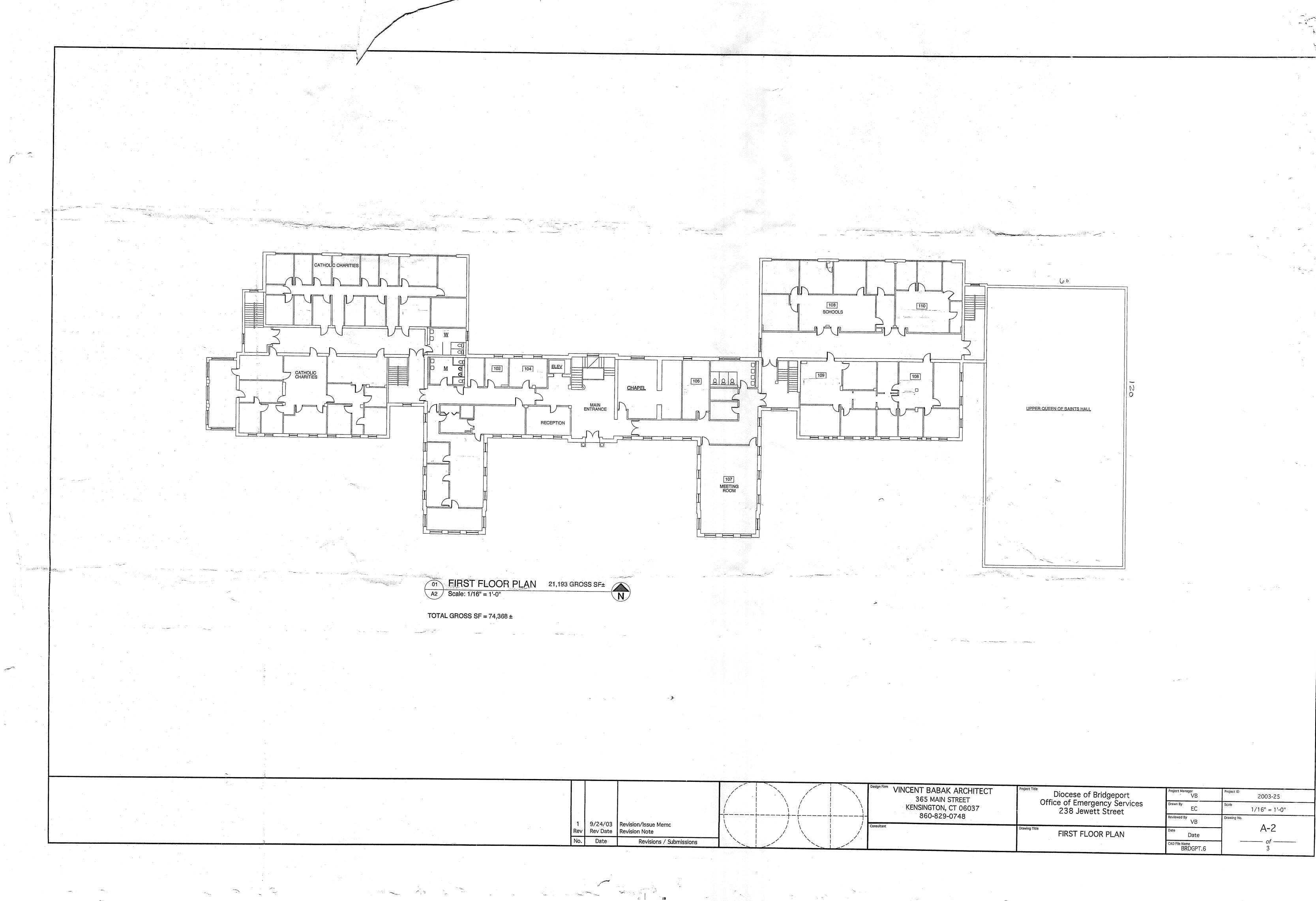
			•
	STAGE		
	QUEEN OF SAINTS HALL		
ARCHITECT STREET CT 06037 0748	Project Title Diocese of Bridgeport Office of Emergency Services 238 Jewett Street	$\begin{array}{c c} Project Manager \\ VB \end{array} \begin{array}{c} Project ID \\ 2003-25 \end{array} \\ \hline \\ Drawn By \\ EC \end{array} \begin{array}{c} Scale \\ 1/16'' = 1'-0'' \end{array} \\ \hline \\ Reviewed By \\ VB \end{array} \begin{array}{c} Drawing No. \end{array} \\ \hline \\ Date \\ OD (24.100000) \end{array} \end{array}$	
	GROUND FLOOR PLAN	09/24/2003         of           CAD File Name         3	



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CARCHITECT STREET CT 06037	Project Title Diocese of Office of Emer 238 Jewe	Bridgeport gency Services ett Street	Project Manager VB Drawn By EC	Scale 1/16"	3-25 = 1'-0"
0748	Drawing Title	_OOR PLAN	Reviewed By VB Date Date	Drawing No.	
	-		CAD File Name BRDGPT.5		9f 3

-



ABAK ARCHITECT	Project Title Diocese of Bridgeport	Project Manager VB	Project ID 2003-25
STON, CT 06037	Office of Emergency Services 238 Jewett Street	Drawn By EC	Scale $1/16'' = 1'-0''$
)-829-0748		Reviewed By VB	Drawing No.
	Drawing Title FIRST FLOOR PLAN	Date Date	- A-2
		CAD File Name BRDGPT.6	of 3

-

E THE	CITY OF BRIDGEPORT File No
SEAL O	PLANNING & ZONING COMMISSION APPLICATION
1.	NAME OF APPLICANT: Crescent Crossings, LLC
2.	Is the Applicant's name Trustee of Record? Yes No X
	If yes, a sworn statement disclosing the Beneficiary shall accompany this application upon filing.
3.	Address of Property: 252 Hallett St / CT / 06608
	(number) (street) (state) (zip code)
	Assessor's Map Information: Block No. <u>43/857</u> Lot No. <u>99</u>
5.	Amendments to Zoning Regulations: (indicate) Article: <u>N/A</u> Section:
	(Attach copies of Amendment)
6.	Description of Property (Metes & Bounds): See submitted survey; 475.71' x 133.43' x 60.91' x 20.87' x 76.15' x 13.32' x 9.65' x 776.90' x 647.07' x 164.92' x 88.48'
7.	Existing Zone Classification: NCVD
3.	Zone Classification requested: N/A
).	Describe Proposed Development of Property: Modification of prior approval to construct of a new mixed-use building containing
	retail use and Eighty-five (85) dwelling units with supporting community spaces and associated Site improvements
	Approval(s) requested: Modification of previously approved Coastal Site Plan Review and Site Plan Review
	Signature: Date: 12/23/2021
	Print Name:
	If signed by Agent, state capacity (Lawyer, Developer, etc.) Signature:
	Print Name:
	Mailing Address: <u>c/o Chris Russo, Russo &amp; Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06824</u>
	Phone: 203-528-0590 Cell: 203-520-4603 Fax:
	E-mail Address: Chris@russorizio.com
	Fee received         Date:         Clerk:
	THIS APPLICATION MUST BE SUBMITTED IN PERSON AND WITH COMPLETED CHECKLIST         Image: Completed & Signed Application Form       Image: A-2 Site Survey       Image: Building Floor Plans
	Completed Site / Landscape Plan     Drainage Plan     Building Elevations
	Written Statement of Development and Use Property Owner's List Fee
	Cert. of Incorporation & Organization and First Report (Corporations & LLC's)
	Mar
	PROPERTY OWNER'S ENDORSEMENT OF APPLICATION
	Crescent Crossings LLC     12/23/2021       Print Owner's Name     Owner's Signature     Date
	Date Date
	Print Owner's Name Owner's Signature Date

Rev. 6/18/2016

Lisa S. Broder\* LBroder@russorizio.com

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David K. Kurata DKurata@russorizio.com

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Victoria L. Miller<sup>+</sup> Victoria@russorizio.com

Anthony J. Novella\* Anovella@russorizio.com



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5 Brook St., Suite 2B, Darien, CT 06820 Tel 203-309-5500

www.russorizio.com

December 23, 2021

Leah M. Parisi Leah@russorizio.com

William M. Petroccio\* WPetro@russorizio.com

Raymond Rizio\* Ray@russorizio.com

Christopher B. Russo Chris@russorizio.com

> Robert D. Russo<sup>4</sup> Rob@russorizio.com

John J. Ryan+ John@russorizio.com

Jane Ford Shaw Jane@russorizio.com

Vanessa R. Wambolt Vanessa@russorizio.com

\* Also Admitted in NY Also Admitted in VT + Of Counsel

Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, CT 06604

### Re: Petition for Coastal Site Plan Review and Site Plan Review - 252 Hallett St.

Dear Mr. Buckley:

Please accept this Petition to the Bridgeport Planning and Zoning Commission for modification of a previously approved Coastal Site Plan Review and Site Plan Review on behalf of my client, Crescent Crossings, LLC, for the property located at 252 Hallett Street(the "Site") in the NCVD Zone.

### Proposed Development & Use

The Petitioner proposes to construct Phase 1C of the Crescent Crossings Development. The Site is located within the NCVD Zone. The Petitioner has existing approvals for the Site for a multi-family residential dwelling. Originally, the project to develop the northeast portion of the Site for the phase of the development at the corner of Crescent Avenue and Waterview Avenue, known as Phase 1C, to construct a multi-family residential dwelling was approved in 2017. Said previous project proposed Ninety-three (93) dwelling units. Subsequently, the project was revised with changes to the number of dwelling units and the addition of commercial floor area. The latest revision was approved in a letter dated October 14, 2020, (included with this submission) which reduced the number of dwelling units in the proposed single building from Eighty-six (86) to Eighty-four (84) units. In preparing the final building permit set of plans, the Petitioner was able to increase the number of dwelling units to Eighty-five (85) units as proposed in this Petition.

This will be the third phase of development following the now constructed first and second phase of Crescent Crossings. The Site has mostly been prepared for construction during the prior construction for the first and second phase. The Petition will be support by ninety-six (96) parking spaces, which is more than compliant under the Bridgeport Zoning Regulations (the "Regulations"). The development will consist of one (1) main multi-family dwelling containing Eighty-one (85) dwelling units, but Two (2) additional future proposed buildings containing six (6) dwelling units each are depicted on the site plan. The proposed building contains numerous entrances for residents to promote a more townhouse/single-family aesthetic to the development. A previous outlet from the proposed Cutter Drive onto Crescent Avenue remains removed from this submission. Ingress and egress onto Cutter Drive will be exclusively from Church Street and Waterview Avenue.

The Petition still provides a significant interior courtyard landscaping and a children's playground for the residents' enjoyment included in the prior submission. The landscaped area for this phase of the development will cover almost forty percent (40%) of the lot area, which significantly exceeds the required minimum of ten percent (10%). Parking is conveniently located in front of the residential dwellings for easy access to the residents and their visitors. The proposed buildings, particularly along Waterview Avenue, retain the prevailing setbacks of the prior development to retain continuity amongst the different phases. The entire Site, including the proposed sidewalks, will be handicap accessible. This portion of the Site will be easily accessed from Hallett Street and Waterview Avenue.

Stormwater will be retained/detained and treated entirely on-site through the use of deep catch basin sumps, a subsurface infiltration system, and a grassed drainage basin. There are no adverse impacts to coastal resources expected as a result of the Petition. The Site falls within a coastal hazard area due to the fact a small portion of the Site is located within the one hundred year (100-yr) floodplain and a larger portion is within the five hundred year (500-yr) floodplain. The Petition will have no effect on the Yellow Mill Channel or Long Island Sound.

The submitted elevations show a variety of materials and colors consistent with apartment design found in the already constructed portion of Crescent Crossings. A significant amount of landscaping will be added to the Site with plantings along the rear property line and street trees along the frontage. Existing structures along the rear property line will also be removed. The Petition will be a tremendous improvement to the Site and neighborhood to provide new construction housing to Bridgeport residents.

### Site Plan Review

The Petition satisfies the Section 14-2-5 Site Plan Review standards of the Regulations. The design of the proposed building and landscaping create a harmonious building-street interaction providing a tremendous improvement to the existing streetscape. The scale and proportion of the buildings conform to the NCVD Zone Development Standards as it is fully compliant with the Regulations. The Petition proposes significant landscaping along the street frontage and on the Site. The proposed multi-family residential dwelling use, its density and parking are permitted in the NCVD Zone. The proposed use and building replace vacant land on an underutilized portion of the Site and continues the amazing progress that has been made on the Site through the Crescent Crossings development. As the Site has already been partially developed for high-density housing, the proposed use will be in conformity as an additional expected phase that has already been approved by the Planning and Zoning Commission.

### **Coastal Site Plan Review**

The Petition also complies with Section 14-3 of the Regulations regarding coastal site plan review. While the Site is located within the coastal boundary, it is not adjacent to a coastal resource. A public street and additional property exist between the coastal resource and the Site. It has no connection to the coastal resource but for being included within its boundary. There are no natural features associated with the coastal resource on the Site. As stated above, the Petition fully complies with the site plan review standards of the Regulations. The Petition poses no danger or threat to coastal resources and it has no potential adverse impacts. The proposed building and Site improvements will all be constructed in accordance with current codes and regulations, including appropriate stormwater drainage systems. Appropriate sediment and erosion controls, such as silt fencing and anti-tracking aprons. The Site has already been approved for coastal site plan review and this Petition is simply changing the number of units within the building.

For these reasons, we respectfully request approval of the Petition to slightly modify a previously approved Coastal Site Plan Review and Site Plan Review to construct a multi-family residential apartment dwelling containing Eighty-five (85) dwelling units with associated Site improvements on the Site in the NCVD Zone.

Sincerely,

Christopher Russo



### City of Bridgeport Zoning Department PLANNING AND ECONOMIC DEVELOPMENT

45 Lyon Terrace • Bridgeport, Connecticut 06604 Telephone (203) 576-7217 Fax (203) 576-7213

October 14, 2020

TODD D. MCCLUTCHY CRESCENT CROSSINGS, 1C, LLC C/O JHM GROUP OF COMPANIES 1266 EAST MAIN STREET, SUITE 601 STAMFORD, CT 06902

### RE: CRESCENT CROSSINGS PHASE 1C – 252 HALLETT STREET

Dear Mr. McClutchy:

On October 14, 2020 at your request an administrative review was conducted regarding Crescent Crossings Phase 1C.

Plan revisions shown on "Building 13 Plans and Elevations" (A-1) dated 10/08/2020 including a decrease from 86 to 84 apartment units in building #13, as well as the increase in ground floor commercial space from approximately 500-sq.ft. to 2,650-sq. ft. are substantially in accordance with the original approval. There is no need for a formal application to the Planning & Zoning Commission for any modification as the project plans remain in compliance with the current Zoning Regulations of the City of Bridgeport, the Development Standards for the NCVD zone, the Site Plan Review Standards, and the Special Permit Standards.

If you have any questions, feel free to contact me at 203-576-7217.

Sincerely,

lul Dennis Buckley,

Zoning Administrator

DB/gb

# LIST OF PROPERTY OWNERS WITHIN 100' OF 252 HALLETT ST

LOCATION	OWNER	Σ
540 CRESCENT AV	HOUSING AUTHORITY CITY OF BPT	÷,
552 CRESCENT AV	UNITED ILLUMINATING CO ATTN: TAX DEPARTMENT	0
252 HALLETT ST	CRESCENT CROSSINGS LLC	T
261 HALLETT ST	BRIDGEPORT HOUSING AUTHORITY	Ϋ́
271 HALLETT ST #273	HOUSING AUTHORITY CITY OF BPT	ŝ
	RIVER SIDE PARK	
221 HALLETT ST	BRIDGEPORT HOUSING AUTHORITY	Ĥ
99 CHURCH ST	SLOVAK ROMAN CATHOLIC CHURCH	7
598 WATERVIEW AV	BRIDGEPORT REDEVELOPMENT AGENCY	4
<b>185 HALLETT ST</b>	BRIDGEPORT HOUSING AUTHORITY	÷
<b>199 HALLETT ST</b>	BRIDGEPORT HOUSING AUTHORITY	÷
576 WATERVIEW AV	BRIDGEPORT REDEVELOPMENT AGENCY	4
209 HALLETT ST	BRIDGEPORT HOUSING AUTHORITY	ij
<b>150 HALLETT ST</b>	BRIDGEPORT PUBLIC SCHOOLS	4
249 HALLETT ST	BRIDGEPORT HOUSING AUTHORITY	H
235 HALLETT ST	BRIDGEPORT HOUSING AUTHORITY	11

MAILING ADDRESS	СІТҮ	STATE ZIP	ZIP
150 HIGHLAND AVE	BRIDGEPORT	С	06604
ONE CITY CTR 5TH FLR	PORTLAND	ME	04101
1281 EAST MAIN ST STE 201	STAMFORD	с С	06902
<b>150 HIGHLAND AVE</b>	BRIDGEPORT	С	06604
376 EAST WASHINGTON AVE	BRIDGEPORT	С	06608
<b>150 HIGHLAND AVE</b>	BRIDGEPORT	Ե	06604
79 CHURCH ST	BRIDGEPORT	сŢ	06608
45 LYON TER	BRIDGEPORT	С	06604
<b>150 HIGHLAND AVE</b>	BRIDGEPORT	сŢ	06604
<b>150 HIGHLAND AVE</b>	BRIDGEPORT	С	06604
45 LYON TER	BRIDGEPORT	С	06604
<b>150 HIGHLAND AVE</b>	BRIDGEPORT	ст	06604
45 LYON TER	BRIDGEPORT	сŢ	06604
<b>150 HIGHLAND AVE</b>	BRIDGEPORT	СТ	06604
150 HIGHLAND AVE	BRIDGEPORT	С	06604

### CRESCENT CROSSINGS, LLC ACTIVE

C/O JHM FINANCIAL GROUP, LLC 1281 EAST MAIN ST STE 201, STAMFORD, CT, 06902, United States

BUSINESS DETAILS V	
Business Details	
General Information	[-]
Business Name CRESCENT CROSSINGS, LLC	
Business status ACTIVE	
Citizenship/place of formation Domestic/Connecticut	
Business address C/O JHM FINANCIAL GROUP, LLC 1281 EAST MAIN ST STE 201, STAMFORD, CT, 06902, United States	
Annual report due 3/31/2022	
NAICS code Other Activities Related to Real Estate (531390)	
Business ALEI 1138922	
Date formed 4/10/2014	
Business type LLC	
Mailing address C/O TOMASETTI KULAS & CO PC 631 FARMINGTON AVE, HARTFORD, CT, 06105, United States	
Last report filed 2021	
NAICS sub code 531390	
Principal Details	_
Principal Name JOHN H. MCCLUTCHY JR.	
Principal Title MANAGER	
Principal Business address 1281 EAST MAIN ST, SUITE 201, STAMFORD, CT, 06902, United States	

### 12/29/21, 3:13 PM

onlineBusinessSearch

^

Principal Residence address 11 MOLLY LANE, DARIEN, CT, 06820, United States

Principal Name TODD D. MCCLUTCHY

Principal Title MANAGER

Principal Business address 1281 EAST MAIN ST, SUITE 201, STAMFORD, CT, 06902, United States

Principal Residence address 158 HOLMES AVE, DARIEN, CT, 06820, United States

Principal Name BRIDGEPORT COMMUNITY RENEWAL GP, LLC

Principal Title MANAGING MEMBER

Principal Business address C/O JHM FINANCIAL GROUP, LLC, 1281 EAST MAIN STREET, SUITE 201, STAMFORD, CT, 06902, United States

### Agent details

Agent name TODD D. MCCLUTCHY

Agent Business address 1281 EAST MAIN ST, SUITE 201, STAMFORD, CT, 06902, United States

Agent Mailing address 1281 EAST MAIN ST, SUITE 201, STAMFORD, CT, 06902, United States

Agent Residence addresss 158 HOLMES AVE , DARIEN, CT, 06820, United States

### **Filing History**

Business Formation - Certificate of Organization OOO5083876 Filing date: 4/10/2014 Volume Type B Volume 1926 Start page 794 Pages 2 Date generated 4/10/2014

Annual Report(2015)



# 54CITY OF BRIDGEPORT

Application Form **Municipal Coastal Site Plan Review** For Projects Located Fully or Partially Within the Coastal Boundary

Please complete this form in accordance with the attached instructions (CSPR-INST-11/99) and submit it with the appropriate plans to the Zoning office.

### Section I: Applicant Identification

Applicant: Crescent Crossings, LLC	Date: 12/23/2021
Address: c/o Russo & Rizio, LLC, 10 Sasco Hill Rd, Fairfield, CT Pr	
Project Address or Location: Northeast portion of 252 Hallett St, Bridgeport, CT	
Interest in Property: $\mathbf{R}$ fee simple $\Gamma$ option $\Gamma$ lessee $\Gamma$ easement	
Γ other (specify)	
List primary contact for correspondence if other than applicant: Name: Chris Russo, Russo & Rizio, LLC	
Address: 10 Sasco Hill Road	
City/Town:Fairfield State: CT	Zip
Code: 06824	
Business Phone: 203-528-0590	
e-mail: <u>Chris@russorizio.com</u>	

### Section II: Project Site Plans

Please provide project site plans that clearly and accurately depict the following information, and check the appropriate boxes to indicate that the plans are included in this application:

K Project location

K Existing and proposed conditions, including buildings and grading

KCoastal resources on and contiguous to the site

 $\Gamma$  High tide line [as defined in CGS Section 22a-359(c)] and mean high water mark elevation contours (for parcels abutting coastal waters and/or tidal wetlands only)

XSoil erosion and sediment controls

K Stormwater treatment practices

K Ownership and type of use on adjacent properties

KReference datum (i.e., National Geodetic Vertical Datum, Mean Sea Level, etc.)

### Section III: Written Project Information

Please check the appropriate box to identify the plan or application that has resulted in this Coastal Site Plan Review:

R Site Plan for Zoning Compliance

 $\Gamma$  Subdivision or Resubdivision

 $\Gamma$  Special Permit or Special Exception

Γ Variance

Γ Municipal Project (CGS Section 8-24)

### Part I: Site Information

1.	Street /	Address	or	Geographical	Description:
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252 Hallett Street

City or Town: Bridgeport

- 2. Is project or activity proposed at a waterfront site (includes tidal wetlands frontage)? Γ YES KNO
- 3. Name of on-site, adjacent or downstream coastal, tidal or navigable waters, if applicable: Yellow Mill Channel and Long Island Sound. There is no adjacent water.
- 4. Identify and describe the existing land use on and adjacent to the site. Include any existing structures, municipal zoning classification, significant features of the project site: The northeast portion of the Site is currently vacant land. The Site is located in the NCVD Zone. The southern portion of the Site contains a multi-family residential apartment building. The remainder of the Site borders vacant land and railroad tracks are located to its North.

5. Indicate the area of the project site:	172,231	acres or square feet (circle one)
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- 6. Check the appropriate box below to indicate total land area of disturbance of the project or activity (please also see Part II.B. regarding proposed stormwater best management practices):
  - Γ Project or activity will disturb 5 or more total acres of land area on the site. It may be eligible for registration for the Department of Environmental Protection's (DEP) General Permit for the Discharge of Stormwater and Dewatering Wastewaters Associated with Construction Activities
  - Project or activity will disturb one or more total acres but less than 5 total acres of land area. A soil erosion and sedimentation control plan must be submitted to the municipal land use agency reviewing this application.
  - Γ Project or activity will not disturb 1 acre total of land area. Stormwater management controls may be required as part of the coastal site plan review.

### Part II.A.: Description of Proposed Project or Activity

Describe the proposed project or activity including its purpose and related activities such as site clearing, grading, demolition, and other site preparations; percentage of increase or decrease in impervious cover over existing conditions resulting from the project; phasing, timing and method of proposed construction; and new uses and changes from existing uses (attach additional pages if necessary):

Petitioner proposes a modification of a prior approval to construct a new mixed-use building with first floor retail space and Eighty-five (85) residential dwelling units with supporting community spaces. The Petitioner will construct a street level parking area to provide sufficient parking for the development. The proposed grading is shown on the submitted plan. The proposed building and site coverage is below the maximum standards of the zone under the Zoning Regulations. The development will be completed in one phase in an anticipated Eighteen (18) months of construction. Demolition will be limited to the removal of existing sidewalks, removal of native vegetation, and removal of any remaining site appurtenances. Other preparation activities include excavation, trenching and grading. Construction activities include building construction, paving, concrete pouring, installation of various site features, utilities & drainage infrastructure, and planting/seeding.

### Part II.B.: Description of Proposed Stormwater Best Management Practices

Describe the stormwater best management practices that will be utilized to ensure that the volume of runoff generated by the first inch of rainfall is retained on-site, especially if the site or stormwater discharge is adjacent to tidal wetlands. If runoff cannot be retained on-site, describe the site limitations that prevent such retention and identify how stormwater will be treated before it is discharged from the site. Also demonstrate that the loadings of total suspended solids from the site will be reduced by 80 percent on an average annual basis, and that post-development stormwater runoff rates and volumes will not exceed pre-development runoff rates and volumes (attach additional pages if necessary):

Storm water run-off will be retained/detained and treated on-site through the use of deep catch basin sumps, a subsurface infiltration system, and a grassed drainage basin.

The primary stormwater treatment will be implemented as to Stormwater Best Management Practice.

### Part III: Identification of Applicable Coastal Resources and Coastal Resource Policies

Identify the coastal resources and associated policies that apply to the project by placing a check mark in the appropriate box(es) in the following table.

Coastal Resources	On-site	Adjacent	Off-site but within the influence of project	Not Applicable
General Coastal Resources* - Definition: CGS Section 22a-93(7); Policy: CGS Section 22a-92(a)(2)	X	X	X	
Beaches & Dunes - Definition: CGS Section 22a-93(7)(C); Policies: CGS Sections 22a-92-(b)(2)(C) and 22a-92(c)(1)(K)				×
Bluffs & Escarpments - Definition: CGS Section 22a-93(7)(A); Policy: CGS Section 22a-92(b)(2)(A)				×
Coastal Hazard Area - Definition: CGS Section 22a-93(7)(H); Policies: CGS Sections 22a-92(a)(2), 22a-92(a)(5), 22a-92(b)(2)(F), 22a- 92(b)(2)(J), and 22a-92(c)(2)(B)	×			
Coastal Waters, Estuarine Embayments, Nearshore Waters, Offshore Waters - Definition: CGS Sections 22a-93(5), 22a-93(7)(G), and 22a- 93(7)(K), and 22a-93(7)(L) respectively; Policies: CGS Sections 22a-92(a)(2) and 22a-92(c)(2)(A)		×	×	
Developed Shorefront - Definition: CGS Section 22a-93(7)(I); Policy: 22a-92(b)(2)(G)				×
Freshwater Wetlands and Watercourses - Definition: CGS Section 22a-93(7)(F); Policy: CGS Section 22a-92(a)(2)		$\times$	X	
Intertidal Flats - Definition: CGS Section 22a-93(7)(D); Policies: 22a-92(b)(2)(D) and 22a-92(c)(1)(K)				$\times$
Islands - Definition: CGS Section 22a-93(7)(J); Policy: CGS Section 22a-92(b)(2)(H)				×
Rocky Shorefront - Definition: CGS Section 22a-93(7)(B); Policy: CGS Section 22a-92(b)(2)(B)				$\times$
Shellfish Concentration Areas - Definition: CGS Section 22a-93(7)(N); Policy: CGS Section 22a-92(c)(1)(I)				×
Shorelands - Definition: CGS Section 22a-93(7)(M); Policy: CGS Section 22a-92(b)(2)(I)				×
Tidal Wetlands - Definition: CGS Section 22a-93(7)(E); Policies: CGS Sections 22a-92(a)(2), 22a-92(b)(2)(E), and 22a- 92(c)(1)(B)				×

\* General Coastal Resource policy is applicable to all proposed activities

### Part IV: Consistency with Applicable Coastal Resource Policies and Standards

Describe the location and condition of the coastal resources identified in Part III above and explain how the proposed project or activity is consistent with all of the applicable coastal resource policies and standards; also see adverse impacts assessment in Part VII.A below (attach additional pages if necessary): Coastal resources will be preserved since there are no adverse impacts to coastal resources expected as a result of the proposed work. The Site falls within a coastal hazard area since a small portion of the Site is within the 100-year floodplain and a larger portion within the 500-year floodplain. Erosion & sediment control measures will be used during work. The proposed project complies with CGS Sec. 22a-92(a)(1) "...by promoting economic growth without significantly disrupting the environment...", with CGS Sec. 22a-92(b)(2)(F) "...manage coastal hazard areas to minimize hazards to property..." and with CGS Sec. 22a-92(c)(2)(B) ...maintain patterns of water circulation in the placement of drainage control structures..."

### Part V: Identification of Applicable Coastal Use and Activity Policies and Standards

Identify all coastal policies and standards in or referenced by CGS Section 22a-92 applicable to the proposed project or activity:

- X General Development\* CGS Sections 22a-92(a)(1), 22a-92(a)(2), and 22a-92(a)(9)
- 9 Water-Dependent Uses\*\* CGS Sections 22a-92(a)(3) and 22a-92(b)(1)(A); Definition CGS Section 22a-93(16)
- 9 Ports and Harbors CGS Section 22a-92(b)(1)(C)
- 9 Coastal Structures and Filling CGS Section 22a-92(b)(1)(D)
- 9 Dredging and Navigation CGS Sections 22a-92(c)(1)(C) and 22a-92(c)(1)(D)
- 9 Boating CGS Section 22a-92(b)(1)(G)
- 9 Fisheries CGS Section 22a-92(c)(1)(l)
- 9 Coastal Recreation and Access CGS Sections 22a-92(a)(6), 22a-92(C)(1)(j) and 22a-92(c)(1)(K)
- Sewer and Water Lines CGS Section 22a-92(b)(1)(B)
- 9 Fuel, Chemicals and Hazardous Materials CGS Sections 22a-92(b)(1)(C), 22a-92(b)(1)(E) and 22a-92(c)(1)(A)
- 9 Transportation CGS Sections 22a-92(b)(1)(F), 22a-92(c)(1)(F), 22a-92(c)(1)(G), and 22a-92(c)(1)(H)
- 9 Solid Waste CGS Section 22a-92(a)(2)
- 9 Dams, Dikes and Reservoirs CGS Section 22a-92(a)(2)
- 9 Cultural Resources CGS Section 22a-92(b)(1)(J)
- 9 Open Space and Agricultural Lands CGS Section 22a-92(a)(2)

\* General Development policies are applicable to all proposed activities

<sup>\*\*</sup> Water-dependent Use policies are applicable to all activities proposed at waterfront sites, including those with tidal wetlands frontage.

### Part VI: Consistency With Applicable Coastal Use Policies And Standards

Explain how the proposed activity or use is consistent with all of the applicable coastal use and activity policies and standards identified in Part V. For projects proposed at waterfront sites (including those with tidal wetlands frontage), particular emphasis should be placed on the evaluation of the project's consistency with the water-dependent use policies and standards contained in CGS Sections 22a-92(a)(3) and 22a-92(b)(1)(A) -- also see adverse impacts assessment in Part VII.B below (attach additional pages if necessary):

No adverse impacts were determined on adjacent coastal resources. Stormwater treatment is proposed which will help reduce

erosion impacts as well as provide water infiltration. This project will be limited to the confines of the Site and will be completed within

Eighteen (18) months. All disturbed pervious areas will be loamed, seeded and planted upon completion of construction.

Erosion & sediment controls will be in place during construction. Sanitary sewer and water lines are proposed to encourage

concentrated development within areas that are suitable and appropriate for development.

### Part VII.A.: Identification of Potential Adverse Impacts on Coastal Resources

### Please complete this section for all projects.

Identify the adverse impact categories below that apply to the proposed project or activity. The Aapplicable≅ column **must** be checked if the proposed activity has the **potential** to generate any adverse impacts as defined in CGS Section 22a-93(15). If an adverse impact may result from the proposed project or activity, please use Part VIII to describe what project design features may be used to eliminate, minimize, or mitigate the potential for adverse impacts.

Potential Adverse Impacts on Coastal Resources	Applicable	Not Applicable
Degrading tidal wetlands, beaches and dunes, rocky shorefronts, and bluffs and escarpments through significant alteration of their natural characteristics or functions - CGS Section 22a-93(15)(H)		×
Increasing the hazard of coastal flooding through significant alteration of shoreline configurations or bathymetry, particularly within high velocity flood zones - CGS Section 22a-93(15)(E)		×
Degrading existing circulation patterns of coastal water through the significant alteration of patterns of tidal exchange or flushing rates, freshwater input, or existing basin characteristics and channel contours - CGS Section 22a-93(15)(B)		×
Degrading natural or existing drainage patterns through the significant alteration of groundwater flow and recharge and volume of runoff - CGS Section 22a-93(15)(D)		×
Degrading natural erosion patterns through the significant alteration of littoral transport of sediments in terms of deposition or source reduction - CGS Section 22a-93(15)(C)		×
Degrading visual quality through significant alteration of the natural features of vistas and view points - CGS Section 22a-93(15)(F)		×
Degrading water quality through the significant introduction into either coastal waters or groundwater supplies of suspended solids, nutrients, toxics, heavy metals or pathogens, or through the significant alteration of temperature, pH, dissolved oxygen or salinity - CGS Section 22a-93(15)(A)		×
Degrading or destroying essential wildlife, finfish, or shellfish habitat through significant alteration of the composition, migration patterns, distribution, breeding or other population characteristics of the natural species or significant alterations of the natural components of the habitat - CGS Section 22a-93(15)(G)		×

### Part VII.B.: Identification of Potential Adverse Impacts on Water-dependent Uses

Please complete the following two sections only if the project or activity is proposed at a waterfront site:

 Identify the adverse impact categories below that apply to the proposed project or activity. The Aapplicable≅ column **must** be checked if the proposed activity has the **potential** to generate any adverse impacts as defined in CGS Section 22a-93(17). If an adverse impact may result from the proposed project or activity, use Part VIII to describe what project design features may be used to eliminate, minimize, or mitigate the potential for adverse impacts.

Potential Adverse Impacts on Future Water-dependent Development Opportunities and Activities	Applicable	Not Applicable
Locating a non-water-dependent use at a site physically suited for or planned for location of a water-dependent use - CGS Section 22a-93(17)		×
Replacing an existing water-dependent use with a non-water- dependent use - CGS Section 22a-93(17)		×
Siting a non-water-dependent use which would substantially reduce or inhibit existing public access to marine or tidal waters - CGS Section 22a-93(17)		×

2. Identification of existing and/or proposed Water-dependent Uses

Describe the features or characteristics of the proposed activity or project that qualify as water- dependent uses as defined in CGS Section 22a-93(16). If general public access to coastal waters is provided, please identify the legal mechanisms used to ensure public access in perpetuity, and describe
any provisions for parking or other access to the site and proposed amenities associated with the access
(e.g., boardwalk, benches, trash receptacles, interpretative signage, etc.)*: There is no proposed activity that will qualify as a water-dependent use as there is no
adjacent water to the Site.

\*If there are no water-dependent use components, describe how the project site is not appropriate for the development of a water-dependent use.

### Part VIII: Mitigation of Potential Adverse Impacts

Explain how all potential adverse impacts on coastal resources and/or future water-dependent development opportunities and activities identified in Part VII have been avoided, eliminated, or minimized (attach additional pages if necessary): No adverse impacts were determined on adjacent coastal resources. Stormwater treatment

is proposed which will help reduce erosion impacts as well as provide water infiltration.

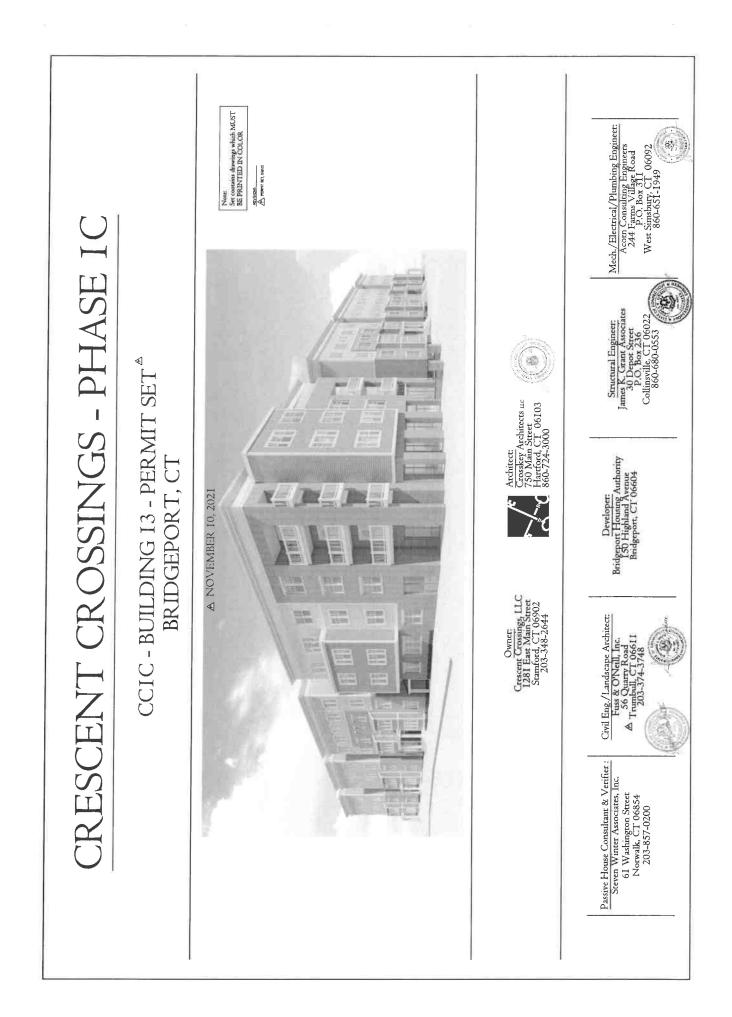
### Part IX: Remaining Adverse Impacts

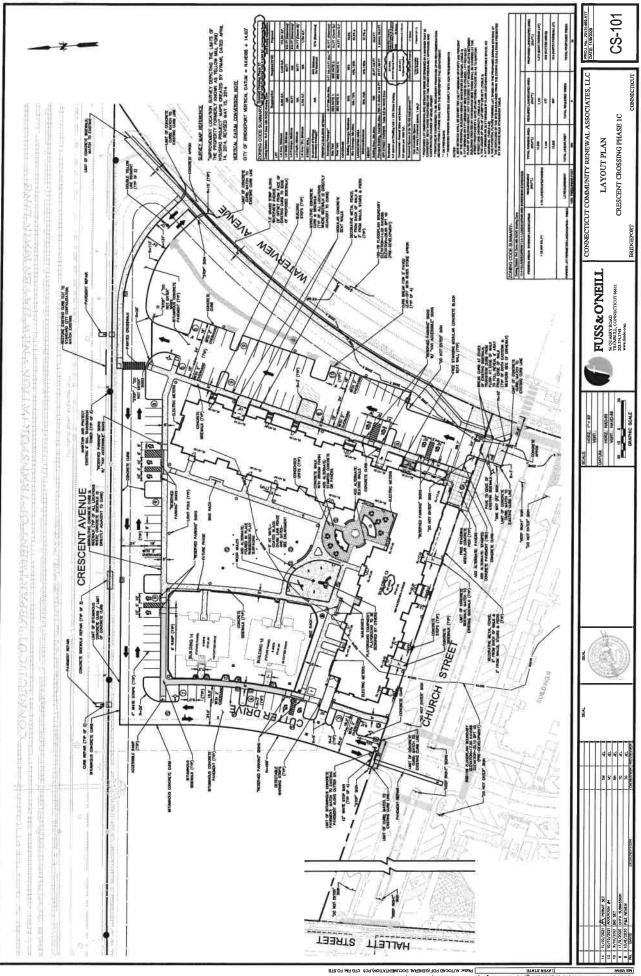
Explain why any remaining adverse impacts resulting from the proposed activity or use have not been mitigated and why the project as proposed is consistent with the Connecticut Coastal Management Act

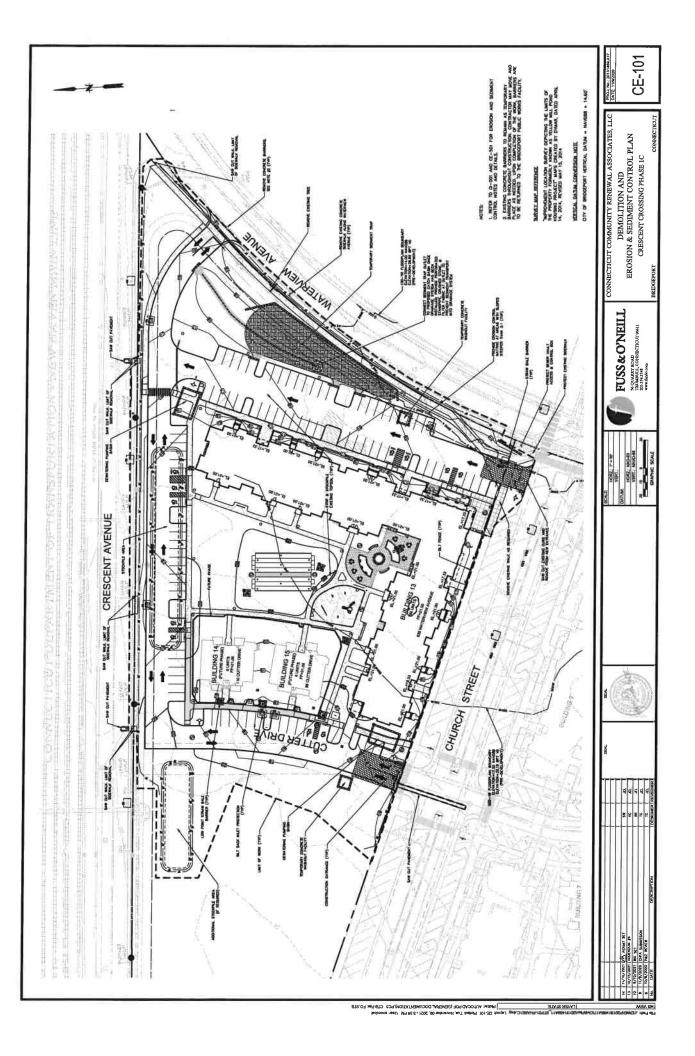
(attach additional pages if necessary):

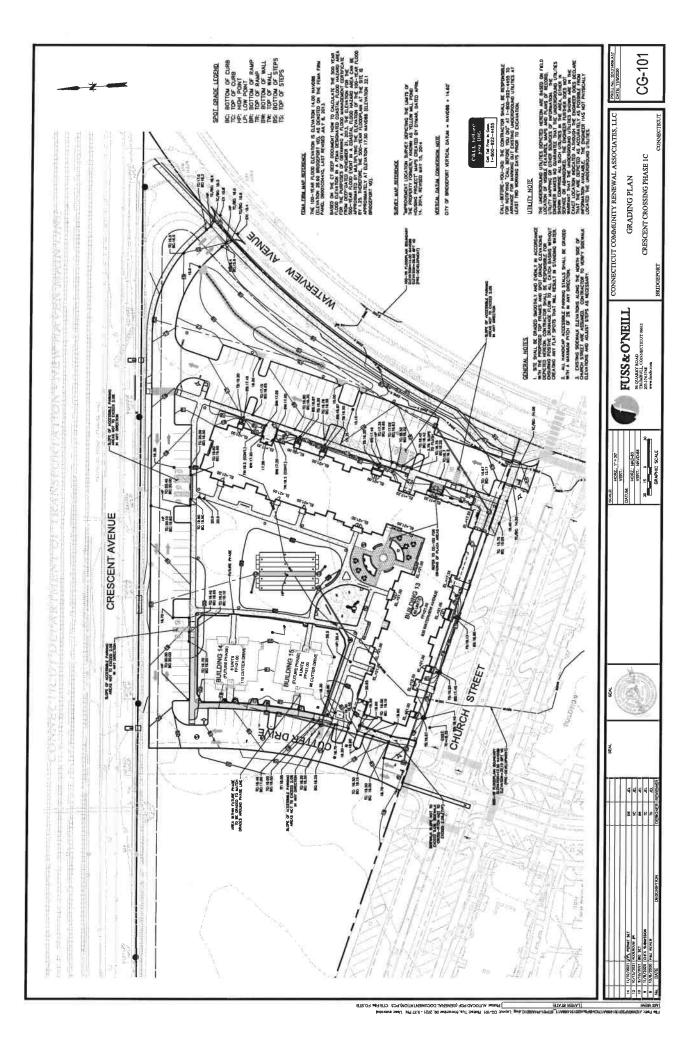
There will be no remaining adverse impacts resulting from the proposed activity.

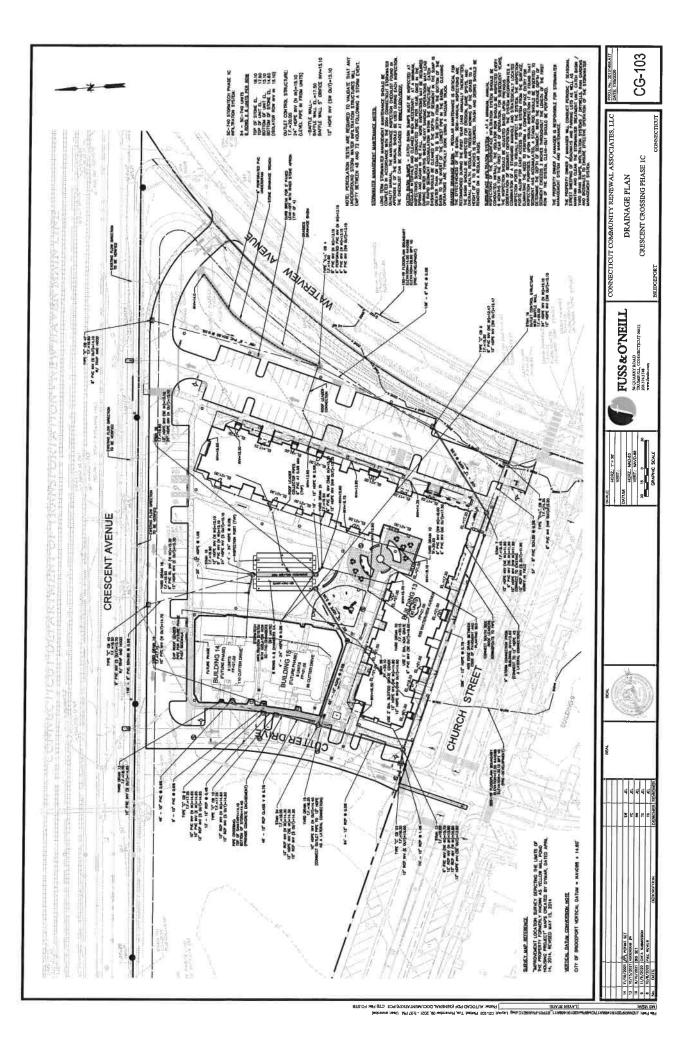
Model Municipal CSPR.doc

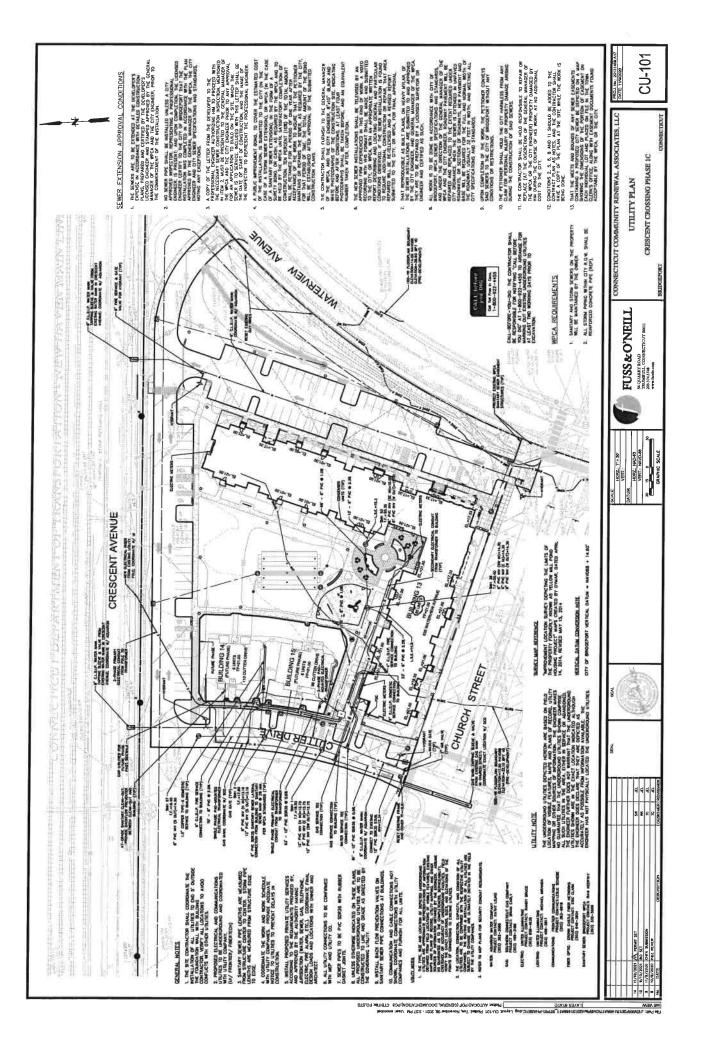


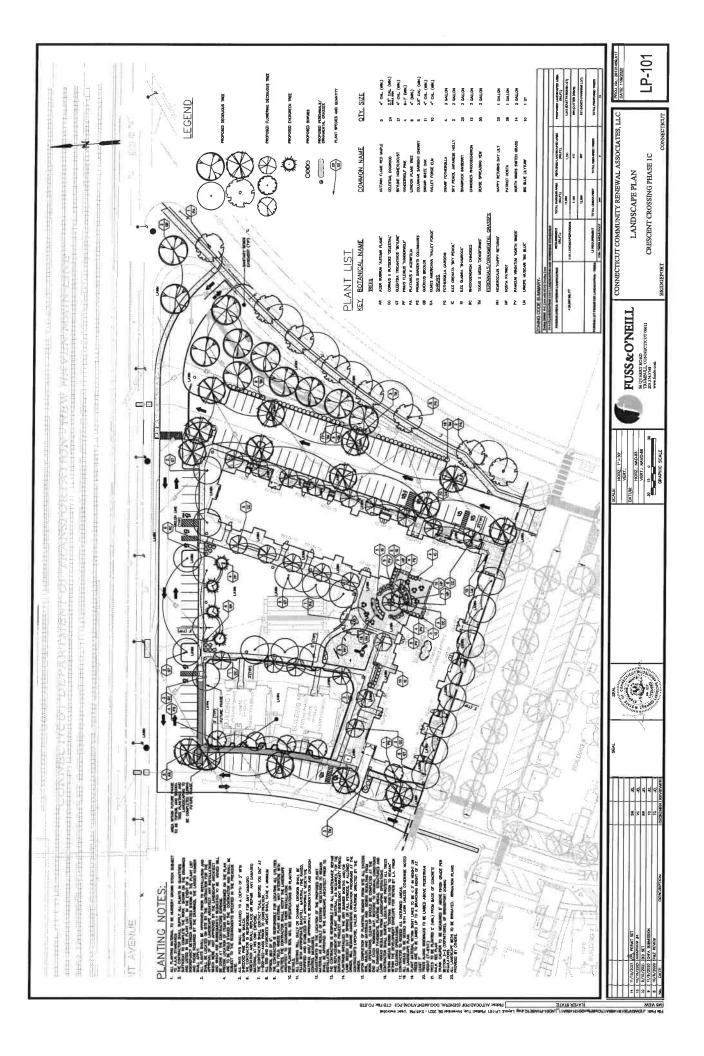


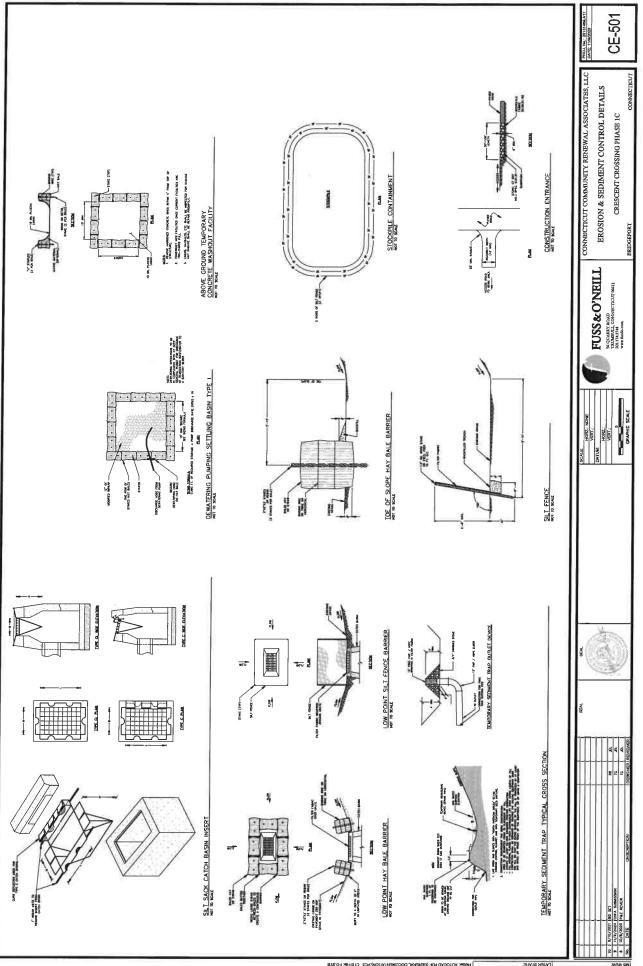




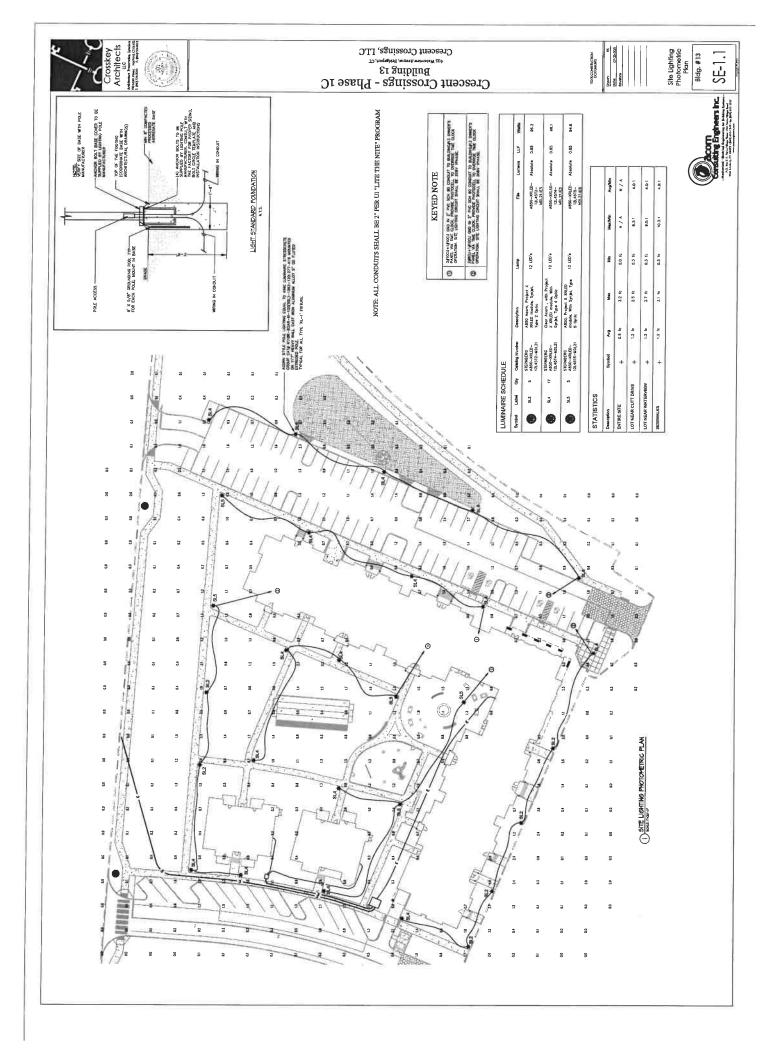


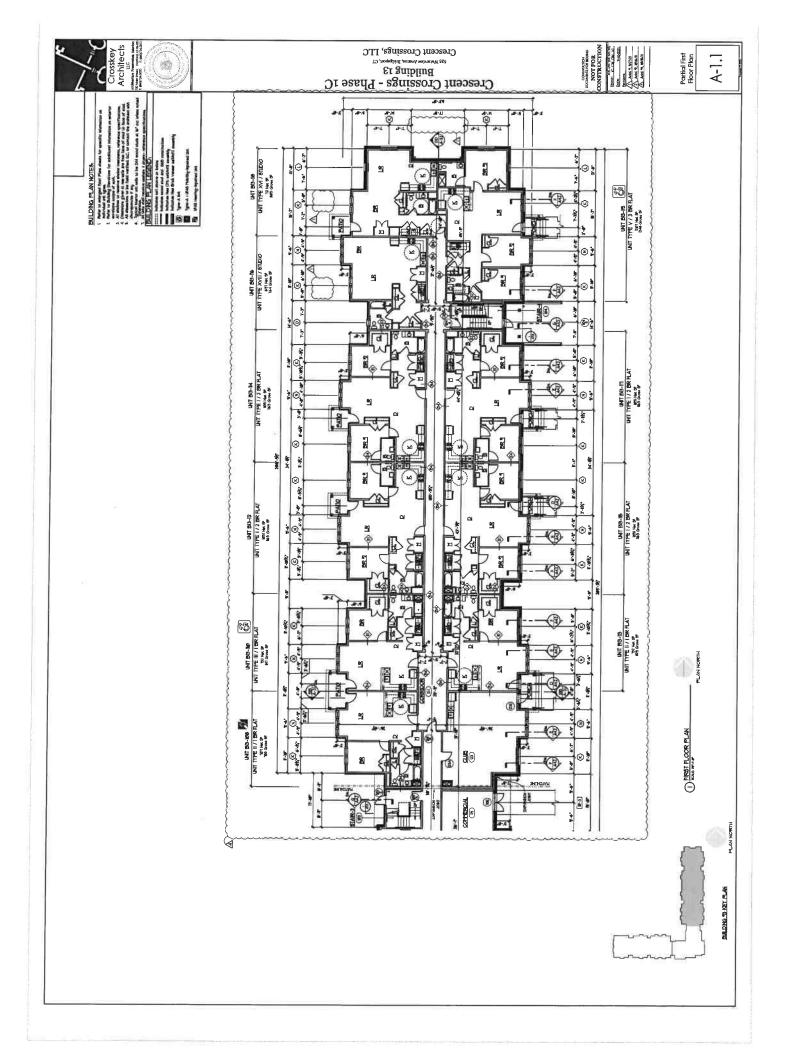


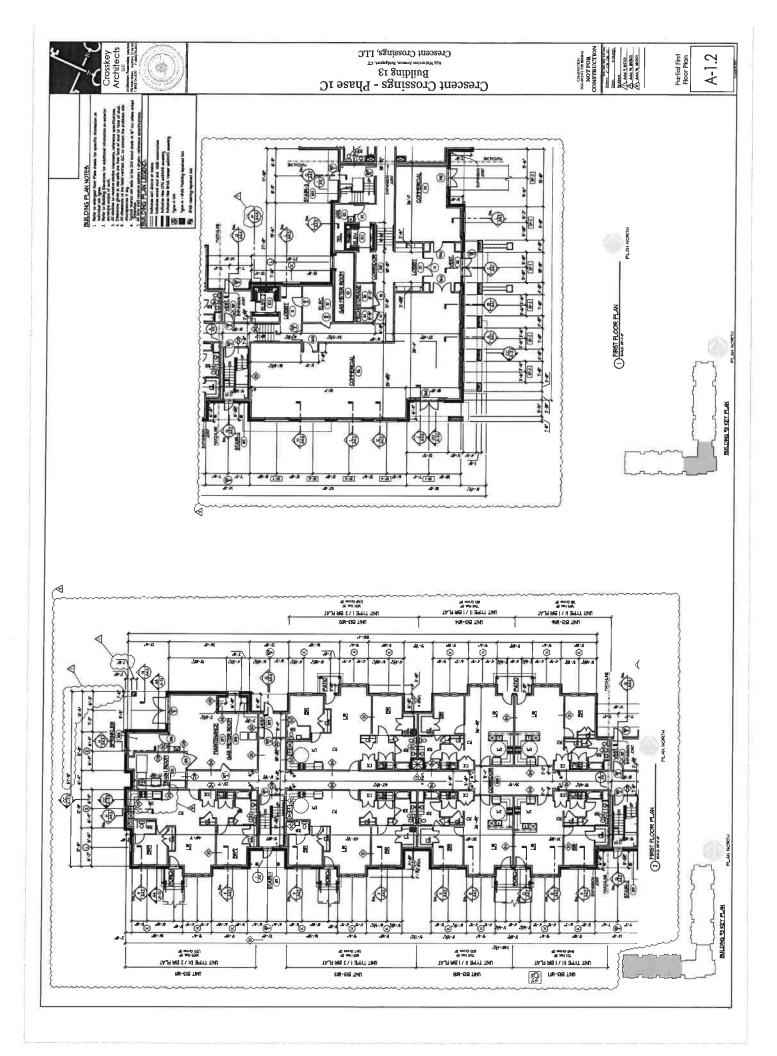


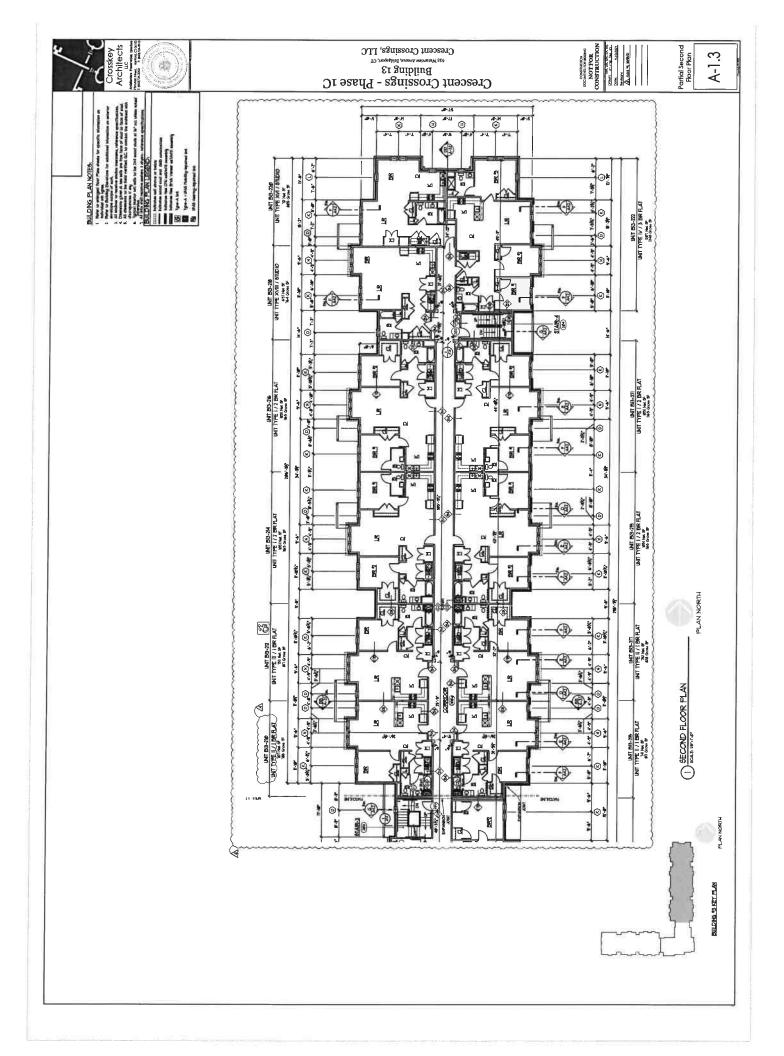


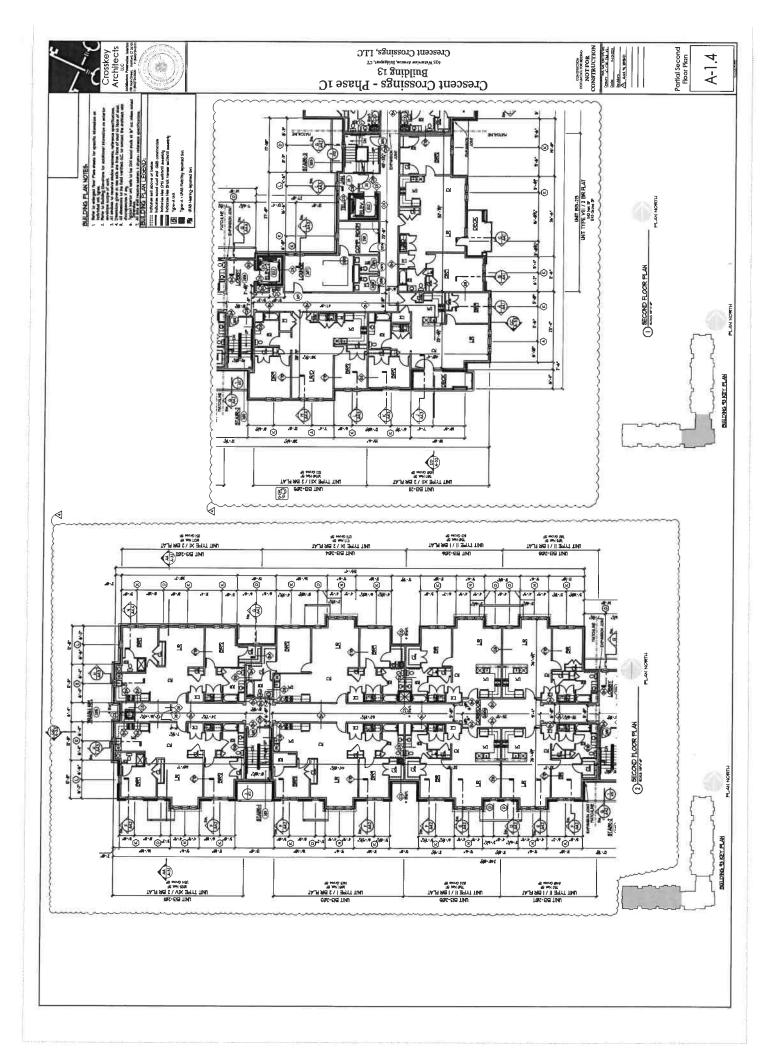
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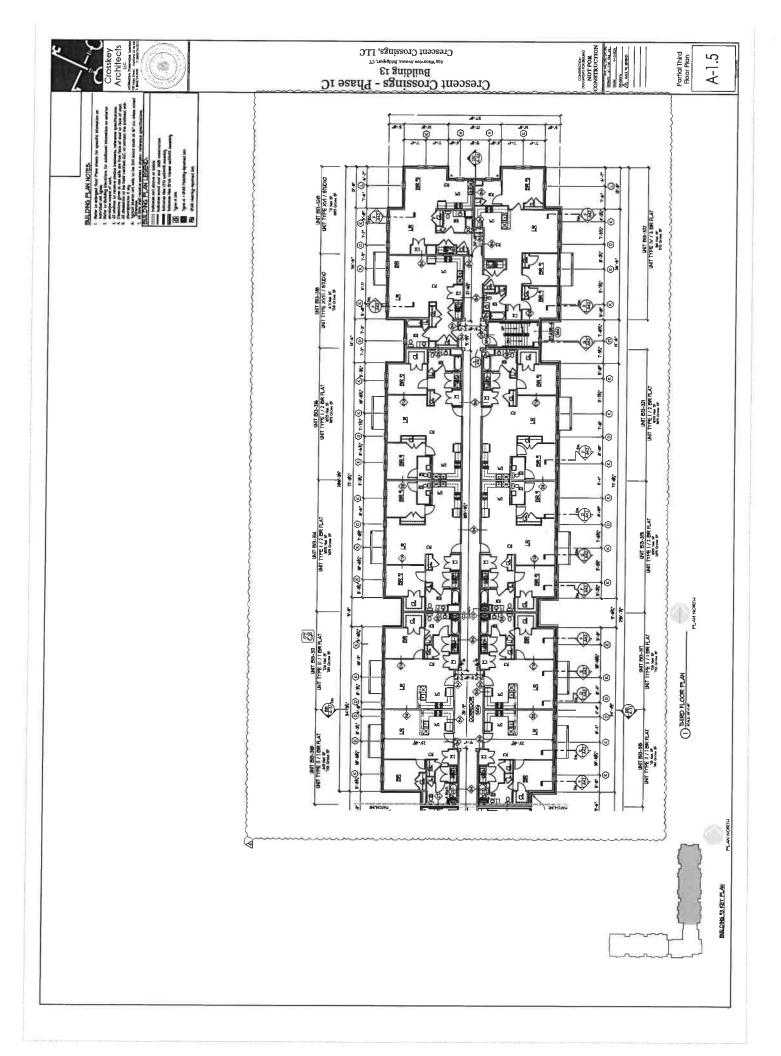


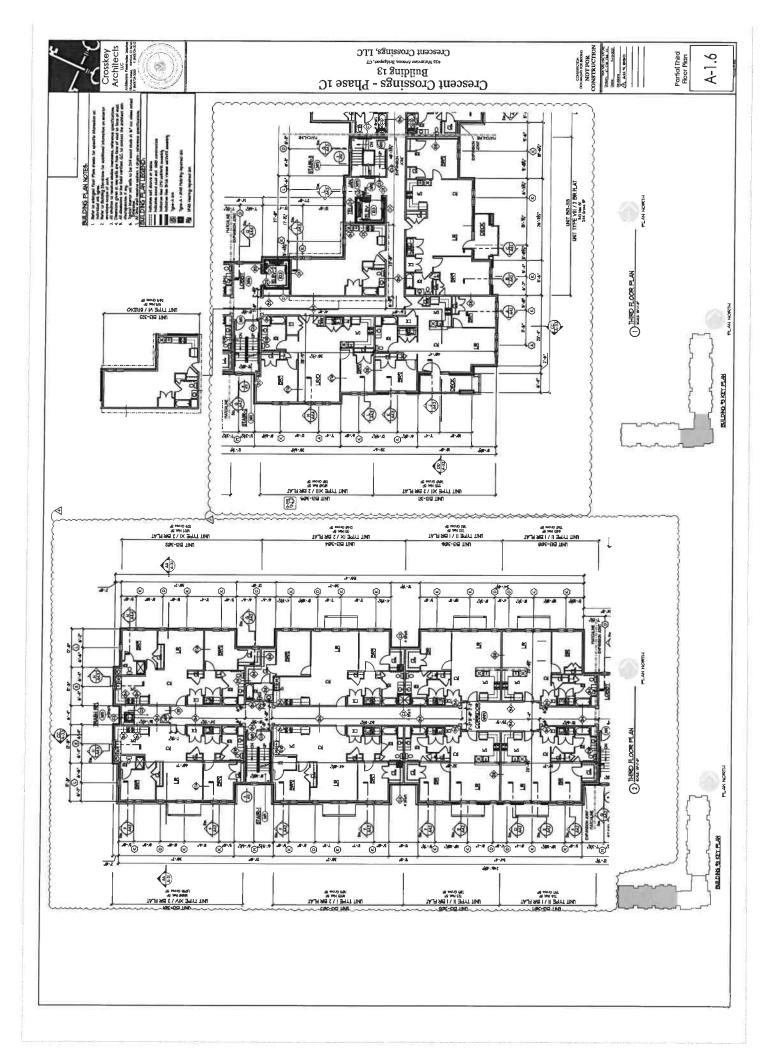


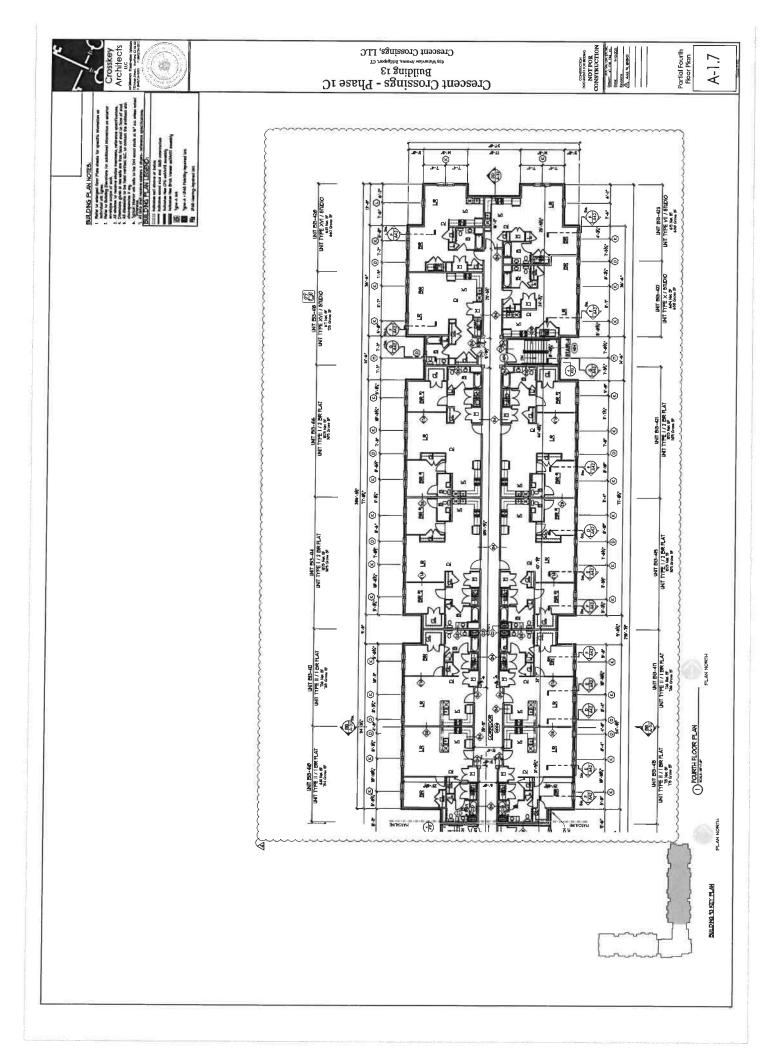


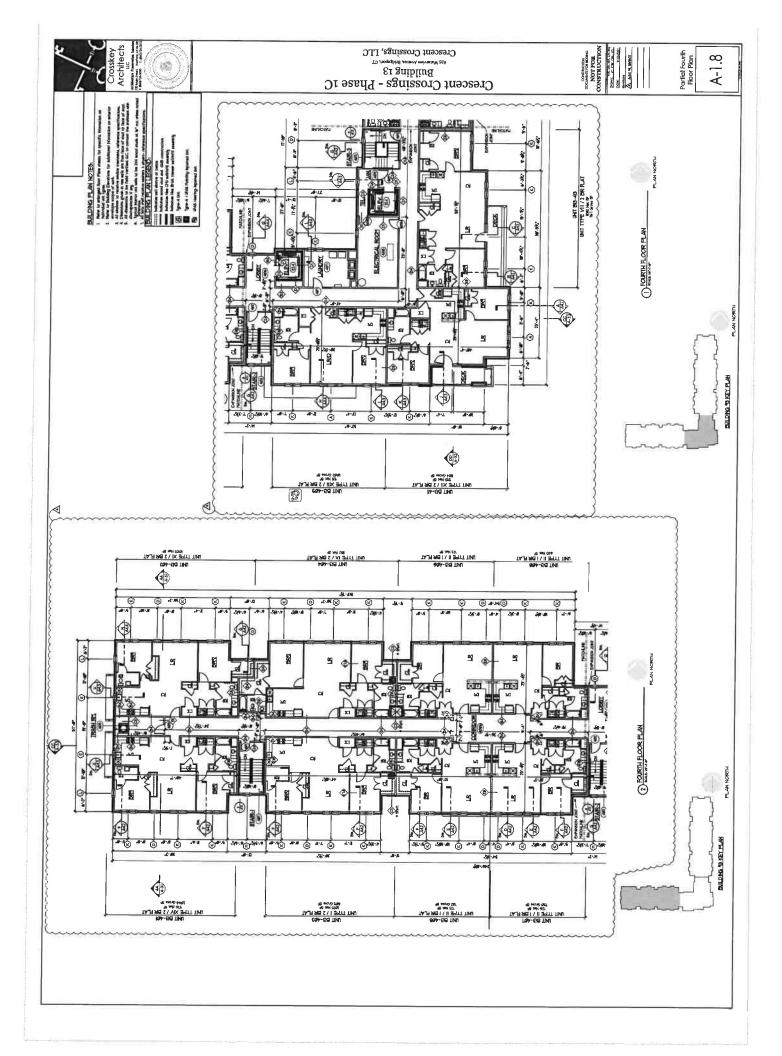


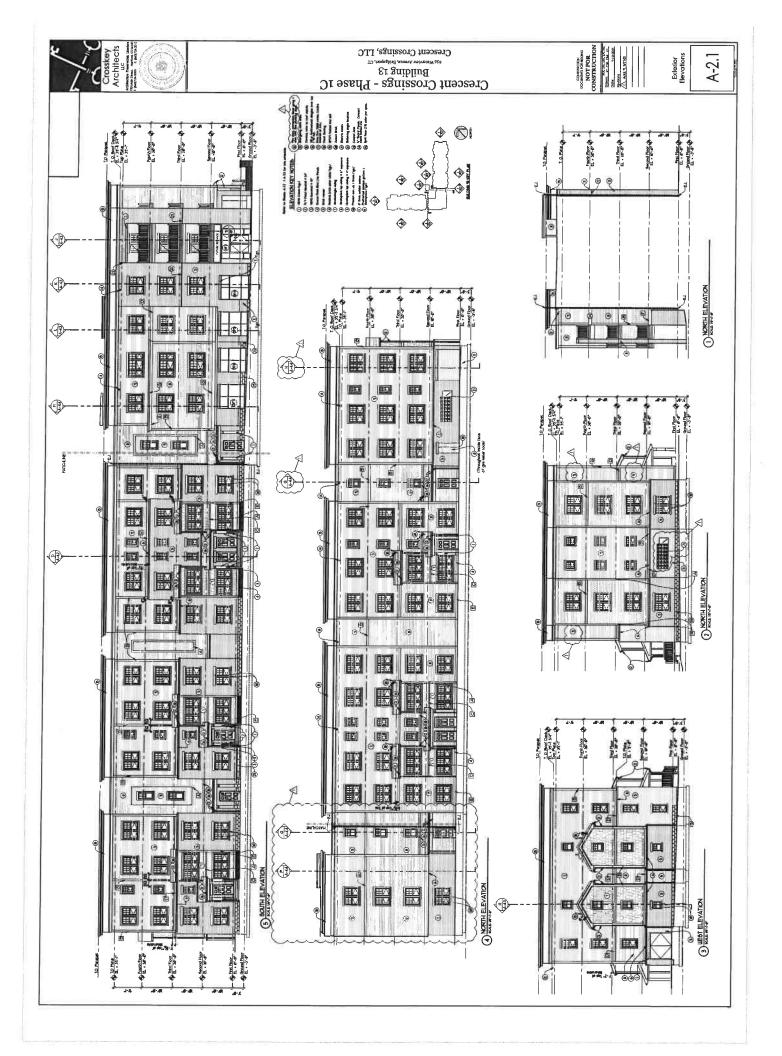


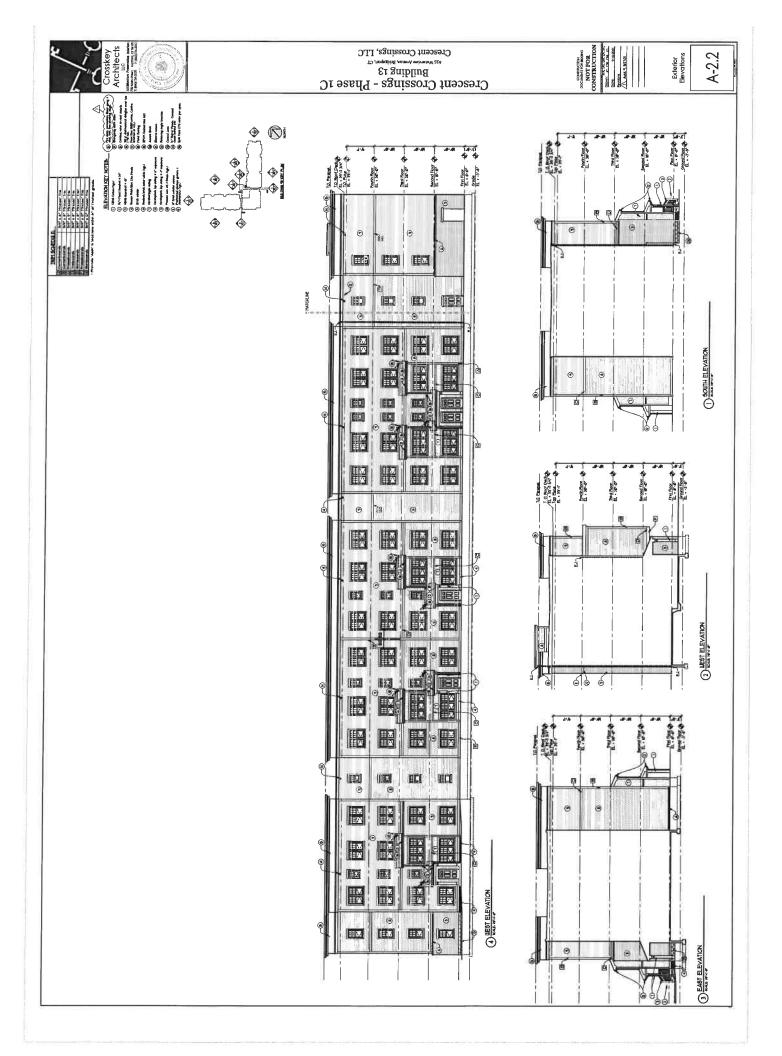


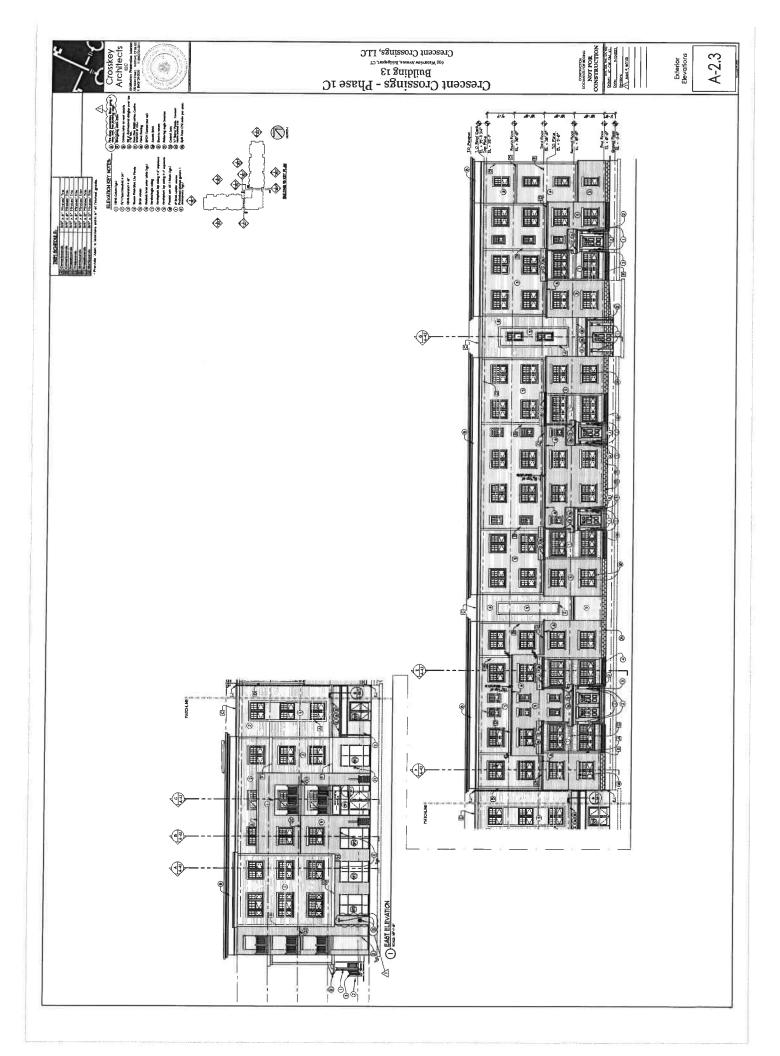












	CITY OF B	RIDGEPORT	File	e No.
	PLANNING & ZON APPLI	NING CON CATION		
NA	AME OF APPLICANT: JEM 500 North, LL	.C		
ls	the Applicant's name Trustee of Record?	Yes	No X	
	yes, a sworn statement disclosing the Beneficia ddress of Property: <u>436-500 North Avenue</u>			upon filing.
	(number)	(street)	(state)	(zip code)
As	ssessor's Map Information: Block No. 2131		Lot No. 3	
Ar	mendments to Zoning Regulations: (indicate) Ar	rticle:	Sec	ction:
	Attach copies of Amendment)			
x	escription of Property (Metes & Bounds): 237. 12.36' See Schedule A, attached		08.2' x 208.98' x	60.58' x 274.97'
	xisting Zone Classification: Mixed Use - Ligh			
	one Classification requested: Industrial Light			
	escribe Proposed Development of Property: <u>CO</u> <i>v</i> ith a drive-through facility	Instruction of	a Wendy's fast fo	ood restaurant
Ar	pproval(s) requested: Change of Zone, Spe	ecial Permit a	nd Site Plan App	roval
			>	
Si	ignature:		Dat	e: 10/27/20
Pr	ignature: rint Name: <u>Anades Turit</u> signed by Agent, state capacity (Lawyer, Develo		TR ature: A.A.	e: 10/27/20 tanes
Pr lf s Ma Ph	rint Name: <u>Chardes Turit</u> signed by Agent, state capacity (Lawyer, Develo lailing Address: <u>1000 Brielgeport A</u> hone: <u>203-366-3939</u> Cell:	Print I De. Suite	TR ature: <u>A.</u> Name: 501; Shellow	tanej 1, CT 06484
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Pr lf s Ma Pr E-	rint Name: <u>Chip des Tuil</u> signed by Agent, state capacity (Lawyer, Develo lailing Address: <u>1000 Brielgeport An</u> hone: <u>203-366-3939</u> Cell: -mail Address: <u>d lord &amp; wwblaw</u> , c Fee received Date:	Print I De. 50; Le COM	TR ature: <u>A.A.</u> Name: SOI; Shellow Fax Fax	PLETED CHECKLIST
Pr lf : Ma Pr E-	rint Name: <u><i>Mades</i></u> <u>Tuid</u> signed by Agent, state capacity (Lawyer, Develo lailing Address: <u>1000 Brielgeport Ac</u> hone: <u>03-366-3739</u> Cell: -mail Address: <u><i>d</i>. 10rd Owblaw</u> , <u>c</u> Fee received Date: <u>THIS APPLICATION MUST BE SUBMIT</u> Completed & Signed Application Form	Print I De. 50; Le COM TTED IN PERSO	TR ature: <u>A</u> Name: SOI, <u>Shellor</u> Fax Clerk: <u>Clerk:</u>	PLETED CHECKLIST
Pr If : Ma Pr E- \$_	rint Name: <u><i>Lagdes</i></u> <u>Tuid</u> signed by Agent, state capacity (Lawyer, Develo lailing Address: <u><i>JOOO Bridgeport Ad</i></u> hone: <u>203 - 366 - 3939</u> Cell: -mail Address: <u><i>Alord Swablaw</i></u> , cell: Fee received Date: <u>THIS APPLICATION MUST BE SUBMIT</u> Completed & Signed Application Form Completed Site / Landscape Plan	Print 1 De. 50; be COM TTED IN PERSO Dra	TR         ature:	PLETED CHECKLIST
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Pr If : Pr E- \$ 0 0	rint Name: <u><u><u><u></u></u><u><u><u></u><u><u></u></u><u><u></u><u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u><u></u></u></u></u></u></u></u>	Print I De. Suite COM TTED IN PERSO A	TR         ature:       Ame:         Sol:       Shellow         Sol:       Shellow         Fax         Clerk:       Fax         DN AND WITH COM         Site Survey         inage Plan         berty Owner's List         orations & LLC's)	PLETED CHECKLIST Building Floor Plan Building Elevations Fee

Rev. 6/18/2016

#### SCHEDULE A

#### Property Description 436-500 North Avenue Bridgeport, CT

Being a certain parcel of land situated in the City of Bridgeport and State of Connecticut, as depicted on a map entitled "Property and Topographic Survey, of property located at 436-500 North Avenue, Bridgeport, Connecticut, prepared for JEM 500 North LLC", scale 1"=20'. Dated Aug. 20, 2021, by Rose-Tiso & Co., LLC, being more particularly bounded and described as follows:

Commencing at a point, said point being the intersection of the northwesterly street line of North Avenue with the northerly street line of Lindley Street, said point also being the southeasterly corner of land now or formerly of 512 North Avenue, LLC, said point also being the southernmost point of the parcel herein described,

Thence in a northwesterly direction, bounded westerly by land now or formerly of 512 North Avenue, LLC the following 3 courses:

N 00° 09' 08" W,	237.15 feet,
N 56° 22' 25" W,	6.35 feet, and
N 00° 09' 08" W,	a distance of 108.20 feet to a point,

Thence S 72° 56' 31" E, bounded northwesterly by land now or formerly of Estate of F. Francis D'addario, a distance of 208.98 feet to a point;

Thence in a southwesterly direction along the westerly street line of North Avenue the following four courses:

Along a curve to the right having a radius of 391.72 feet, an interior angle of 80° 51' 39" and an arc length of 60.58 feet, S 33° 54" 27" W, 274.97 feet and S 57° 19' 45" W, a distance of 12.36 feet to the point of a commencement.

Said described parcel of land contains 35,859 sq. ft. or 0.8233 Acres.

#### CITY OF BRIDGEPORT PLANNING & ZONING COMMISSION

#### STATEMENT IN SUPPORT OF:

#### APPLICATION FOR CHANGE OF ZONE, SPECIAL PERMIT AND SITE PLAN APPROVAL

#### 436 -500 NORTH AVENUE

#### JEM 500 NORTH, LLC.

The applicant, JEM 500 North, LLC, is the owner of property known as 436-500 North Avenue. The property is located in the Mixed Use-Light Industrial ("MU-LI") zoning district. The applicant seeks to develop the property with a Wendy's fast-food restaurant with a drive-through facility. The MU-LI zoning district does not permit drive-through facilities, thus the applicant seeks to change the zone to Industrial Light ("IL").

The vast majority of the immediate area is zoned IL and contains a variety of commercial uses as well as three other national fast-food restaurants with drive-through facilities, namely Taco Bell, Popeyes and McDonald's. All of those sites are in the IL zoning district. Unfortunately, the 436-500 North Avenue parcel was placed in the MU-LI zoning district.

The change of zone will not adversely affect the comprehensive plan of development and will place the applicant on a par with the neighboring property owners. As can be noted, the plans for the Wendy's fast food restaurant show that the development site can easily and safely accommodate the proposed restaurant and drive-through plan without creating any negative impacts to the area.

#### JEM 500 NORTH, LLC 436-500 NORTH AVENUE, BRIDGEPORT, CT ABUTTING PROPERTY OWNERS &OWNERS WITHIN 100 FEET OF SUBJECT PROPERTY

#### **ABUTTING PROPERTY OWNERS**

<b>Property Description</b>	Owner(s)	Mailing Address
360 Lindley St.	512 North Avenue, LLC	120 River St. Bridgeport, CT 06604
410 North Ave.	Estate of Francis Daddario	

#### Non-Abutting Property Owners within 100'

493 North Ave.	Shiangling Lin Wong	183 S. Bonnie Ave., #5
		Pasadena, CA 91106
133 Evergreen St.	425 North Avenue, LLC	3421 Main St., Unit D
		Stratford, CT 06614
485 North Ave.	425 North Avenue, LLC	3421 Main St., Unit D
		Stratford, CT 06614
380 Lindley St.	Charlie Lindley, LLC	323 North Ave.
		Bridgeport, CT 06604
415 North Ave.	HOCAP Corp.	469 Brooklawn Ave.
		Fairfield, CT 06825

	State of PHONE: 860-509-6003 •		ts.ct.gov	<ul> <li>OFFICE USE ONLY</li> <li>Use ink. • Print or type.</li> <li>Attach additional 8½ x 11 sheets if nec</li> </ul>		
	FICATE OF					
FILING PA	RTY (Confirmation	will be sent to this	address):			
NAME:	FILE IT USA I	INC			A Contractor and the second	
MAILING	408 SOUTH 5	STH ST			FILING FEE: \$120	
ADDRESS:					Make checks payable to "Secretary of the State"	
CITY:	BROOKLYN					
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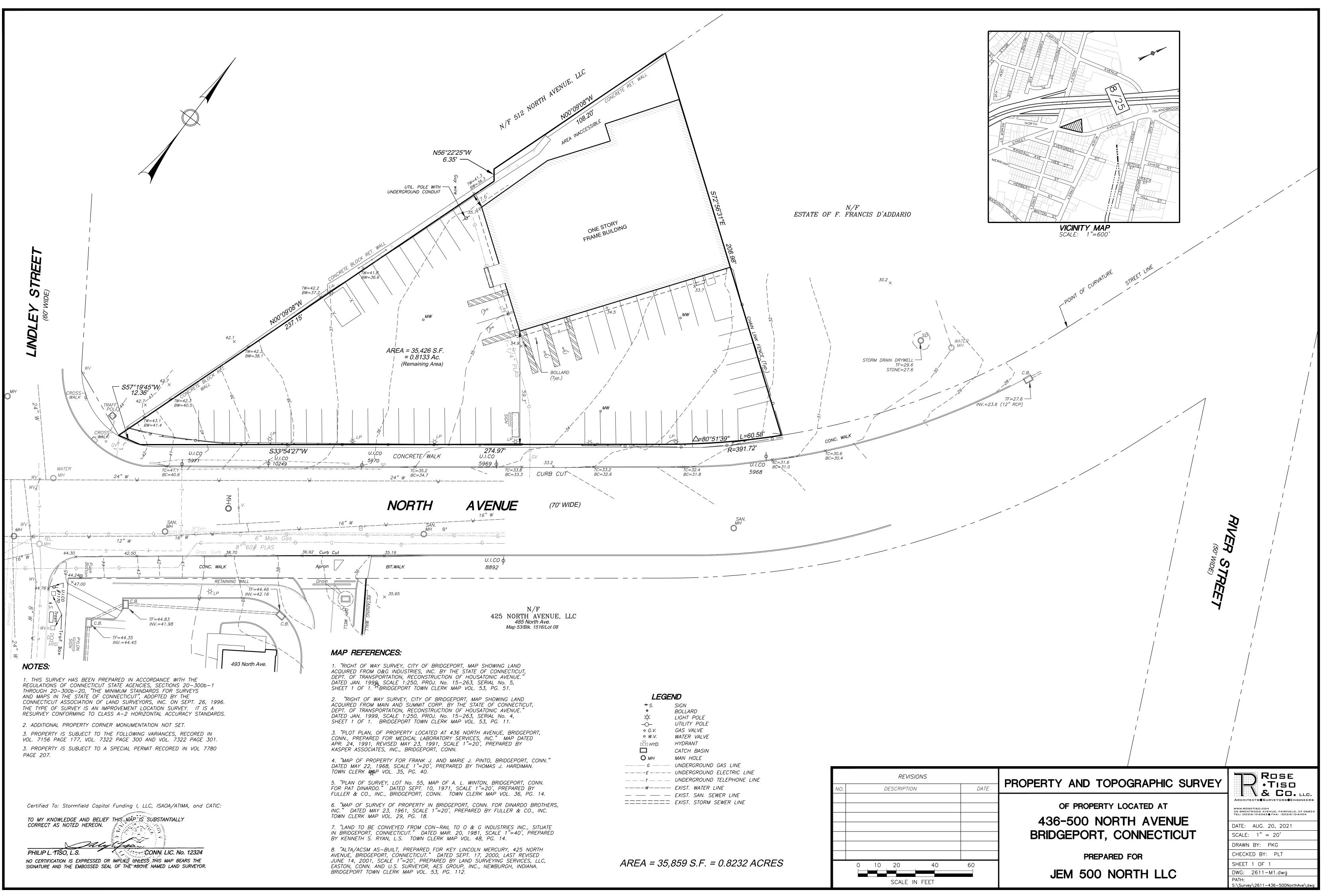
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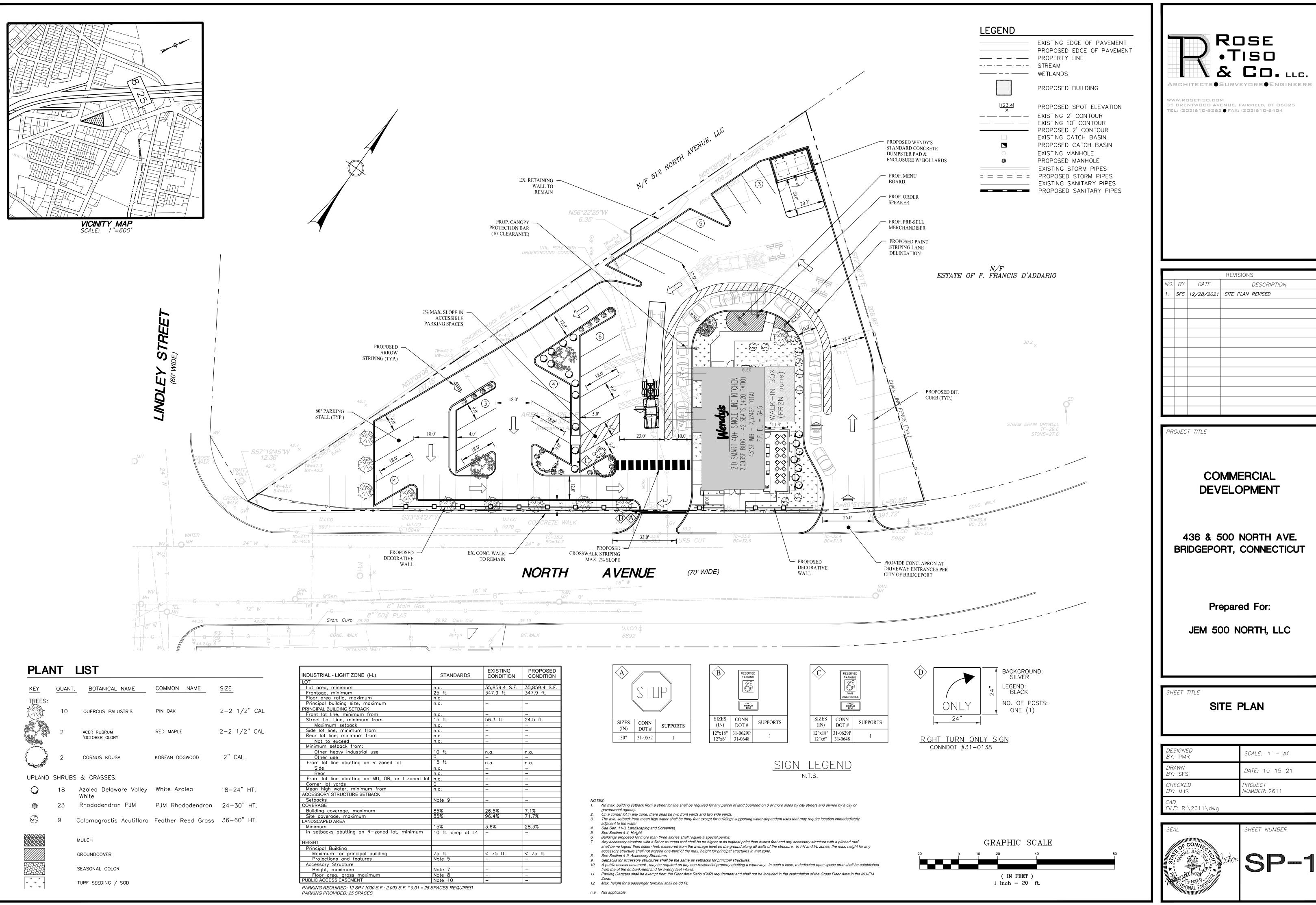
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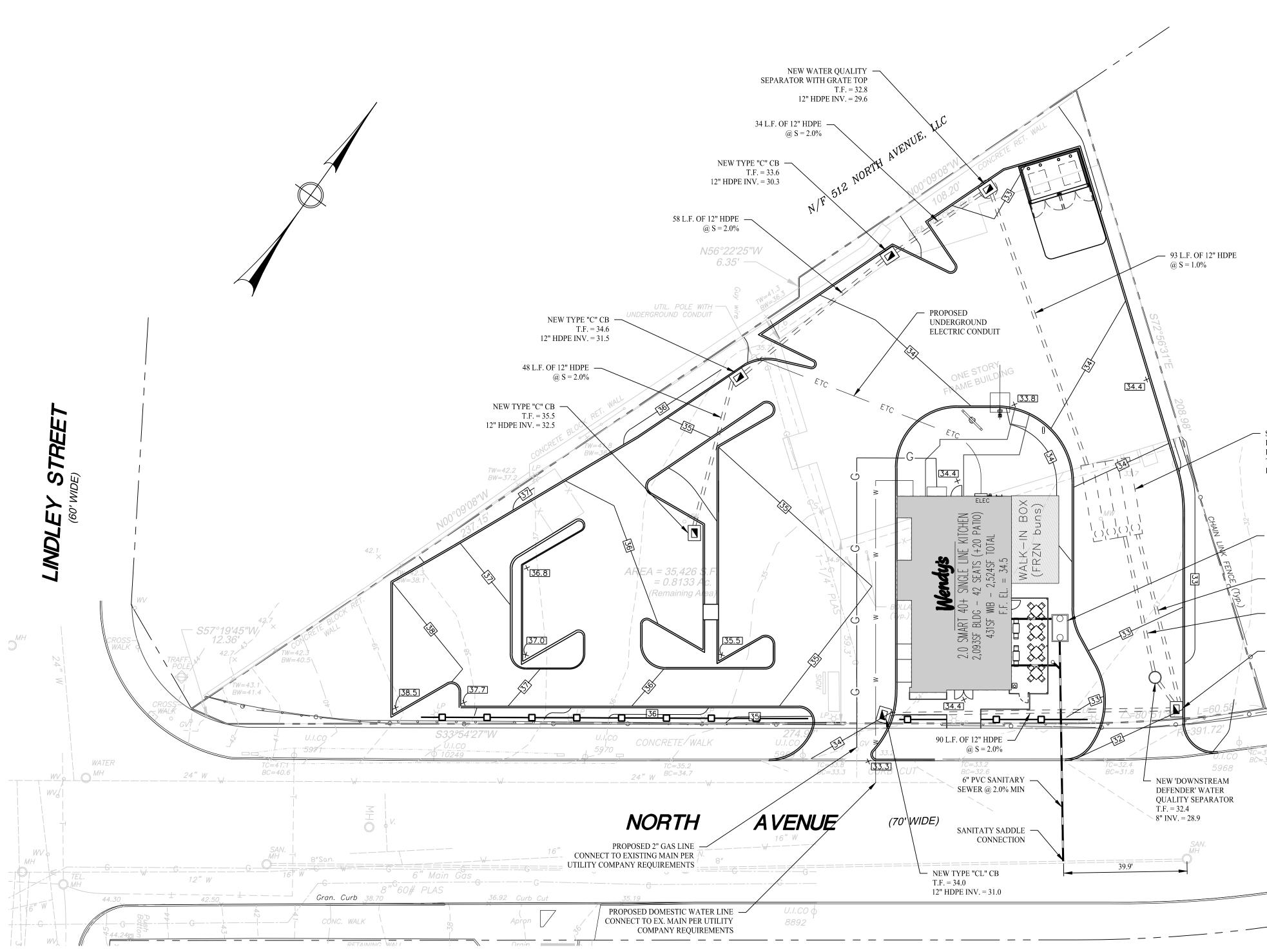
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E If Agent is a business		4			
print or type name of bu	isiness as it appears	s on our records:	Corporate Creation	s Network Inc.	
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	Manager	CITY: Brooklyr STATE: NY Z		CITY: Brooklyn STATE: NY ZIP:	11234 -
Moses Singer	Member		ck if none: 9 Rutledge St. 2A	ADDRESS: 42 Wa	alton St. 4B
	Manager	CITY: Brooklyr STATE: NY Z		CITY: Brooklyn STATE: NY ZIP:	11206 -
6. ENTITY E-MAIL ADDRE (Check box if none. Do not		None	7. NAICS CODE (six o	ligits):	
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8. EXECUTION / SIGNATU	RE (required) (Subje	ect to penalties of fa	l alse statement):		
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# **GRADING & DRAINAGE NOTES**

- 1. ALL ROOF DRAINS AND YARD DRAIN DISCHARGE PIPING SHALL BE MIN 4" PVC PLASTIC PIPE (ASTM D 3034) SDR-35 WITH RUBBER GASKETS, BELL AND SPIGOT TYPE JOINTS.
- 2. ALL PERFORATED DISTRIBUTION PIPES WITHIN GALLERIES SHALL BE 4" PERFORATED PVC PLASTIC PIPE (ASTM D 2729) WITH BELL AND SPIGOT, NO GASKET.
- 3. ALL SITE CONSTRUCTION SHALL CONFORM TO THE CITY OF BRIDGEPORT STANDARD SPECIFICATIONS OR IN THE ABSENCE THEREOF TO THE STATE OF CONNECTICUT
- DEPARTMENT OF TRANSPORTATION FORM 816, 2004.4. ALL PROPOSED CATCH BASISN TO HAVE 2' SUMPS, HOODED OUTLETS, AND FLO-GARD CATCH BASIN INSERTS, UNLESS OTHERWISE NOTED.
- 5. MAXIMUM 2% SLOPE THROUGHOUT ALL ACCESSIBLE PARKING AND ACCESSIBLE STRIPED AREAS

## HYDRODYNAMIC SEPARATOR AND CATCH BASIN INSPECTION & MAINTENANCE PLAN:

 UNITS ARE TO BE INSPECTED EVERY 6-MONTHS AND SUMP VACUUMED IF SEDIMENT DEPTH IS GREATER THAN 18 INCHES
 OIL ACCUMULATION IS TYPICALLY MUCH LESS THAN SEDIMENT, HOWEVER, REMOVAL OF OIL AND SEDIMENT DURING THE SAME SERVICE IS RECOMMENDED.

REMOVE FLOATABLES FIRST, AND THEN REMOVE REMAINING VOLUME.
 GALLERY INSPECTION & MAINTENANCE PLAN:
 GALLERIES ARE TO BE INSPECTED EVERY 12-MONTHS AND VACUUMED IF SEDIMENT

DEPTH IS GREATER THAN 12 INCHES. 2. SITE CATCH BASINS ARE TO BE INSPECTED EVERY 6- MONTHS AND SUMP VACUUMED IF SEDIMENT DEPTH IS GREATER THAN 12 INCHES.

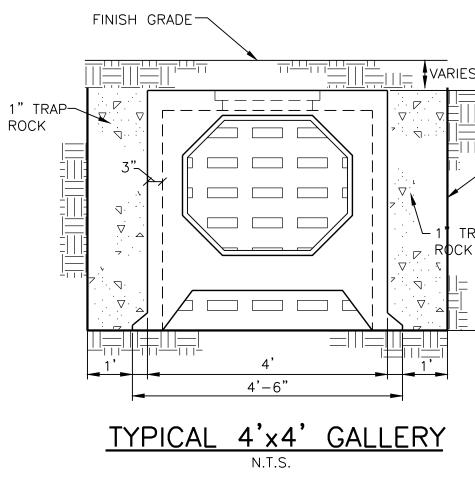
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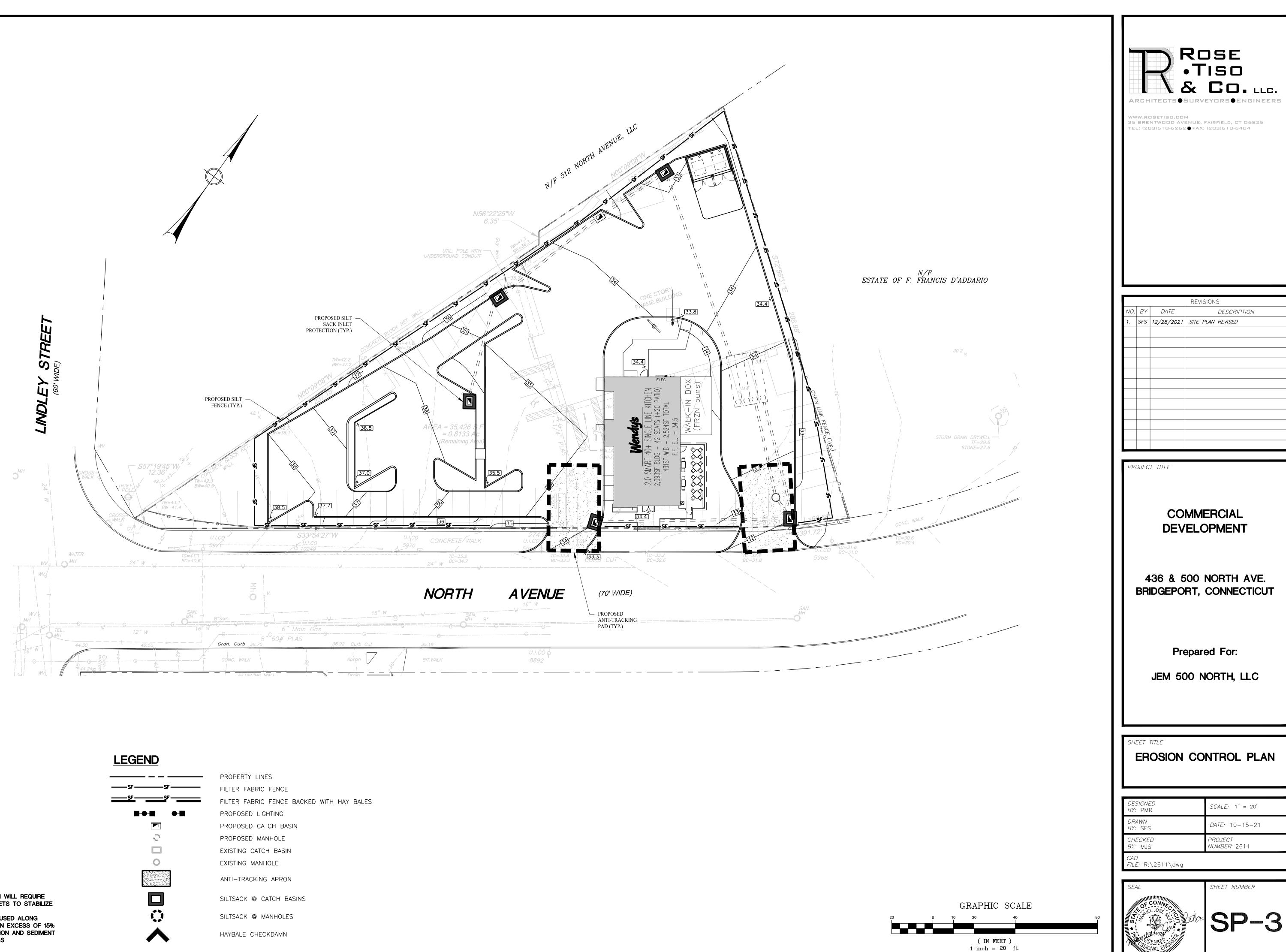
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 PROPOSED EDGE OF PAVEMENT
 PROPERTY LINE
 STREAM
 WETLANDS

PROPOSED BUILDING

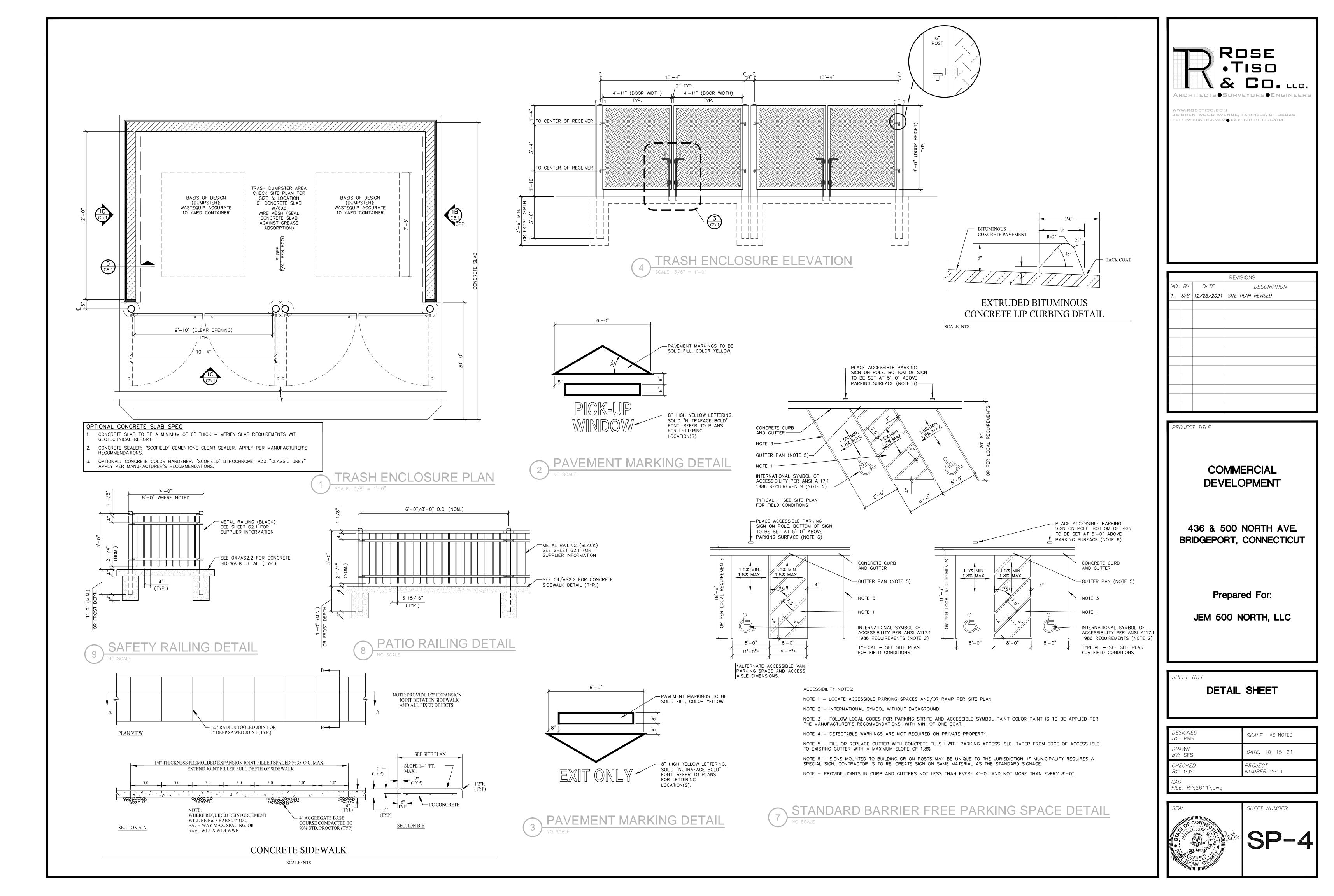
PROPOSED SPOT ELEVATION
 EXISTING 2' CONTOUR
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 PROPOSED 2' CONTOUR
 EXISTING CATCH BASIN
 PROPOSED CATCH BASIN
 EXISTING MANHOLE
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 EXISTING STORM PIPES
 PROPOSED STORM PIPES
 EXISTING SANITARY PIPES
 PROPOSED SANITARY PIPES



N/F ESTATE OF F. FRANCIS D'ADDARIO		
	REVI	SIONS
	NO.         BY         DATE           1.         SFS         12/28/2021         SITE	DESCRIPTION PLAN REVISED
SUBSURFACE STORMWATER STORAGE AREA 100 L.F. OF 4'X4' CONCRETE CHAMBERS		
H-20 LOADING UNITS REQUIRED TOTAL VOLUME =2,200 Cu.Ft. INVERT OF SYSTEM = 28.50 $^2 \times$		
1,000 GAL. CONC.		
GREASE TRAP T.F. = 34.0		
$ \begin{array}{c} 56 \text{ L.F. OF 12" HDPE} \\ \hline @ \text{ S} = 2.0\% \end{array} $ STORM DRAIN DRYWELL $-1 \\ TF=29.6 \end{array} $		
STONE=27.6 45 L.F. OF 12" HDPE @ S = 2.0%		
NEW TYPE "C" CB T.F. = 32.0	PROJECT TITLE	
1.F 32.0 12" HDPE INV. IN (W) = 29.2 8" HDPE INV. O (W) = 29.0 12" HDPE INV. OVERFLOW OUT (N) = 29.67		
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$\Phi$ $TC=30.6$ $BC=30.4$		OPMENT
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- SOILS EROSION NOTES: 1. GRADES STEEPER THEN 2:1 WILL REQUIRE EROSION CONTROL BLANKETS TO STABILIZE SOILS
- WATER BARS ARE TO BE USED ALONG ROADWAYS AND GRADES IN EXCESS OF 15%
   SEE SP-5 FOR FULL EROSION AND SEDIMENT CONTROL NOTES & DETAILS



## SOIL EROSION AND SEDMIMENT CONTROL GENERAL NOTES

<u>SEDIMENT & EROSION CONTROL NARRATIVE</u> THE SEDIMENT AND EROSION CONTROL PLAN WAS DEVELOPED TO PROTECT THE EXISTING ROADWAY AND STORM DRAINAGE SYSTEMS, ADJACENT PROPERTIES, AND ANY ADJACENT WETLAND AREA AND WATERCOURSE FROM SEDIMENT LADEN SURFACE RUNOFF AND EROSION.

CONSTRUCTION SCHEDUL THE ANTICIPATED STARTING DATE FOR CONSTRUCTION IS SPRING 2020 WITH COMPLETION ANTICIPATED BY SPRING 2021. APPROPRIATE EROSION CONTROL MEASURES AS DESCRIBED HEREIN, SHALL BE INSTALLED BY THE CONTRACTOR PRIOR TO THE COMMENCEMENT OF ALL SITE CLEARING OR CONSTRUCTION ACTIVITY. SCHEDULE WORK TO MINIMIZE THE LENGTH OF TIME THAT BARE SOIL WILL BE EXPOSED.

ONTINGENCY EROSION PLA HE CONTRACTOR SHALL INSTALL ALL SPECIFIED EROSION CONTROL MEASURES AND WILL BE REQUIRED TO MAINTAIN THEM IN THEIR INTENDED FUNCTIONING CONDITION. THE LAND USE AGENTS OF THE TOWN OF BRIDGEPORT AND PROJECT ENGINEER SHALL HAVE THE AUTHORITY TO REOUIRE SUPPLEMENTAL MAINTENANCE OR ADDITIONAL MEASURES IF FIELD CONDITIONS ARE ENCOUNTERED BEYOND WHAT WOULD NORMALLY BE ANTICIPATED.

#### OPERATION REQUIREMENT LEARING AND GRUBBING OPERATIONS:

- 1. ALL SEDIMENTATION AND EROSION CONTROL MEASURES, INCLUDING THE CONSTRUCTION OF TEMPORARY SEDIMENTATION TRAPS AND STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS. WILL BE INSTALLED PRIOR TO THE START OF CLEARING AND GRUBBING OPERATIONS. FOLLOWING INSTALLATION OF ALL SEDIMENTATION AND EROSION CONTROL MEASURES, THE CONTRACTOR
- SHALL NOT PROCEED WITH GRADING, FILLING OR OTHER CONSTRUCTION OPERATIONS UNTIL THE ENGINEER HAS INSPECTED AND APPROVED ALL INSTALLATIONS
- THE CONTRACTOR SHALL TAKE EXTREME CARE DURING CLEARING AND GRUBBING OPERATIONS SO AS NOT TO DISTURB UNPROTECTED WETLAND AREAS OR SEDIMENTATION AND EROSION CONTROL DEVICES. FOLLOWING THE COMPLETION OF CLEARING AND GRUBBING OPERATIONS, ALL AREAS SHALL BE STABILIZED
- WITH TOPSOIL AND SEEDING OR PROCESSED AGGREGATE STONE AS SOON AS PRACTICAL. ALL REMOVED INVASIVE PLANT SPECIES MATERIAL SHALL BE FULLY REMOVED FROM THE SITE AND TAKEN TO AN APPROVED AND/OR ACCEPTABLE DISPOSAL LOCATION
- ROUGH GRADING OPERATIONS: 1. DURING THE REMOVAL AND/OR PLACEMENT OF EARTH AS INDICATED ON THE GRADING PLAN, TOPSOIL SHALL BE STRIPPED AND APPROPRIATELY STOCKPILED FOR REUSE. 2. ALL STOCKPILED TOPSOIL SHALL BE SEEDED, MULCHED WITH HAY, AND ENCLOSED BY A SILTATION FENCE.

#### FILLING OPERATIONS

- 1. PRIOR TO FILLING, ALL SEDIMENTATION AND EROSION CONTROL DEVICES SHALL BE PROPERLY IMPLEMENTED, MAINTAINED AND FULLY INSTALLED, AS DIRECTED BY THE ENGINEER AND AS SHOWN ON THIS PLAN. ALL FILL MATERIAL ADJACENT TO ANY WETLAND AREAS, IF APPLICABLE TO THIS PROJECT, SHALL BE GOOD OUALITY, WITH LESS THAN 5% FINES PASSING THROUGH A #200 SIEVE (BANK RUN), SHALL BE PLACED IN LIFT THICKNESS NOT GREATER THAN THAT SPECIFIED IN PROJECT SPECIFICATIONS. LIFTS SHALL BE COMPACTED TO 95% MAX. DRY DENSITY MODIFIED PROCTOR. AS GENERAL GRADING OPERATIONS PROGRESS, ANY TEMPORARY DIVERSION DITCHES SHALL BE RAISED OR
- LOWERED, AS NECESSARY, TO DIVERT SURFACE RUNOFF TO THE SEDIMENT TRAPS.
- PLACEMENT OF DRAINAGE STRUCTURES, UTILITIES, AND ROADWAY CONSTRUCTION OPERATIONS: SILT FENCES SHALL BE INSTALLED AT THE DOWNHILL SIDES OF TEMPORARY SEDIMENT TRAP SLOPES, MUD PUMP DISCHARGES. AND UTILITY TRENCH MATERIAL STOCKPILES. HAY BALES MAY BE USED IF SHOWN ON THE EROSION CONTROL PLANS OR IF DIRECTED BY THE PROJECT ENGINEER.

#### FINAL GRADING AND PAVING OPERATIONS: ALL INLET AND OUTLET PROTECTION SHALL BE PLACED AND MAINTAINED AS SHOWN ON EROSION CONTROL

- PLANS AND DETAILS. AND AS DESCRIBED IN SPECIFICATIONS AND AS DESCRIBED HEREIN NO CUT OR FILL SLOPES SHALL EXCEED 2:1 EXCEPT WHERE STABILIZED BY ROCK FACED EMBANKMENTS OR EROSION CONTROL BLANKETS, JUTE MESH AND VEGETATION. ALL SLOPES SHALL BE SEEDED, AND ANY ROAD OR DRIVEWAY SHOULDER AND BANKS SHALL BE STABILIZED IMMEDIATELY UPON COMPLETION OF FINAL GRADING UNTIL TURF IS ESTABLISHED.
- PAVEMENT SUB-BASE AND BASE COURSES SHALL BE INSTALLED OVER AREAS TO BE PAVED AS SOON AS FINAL SUB-GRADES ARE ESTABLISHED AND UNDERGROUND UTILITIES AND STORM DRAINAGE SYSTEMS HAVE BEEN INSTALLED.
- AFTER CONSTRUCTION OF PAVEMENT, TOPSOIL, FINAL SEED, MULCH AND LANDSCAPING, REMOVE ALL TEMPORARY EROSION CONTROL DEVICES ONLY AFTER ALL AREAS HAVE BEEN PAVED AND/OR GRASS HAS BEEN WELL ESTABLISHED AND THE SITE HAS BEEN INSPECTED AND APPROVED BY THE TOWN OF BRIDGEPORT, EASTERN CONNECTICUT SOILS CONSERVATION DISTRICT, TOWN OF BRIDGEPORT INLAND WETLANDS COMMISSION.

#### INSTALLATION OF SEDIMENTATION AND EROSION CONTROL MEASURES SILTATION FENCE

- A. DIG A SIX INCH TRENCH ON THE UPHILL SIDE OF THE DESIGNATED FENCE LINE LOCATION. B. POSITION THE POST AT THE BACK OF THE TRENCH (DOWNHILL SIDE), AND HAMMER THE POST AT LEAST 1.5
- FEET INTO THE GROUND. C. LAY THE BOTTOM SIX INCHES OF THE FABRIC INTO THE TRENCH TO PREVENT UNDERMINING BY STORM
- WATER RUN-OFF D. BACKFILL THE TRENCH AND COMPACT.

### IL HAY BALES:

- A. BALES SHALL BE PLACED IN A SINGLE ROW, LENGTHWISE, ORIENTED PARALLEL TO THE CONTOUR, WITH ENDS OF ADJACENT BALES TIGHTLY ABUTTING ONE ANOTHER. B. BALES SHALL BE ENTRENCHED AND BACKFILLED. A TRENCH SHALL BE EXCAVATED THE WIDTH OF A BALE AND THE LENGTH OF THE PROPOSED BARRIER TO A MINIMUM DEPTH OF FOUR INCHES. AFTER THE BALES ARE STAKED, THE EXCAVATED SOIL SHALL BE BACKFILLED AGAINST THE BARRIER.
- C. EACH BALE SHALL BE SECURELY ANCHORED BY AT LEAST TWO (2) STAKES.
- D. THE GAPS BETWEEN BALES SHALL BE WEDGED WITH STRAW TO PREVENT WATER LEAKAGE E. THE BARRIER SHALL BE EXTENDED TO SUCH A LENGTH THAT THE BOTTOMS OF THE END BALES ARE HIGHER IN ELEVATION THAN THE TOP OF THE LOWEST MIDDLE BALE, TO ENSURE THAT RUN-OFF WILL FLOW EITHER THROUGH OR OVER THE BARRIER, BUT NOT AROUND IT.

#### OPERATION AND MAINTENANCE OF SEDIMENTATION AND EROSION CONTROL MEASURES SILTATION FENC

- A. ALL SILTATION FENCES SHALL BE INSPECTED AS A MINIMUM WEEKLY OR AFTER EACH RAINFALL. ALL DETERIORATED FABRIC AND DAMAGED POSTS SHALL BE REPLACED AND PROPERLY REPOSITIONED IN ACCORDANCE WITH THIS PLAN. B. SEDIMENT DEPOSITS SHALL BE REMOVED FROM BEHIND THE FENCE WHEN THEY EXCEED A HEIGHT OF ONE
- FOOT.

#### II. HAY BALES:

A. ALL HAY BALE RINGS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OR REPLACEMENT SHALL BE PROMPTLY MADE AS NEEDED B. DEPOSITS SHALL BE REMOVED AND CLEANED-OUT IF ONE HALF OF THE ORIGINAL HEIGHT OF THE BALES BECOMES FILLED WITH SEDIMENT.

#### III. SEDIMENT TRAPS:

- A. CONTRACTOR TO KEEP WEEKLY CHECKLIST LOGS FOR INSPECTIONS OF ALL SEDIMENT AND EROSION CONTROL DEVICES AND HAVE THEM READILY AVAILABLE ON-SITE AT ALL TIMES FOR INSPECTION BY CT DEEP. LOCAL AUTHORITIES OR ENGINEER B. ALL SEDIMENT TRAPS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OF SLOPES SHALL BE
- PROMPTLY MADE AS NEEDED. EROSION CONTROL BLANKETS MAY BE USED FOLLOWING REPAIR OF SLOPE AS DIRECTED BY THE ENGINEER. C. SEDIMENT DEPOSITS SHALL BE REMOVED FROM SEDIMENT TRAPS AND/OR SEDIMENT TRAPS WHEN THEY
- EXCEED A HEIGHT OF ONE FOOT UNLESS OTHERWISE INDICATED ON THE EROSION CONTROL PLANS AND DETAILS TO BE AT A SPECIFIC ELEVATION PER CLEAN OUT MARKERS. D. SEDIMENT SHALL BE DISPOSED OF ON-SITE OR AS DIRECTED BY THE ENGINEER AND LOCAL GOVERNING
- OFFICIALS. SEE SEDIMENT AND EROSION CONTROL NOTES HEREIN REGARDING DISPOSAL REQUIREMENTS FOR OFF SITE SPOIL DISPOSAL. IV. CHECK DAMS:

#### A. ALL STONE CHECK DAMS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OF STONE CHECK DAMS SHALL BE PROMPTLY MADE AND ACCUMULATED SEDIMENT REMOVED WHEN IT REACHES ONE HALF OF THE HEIGHT OF THE CHECK DAM.

V. TEMPORARY/PERMANENT DRAINAGE SWALES: A. SWALES SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OF ANY WASHED OUT OR ERODED SLOPES SHALL BE MADE PROMPTLY AND THE AREA SHALL BE RESEEDED AS NECESSARY. B. EROSION CONTROL BLANKETS MAY BE USED TO REPAIR ERODED SWALES AS DIRECTED BY THE ENGINEER OR TOWN OF BRIDGEPORT AGENT.

## EROSION AND SEDIMENT CONTROL PLAN

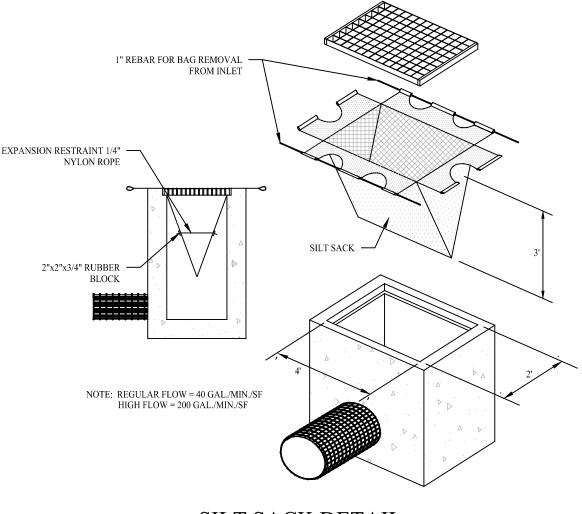
THE PLAN.

- . HAY BALE FILTERS OR SILTATION FENCE WILL BE INSTALLED AT ALL CULVERT OUTLETS IF CULVERT OUTLETS ARE APPLICABLE TO THIS PROJECT AND ALONG THE TOE OF ALL CRITICAL CUT AND FILL SLOPES. 2. CULVERT DISCHARGE AREAS WILL BE PROTECTED WITH RIP RAP CHANNELS; ENERGY DISSIPATERS WILL BE INSTALLED AS SHOWN ON THESE PLANS AND AS NECESSARY
- 3. CATCH BASINS WILL BE PROTECTED WITH HAY BALE FILTERS, SILT SACKS, SILTATION FENCE. OR OTHER INLET PROTECTION DEVICES PER DETAILS, THROUGHOUT THE CONSTRUCTION PERIOD AND UNTIL ALL DISTURBED
- AREAS ARE THOROUGHLY STABILIZED 4. ALL EROSION AND SEDIMENT CONTROL MEASURES WILL BE INSTALLED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL MANUAL.
- LATEST EDITION 5. EROSION AND SEDIMENT CONTROL MEASURES WILL BE INSTALLED PRIOR TO CONSTRUCTION WHENEVER
- POSSIBLE 6. ALL CONTROL MEASURES WILL BE MAINTAINED IN EFFECTIVE CONDITION THROUGHOUT THE CONSTRUCTION
- PERIOD 7. ADDITIONAL CONTROL MEASURES WILL BE INSTALLED DURING THE CONSTRUCTION PERIOD, IF NECESSARY OR REOUIRED OR AS DIRECTED BY THE CIVIL ENGINEER OR BY LOCAL GOVERNING OFFICIALS.
- 8. SEDIMENT REMOVED FROM EROSION CONTROL STRUCTURES WILL BE DISPOSED IN A MANNER WHICH IS CONSISTENT WITH THE INTENT AND REQUIREMENTS OF THE EROSION CONTROL PLANS, NOTES, AND DETAILS. 9. THE OWNER IS ASSIGNED THE RESPONSIBILITY FOR IMPLEMENTING THIS EROSION AND SEDIMENT CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE INSTALLATION AND MAINTENANCE OF CONTROL MEASURES. INFORMING ALL PARTIES ENGAGED ON THE CONSTRUCTION SITE OF THE REQUIREMENTS AND OBJECTIVES OF

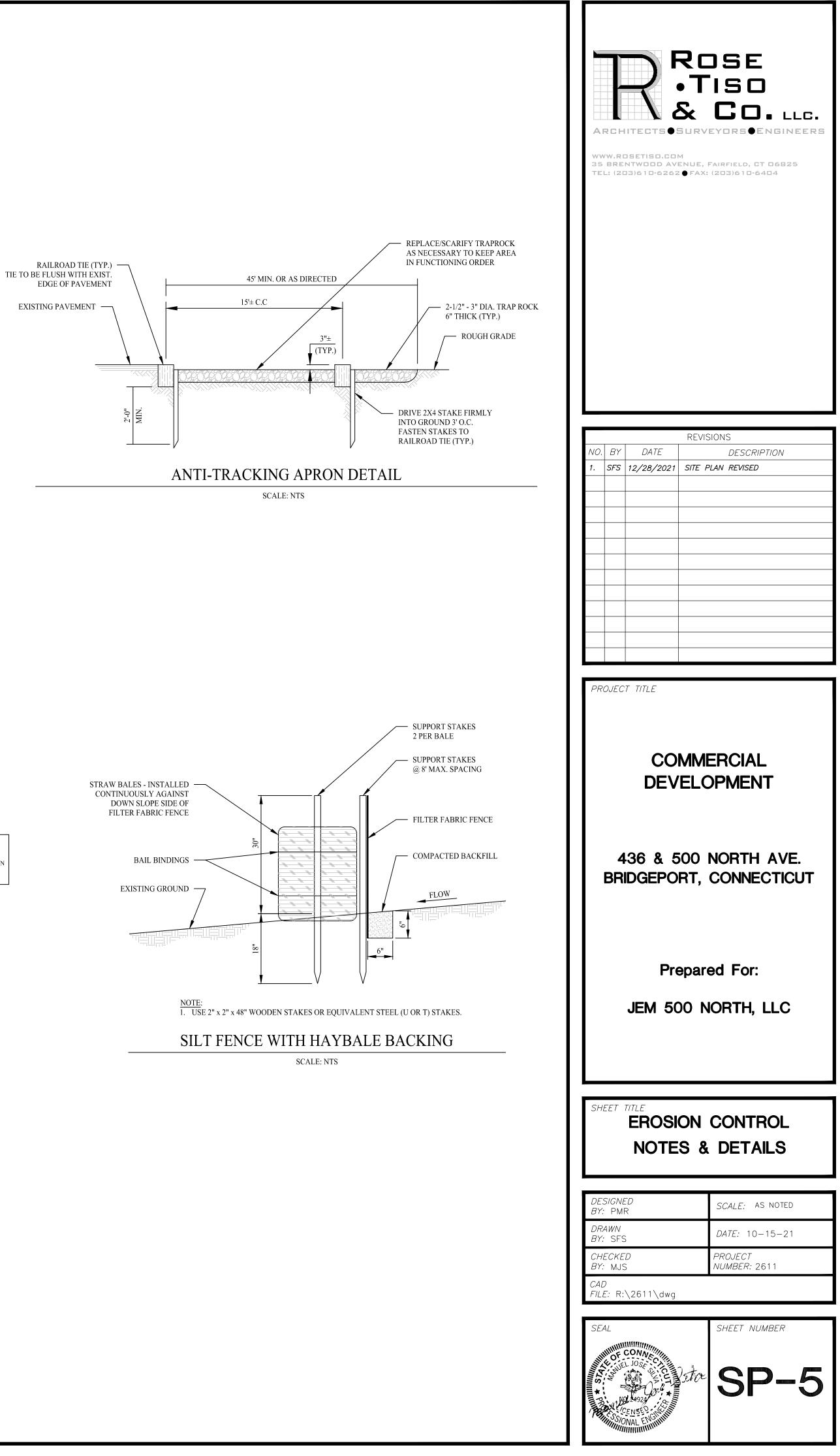
# <u>SEDIMENT AND EROSION CONTROL NOTES</u> I. THE OWNER IS RESPONSIBLE FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS

- INFORMING ALL PARTIES ENGAGED WITH CONSTRUCTION ON THE SITE OF THE REQUIREMENTS AND OBJECTIVES OF THIS PLAN, INFORMING THE GOVERNING AUTHORITY OR INLAND WETLANDS AGENCY OF ANY TRANSFER OF THIS RESPONSIBILITY, AND FOR CONVEYING A COPY OF THE SEDIMENT & EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED
- 2. AN EROSION CONTROL BOND MAY BE REQUIRED TO BE POSTED WITH THE TOWN OF BRIDGEPORT TO ENSURE IMPLEMENTATION OF THE EROSION CONTROL MEASURES. THE OWNER SHALL BE RESPONSIBLE FOR THE POSTING OF THIS BOND AND FOR INQUIRIES TO THE TOWN OF BRIDGEPORT FOR INFORMATION ON THE METHOD. TYPE AND AMOUNT OF THE BOND POSTING UNLESS OTHERWISE DIRECTED.
- 3 VISUAL SITE INSPECTIONS SHALL BE CONDUCTED WEEKLY AND AFTER EACH MEASURABLE PRECIPITATION EVENT OF 0.10 INCHES OR GREATER BY QUALIFIED PERSONNEL, TRAINED AND EXPERIENCED IN EROSION AND SEDIMENT CONTROL, TO ASCERTAIN THAT THE EROSION AND SEDIMENT CONTROL (E&S) BMPS ARE OPERATIONAL AND EFFECTIVE IN PREVENTING POLLUTION. A WRITTEN REPORT OF EACH INSPECTION SHALL BE KEPT. AND INCLUDE: A) A SUMMARY OF THE SITE CONDITIONS, E&S BMPS, AND COMPLIANCE; AND
- B) THE DATE, TIME, AND THE NAME OF THE PERSON CONDUCTING THE INSPECTION THE CONTRACTOR SHALL CONSTRUCT ALL SEDIMENT AND EROSION CONTROLS IN ACCORDANCE WITH THE CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL, PREPARED BY CTDEEP, LATEST EDITION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS, AND AS DIRECTED BY THE TOWN OF BRIDGEPORT. THE CONTRACTOR SHALL KEEP A COPY OF THE GUIDELINES ON-SITE FOR REFERENCE DURING CONSTRUCTION
- ADDITIONAL AND/OR ALTERNATIVE SEDIMENT AND EROSION CONTROL MEASURES MAY BE INSTALLED DURING THE CONSTRUCTION PERIOD IF FOUND NECESSARY BY THE CONTRACTOR, OWNER, CIVIL ENGINEER, TOWN OF BRIDGEPORT EASTERN CONNECTICUT SOILS CONSERVATION DISTRICT. TOWN OF BRIDGEPORT INLAND WETLANDS COMMISSION. OR GOVERNING AGENCIES. THE CONTRACTOR SHALL CONTACT THE OWNER AND APPROPRIATE GOVERNING AGENCIES FOR APPROVAL IF ALTERNATIVE CONTROLS OTHER THAN THOSE SHOWN ON THE PLANS ARE PROPOSED.
- THE CONTRACTOR SHALL INSPECT ALL SEDIMENT AND EROSION CONTROLS BEFORE AND AFTER EACH STORM (0.10 INCHES OR GREATER RAINFALL), OR AT LEAST WEEKLY, TO VERIFY THAT THE CONTROLS ARE OPERATING PROPERLY AND MAKE REPAIRS WHERE NECESSARY.
- THE CONTRACTOR SHALL KEEP A SUPPLY OF EROSION CONTROL MATERIAL (HAY BALES, SILT FENCE, JUTE MESH.RIP RAP ETC.) ON-SITE FOR MAINTENANCE AND EMERGENCY REPAIRS. INSTALL PERIMETER SEDIMENT CONTROLS PRIOR TO CLEARING OR CONSTRUCTION ALL CONSTRUCTION SHALL BE CONTAINED WITHIN THE LIMIT OF DISTURBANCE, WHICH SHALL BE MARKED WITH SILT FENCE, SAFETY FENCE, HAY BALES, RIBBONS, OR OTHER MEANS PRIOR TO CLEARING. CONSTRUCTION ACTIVITY
- SHALL REMAIN ON THE UPHILL SIDE OF THE SILT FENCE UNLESS WORK IS SPECIFICALLY CALLED FOR ON THE DOWNHILL SIDE OF THE FENCE. 9. STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS SHALL BE INSTALLED AT START OF CONSTRUCTION AND MAINTAINED THROUGHOUT THE DURATION OF CONSTRUCTION. THE LOCATION OF THE TRACKING PADS
- MAY CHANGE AS VARIOUS PHASES OF CONSTRUCTION ARE COMPLETED. 10. TOPSOIL SHALL BE STRIPPED AND STOCKPILED FOR USE IN FINAL LANDSCAPING. ALL EARTH STOCKPILES SHALL HAVE HAY BALES OR SILT FENCE AROUND THE LIMIT OF PILE. PILES SHALL BE TEMPORARILY SEEDED IF
- PILE IS TO REMAIN IN PLACE FOR MORE THAN 7 DAYS. . SEDIMENTATION TRAPS SHALL PROVIDE 134 CUBIC YARDS OF SEDIMENT STORAGE PER DISTURBED ACRE CONTRIBUTING TO THE BASIN. PROVIDE BASIN VOLUMES FOR ALL DISTURBANCE ON SITE.
- STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS SHALL BE INSTALLED PRIOR TO ANY ON SITE EXCAVATION AND SHALL BE MAINTAINED DURING ALL EXCAVATION AND CONSTRUCTION ACTIVITIES. MINIMIZE LAND DISTURBANCES SEED AND MULCH DISTURBED AREAS WITH TEMPORARY MIX AS SOON AS 13 PRACTICABLE (2 WEEK MAXIMUM UNSTABILIZED PERIOD) USING PERENNIAL RYEGRASS AT 40 LBS PER ACRE
- MULCH ALL CUT AND FILL SLOPES AND SWALES WITH LOOSE HAY AT A RATE OF 2 TONS PER ACRE. IF NECESSARY, REPLACE LOOSE HAY ON SLOPES WITH EROSION CONTROL BLANKETS OR JUTE CLOTH. MODERATELY GRADED AREAS, ISLANDS, AND TEMPORARY CONSTRUCTION STAGING AREAS MAY BE HYDROSEEDED WITH TACKIFIER 14. SILT FENCE AND OTHER SEDIMENT CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH
- CONTRACT DRAWINGS AND MANUFACTURER'S RECOMMENDATIONS PRIOR TO WORK IN ANY UPLAND AREAS. EXCAVATED MATERIAL FROM TEMPORARY SILT TRAPS MUST BE STOCKPILED ON UPHILL SIDE OF SILT FENCE. 16. INSTALL SILT FENCE ACCORDING TO MANUFACTURER'S INSTRUCTION, PARTICULARLY, BURY LOWER EDGE OF FABRIC INTO GROUND. SILT FENCE SHALL BE MIRAFI ENVIROFENCE, AMOCO SILT STOP OR EQUIVALENT
- APPROVED BY THE CIVIL ENGINEER. FILTER FABRIC USED SHALL BE MIRAFI 100X OR EQUIVALENT. SEE SPECIFICATIONS FOR FURTHER INFORMATION. WHERE INDICATED ON EROSION CONTROL PLANS USE NEW HAY BALES AND REPLACE THEM WHENEVER THEIR CONDITION DETERIORATES BEYOND REASONABLE USABILITY. STAKE HAY BALES SECURELY INTO GROUND
- AND BUTT TIGHTLY TOGETHER TO PREVENT UNDERCUTTING AND BYPASSING 18. INSTALL TEMPORARY DIVERSION DITCHES, PLUNGE POOLS, SEDIMENT TRAPS, AND DEWATERING PITS AS SHOWN AND AS NECESSARY DURING VARIOUS PHASES OF CONSTRUCTION TO CONTROL RUNOFF UNTIL UPHILL AREAS ARE STABILIZED. LOCATION OF TEMPORARY SEDIMENT TRAPS WILL REQUIRE REVIEW AND APPROVAL
- BY THE CIVIL ENGINEER AND GOVERNING OFFICIAL. 19. DIRECT ALL DEWATERING PUMP DISCHARGE TO A SEDIMENT CONTROL DEVICE SUCH AS TEMPORARY PITS, SEDIMENT TRAPS OR GRASS FILTERS WITHIN THE APPROVED LIMIT OF DISTURBANCE. DISCHARGE TO STORM DRAINAGE SYSTEM OR SURFACE WATERS FROM SEDIMENT CONTROLS SHALL BE CLEAR
- 20. BLOCK THE OPEN UPSTREAM ENDS OF DETENTION BASIN/SEDIMENTATION BASIN OUTLET CONTROL ORIFICE UNTIL SITE IS STABILIZED. CONVERT TEMPORARY SEDIMENT TRAPS TO PERMANENT DETENTION BASINS ONCE SITE HAS BEEN STABILIZED. CLEAN OUTLET CONTROL STRUCTURES AS NECESSARY AND REMOVE ACCUMULATED SEDIMENT FROM BOTTOM OF BASIN. BLOCK END OF STORM SEWERS IN EXPOSED TRENCHES WITH BOARDS AND SANDBAGS AT THE END OF EACH WORKING DAY WHEN RAIN IS EXPECTED.
- SWEEP AFFECTED PORTIONS OF OFF SITE ROADS ONE OR MORE TIMES A DAY (OR LESS FREOUENTLY IF TRACKING IS NOT A PROBLEM) DURING CONSTRUCTION. OTHER DUST CONTROL MEASURES TO BE USED AS NECESSARY INCLUDE WATERING DOWN DISTURBED AREAS, USING CALCIUM CHLORIDE, AND COVERING LOADS ON DUMP TRUCKS.
- 22. PERIODICALLY CHECK ACCUMULATED SEDIMENT LEVELS IN THE SEDIMENT TRAPS DURING CONSTRUCTION AND CLEAN ACCUMULATED SILT WHEN NECESSARY OR WHEN ONE FOOT OF SEDIMENT HAS ACCUMULATED OR PER SPECIFIC CLEANOUT MARKER ELEVATION. CLEAN ACCUMULATED SEDIMENT FROM CATCH BASIN SUMPS AS NECESSARY AND AS DIRECTED BY THE CIVIL ENGINEER OR OWNER'S CONSTRUCTION REPRESENTATIVE REMOVE ACCUMULATED SEDIMENT FROM BEHIND HAY BALES AND SILT FENCE WHEN LEVEL REACHES HALF THE HEIGHT OF THE HAY BALE OR ONE FOOT AT SILT FENCE. DISPOSE OF SEDIMENT LEGALLY EITHER ON OR OFF SITE.
- 23. IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO ELIMINATE THE POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION.
- 24. ALL PUMPING OF SEDIMENT LADEN WATER SHALL BE THROUGH A SEDIMENT CONTROL BMP, SUCH AS A PUMPED WATER FILTER BAG OR EQUIVALENT SEDIMENT REMOVAL FACILITY, OVER UNDISTURBED VEGETATED AREAS.
- 25. ALL EXCAVATED MATERIAL SHALL BE PLACED ON THE HIGH SIDE OF UTILITY AND STORM PIPE TRENCHES SO AS TO ALLOW THE TRENCH TO INTERCEPT ALL SILT LADEN RUNOFF. 26. CONTRACTOR SHALL ONLY EXCAVATE AS MUCH UTILITY AND STORM PIPE TRENCH WORK AS CAN BE
- COMPLETED, BACKFILLED AND STABILIZED IN ONE DAY SO AS TO LIMIT THE AMOUNT OF OPEN, DISTURBED TRENCHING.
- ANY STOCKPILES OF STRIPPED MATERIALS ARE TO BE PERIODICALLY SPRAYED WITH WATER OR A CRUSTING AGENT TO STABILIZE POTENTIALLY WIND-BLOWN MATERIAL. HAUL ROADS BOTH INTO AND AROUND THE SITE ARE TO BE SPRAYED AS NEEDED TO SUPPRESS DUST. TRUCKS HAULING IMPORT FILL MATERIAL ARE TO BE TARPED TO AID IN THE CONTROL OF AIRBORNE DUST. DURING HIGH WIND EVENTS (20 TO 30 MPH SUSTAINED)
- CONSTRUCTION ACTIVITY SHALL BE LIMITED OR CEASED IF DUST CANNOT BE CONTROLLED BY WETTING. 28. AN AREA SHALL BE CONSIDERED TO HAVE ACHIEVED FINAL STABILIZATION WHEN IT HAS A MINIMUM OF 70% UNIFORM PERENNIAL VEGETATIVE COVER OR OTHER PERMANENT NON-VEGETATIVE COVER WITH A DENSITY SUFFICIENT TO RESIST ACCELERATED SURFACE EROSION AND SUBSURFACE CHARACTERISTICS SUFFICIENT TO RESIST SLIDING OR OTHER MOVEMENTS.
- 29. MAINTAIN ALL PERMANENT AND TEMPORARY SEDIMENT CONTROL DEVICES IN EFFECTIVE CONDITION THROUGHOUT THE CONSTRUCTION PERIOD. UPON COMPLETION OF WORK SWEEP PARKING LOT AND REMOVE ALL TEMPORARY SEDIMENT CONTROLS WHEN AUTHORIZED BY LOCAL GOVERNING AUTHORITY. FILE NOT (NOTICE OF TERMINATION) WITH GOVERNING AUTHORITY RESPONSIBLE FOR REGULATING STORM WATER DISCHARGES FROM CONSTRUCTION ACTIVITIES PER NPDES.
- 30. A MANDATORY SUBMISSION OF MONTHLY MONITORING REPORTS TO THE TOWN OF BRIDGEPORT INLAND WETLANDS AND PLANNING AND ZONING DEPARTMENTS OF ONGOING CONSTRUCTION AND E&S MAINTENANCE, INCLUDING IDENTIFICATION OF SITE CONDITIONS, CONTROL MAINTENANCE AND ANY ACTIONS TAKEN TO ADDRESS PERIODIC SITE STABILITY AND EROSION CONDITIONS.

# RESPONSIBILITY INCLUDES THE PROPER INSTALLATION AND MAINTENANCE OF EROSION CONTROL MEASURES,

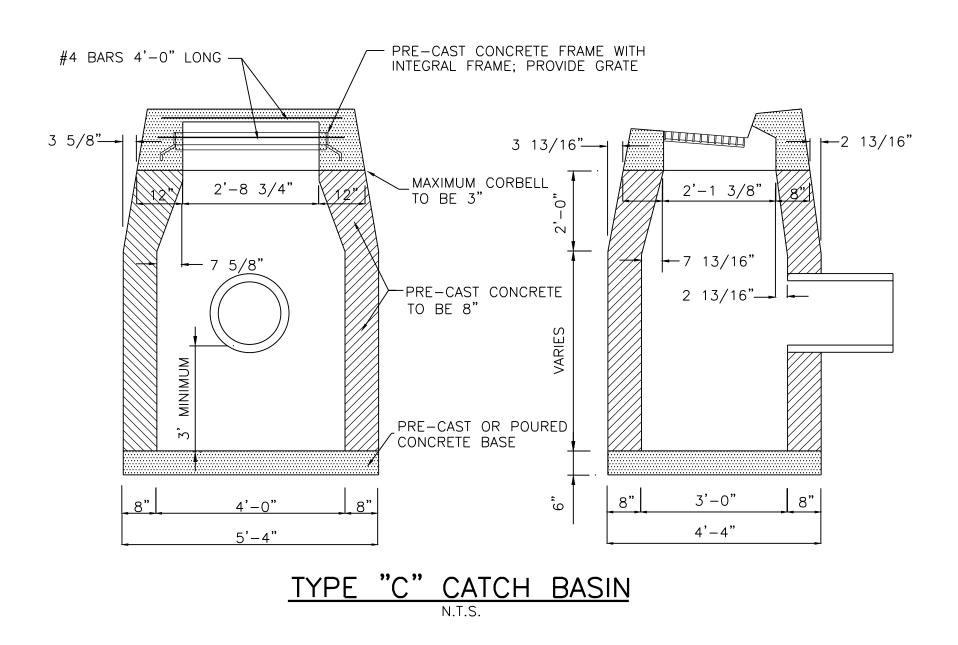


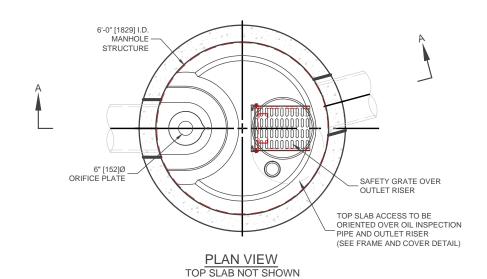


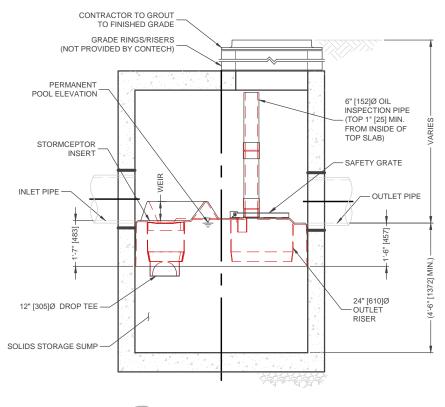


1. SET POSTS AND SILT FENCE — 2. ATTACH FILTER FABRIC WOOD POSTS -EXCAVATE A 6"x6" TO THE POSTS AND EXTENI TRENCH. SET POST IT TO THE TRENCH. DOWNSLOPE. ANGLE 10° UPSLOPE FOR STABILITY AND . BACKFILL THE WHEREVER SILT FENCE RUNS ACROSS OR TRENCH AND COMPACTED -COMPACT THE RPENDICULAR TO SURFACE CONTOURS BACKFILL EXCAVATED SOIL. )-FOOT LONG WINGS OF SILT FENCE PLACED PERPENDICULAR TO THE MAIN RUI OF SILT FENCE SHOULD BE PROVIDED AT APPROXIMATE INTERVALS OF 50-FEET. BOTTOM OI DRAINAGE WAY POINTS "A" SHOULD BE HIGHER THAN POINT "B" PLAN VIEW

SILT FENCE BARRIER DETAIL SCALE: NTS





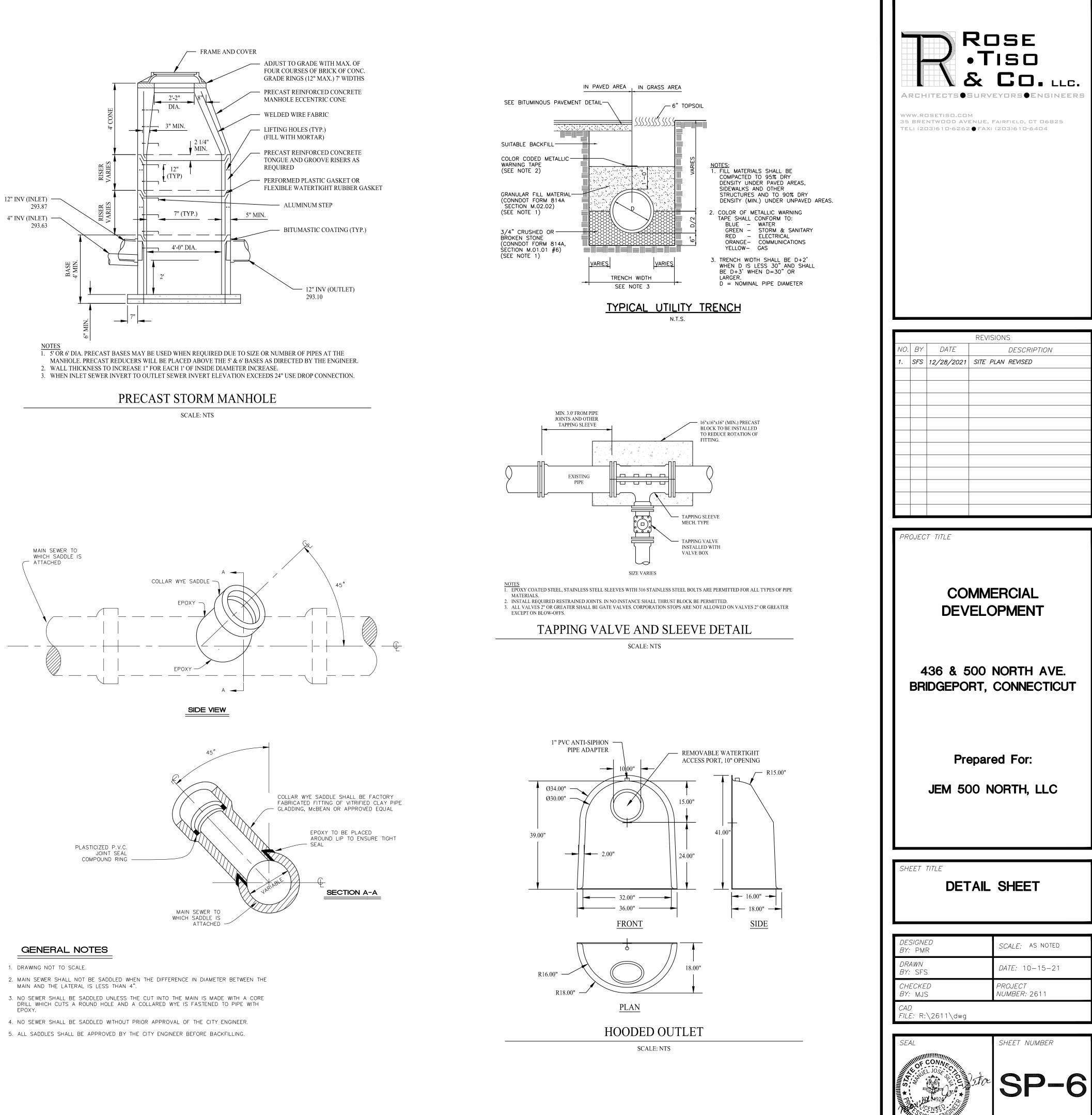


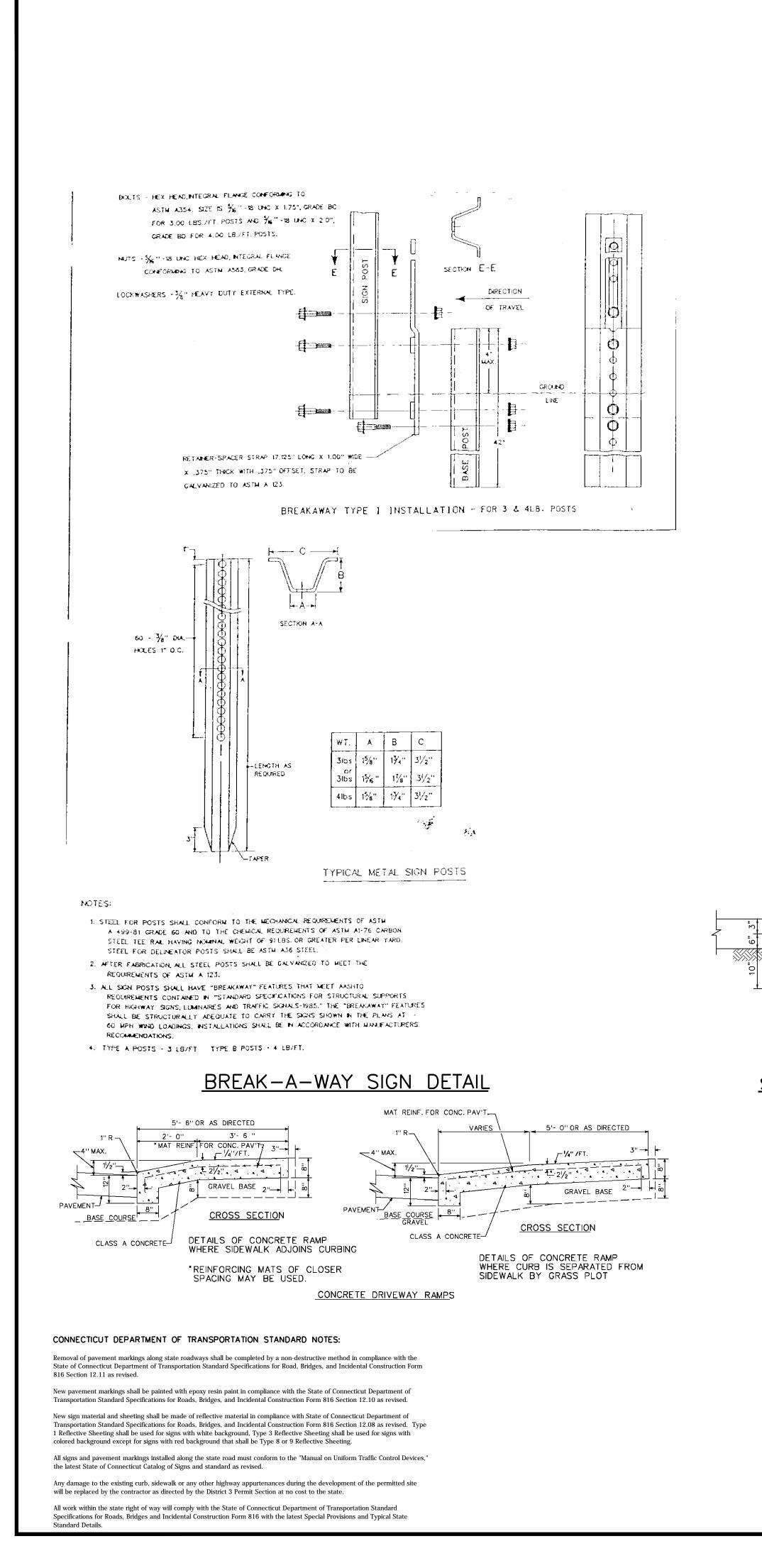
Stormceptor

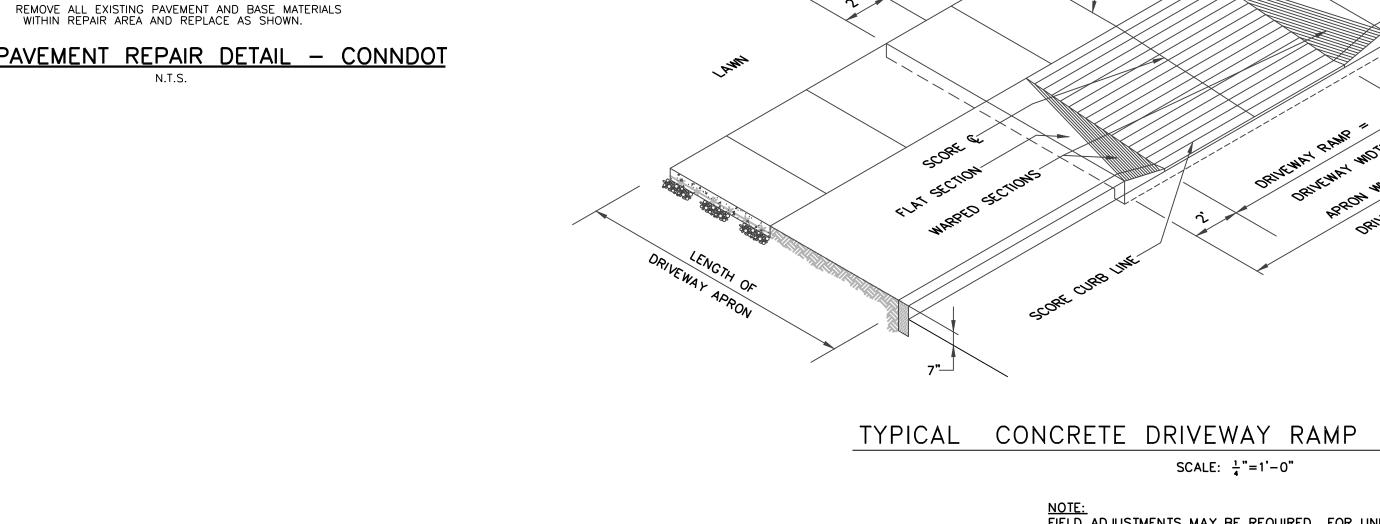
- GENERAL NOTES 1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE. 2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED
- SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com 3. STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS
- DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT. 4. STORMCEPTOR STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' 2' [610], AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
- 5. STORMCEPTOR STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C478 AND AASHTO LOAD FACTOR DESIGN METHOD. 6. ALTERNATE UNITS ARE SHOWN IN MILLIMETERS [mm].
- INSTALLATION NOTES
  A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD. B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMCEPTOR MANHOLE
- STRUCTURE.
- CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
   CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES. E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

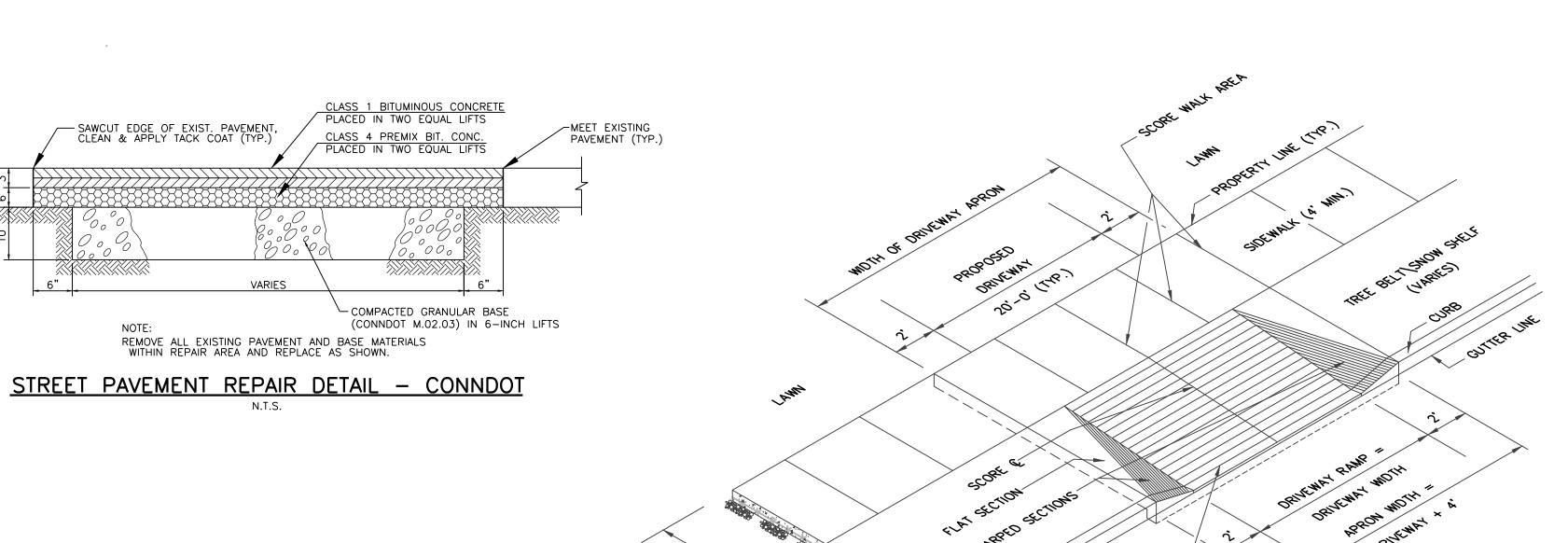
### STORMCEPTOR DESIGN NOTES

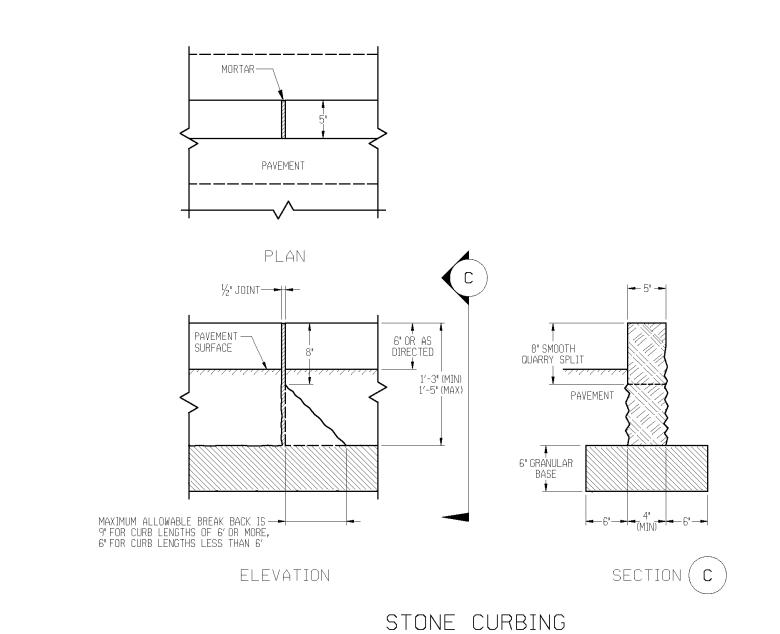
THE STANDARD ###### CONFIGURATION IS SHOWN.







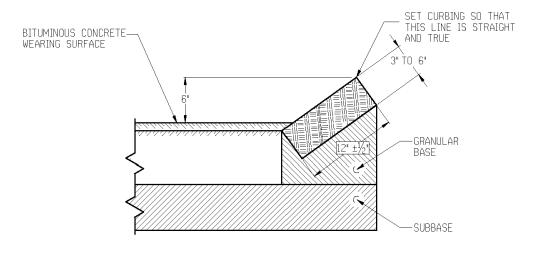




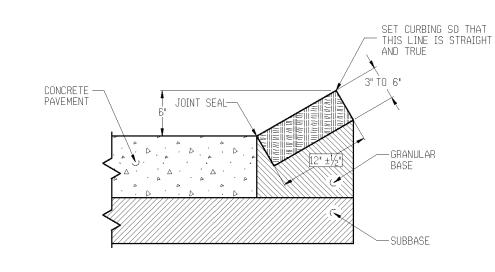
GRANITE SLOPE CURBING

TYPICAL SECTION SHOWING SLOPE CURBING SET ADJACENT TO CONCRETE SURFACES

.2%".



TYPICAL SECTION SHOWING SLOPE CURBING SET ADJACENT TO BITUMINOUS CONCRETE SURFACES



		Rose
		•TISO
		& CO. LLC.
ARCI		SURVEYORS
	OSETISO.CO	лм
35 BR	ENTWOOD A	VENUE, FAIRFIELD, GT 06825 52 ● FAX: (203)610-6404
		•
	_	REVISIONS
NO. BY	DATE	DESCRIPTION

(ISOMETRIC VIEW) SCALE:  $\frac{1}{4}$  = 1'-0"

NOTE: FIELD ADJUSTMENTS MAY BE REQUIRED FOR UNUSUAL EXISTING FEATURES SUCH AS TREES, POWER POLES, ETC.

# COMMERCIAL DEVELOPMENT

436 & 500 NORTH AVE. BRIDGEPORT, CONNECTICUT

Prepared For:

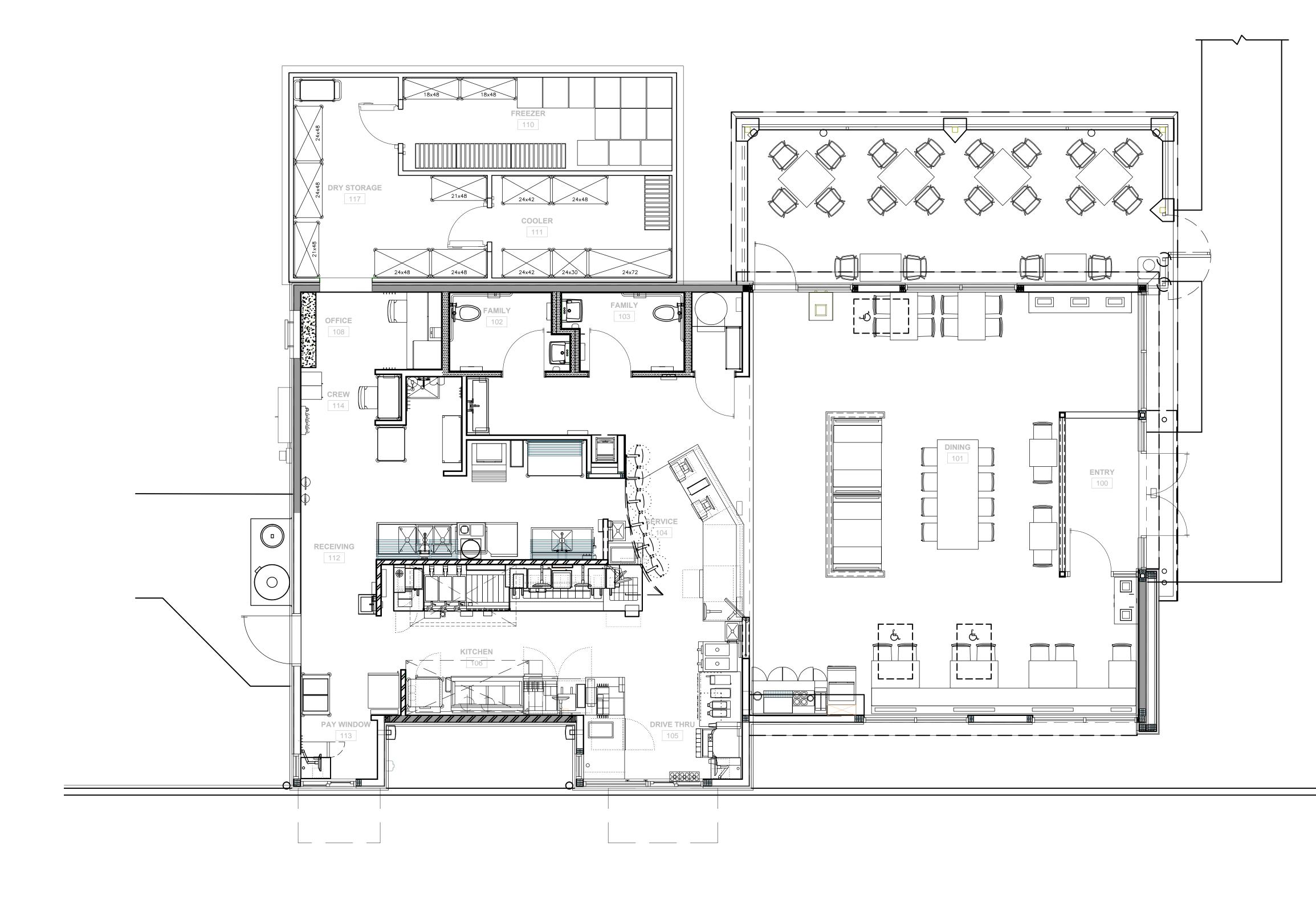
JEM 500 NORTH, LLC

SHEET TITLE

DETAIL SHEET

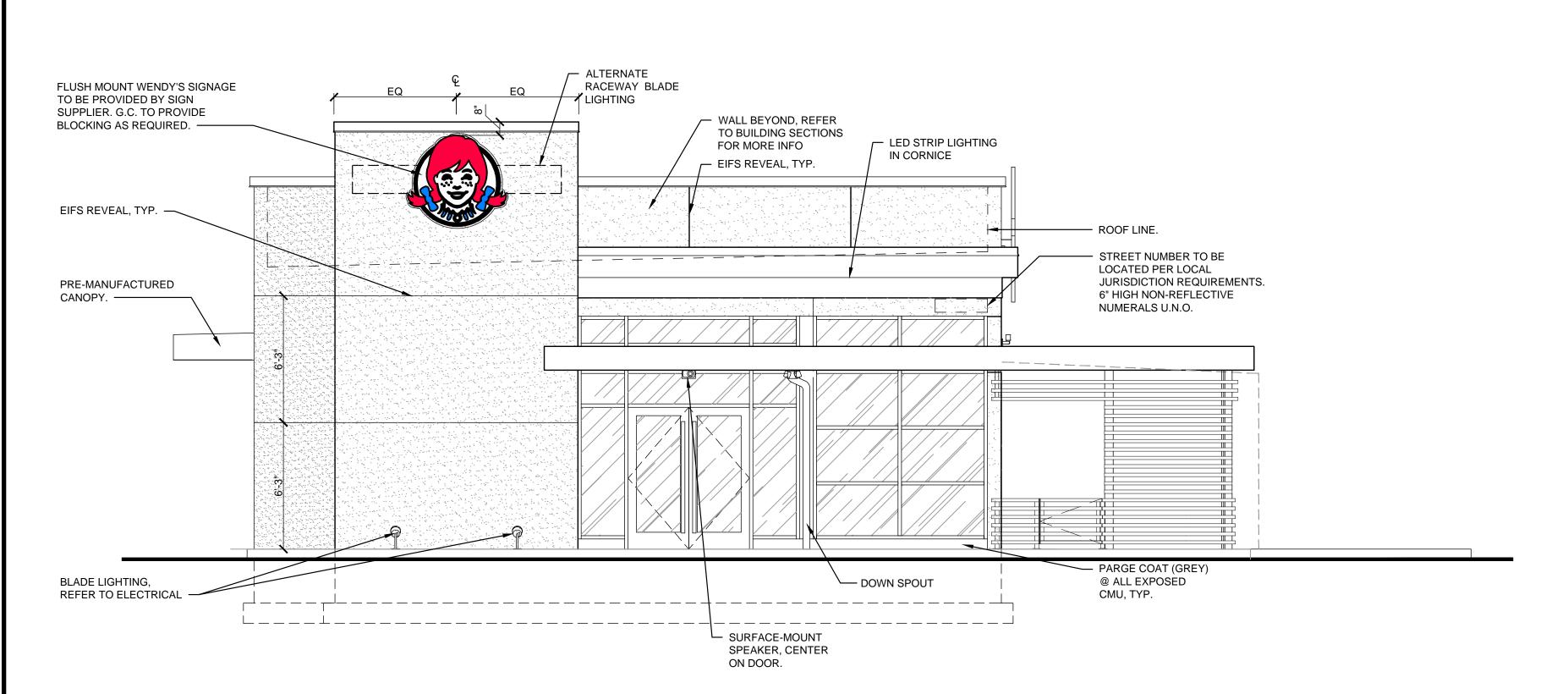
<i>DESIGNED BY</i> : PMR	SCALE: AS NOTED
<i>DRAWN BY:</i> SFS	<i>DATE:</i> 10–15–21
<i>CHECKED BY:</i> MJS	<i>PROJECT NUMBER:</i> 2611
<i>CAD</i> <i>FILE:</i> R:∖2611∖dwg	

SHEET NUMBER SFAI SP-7

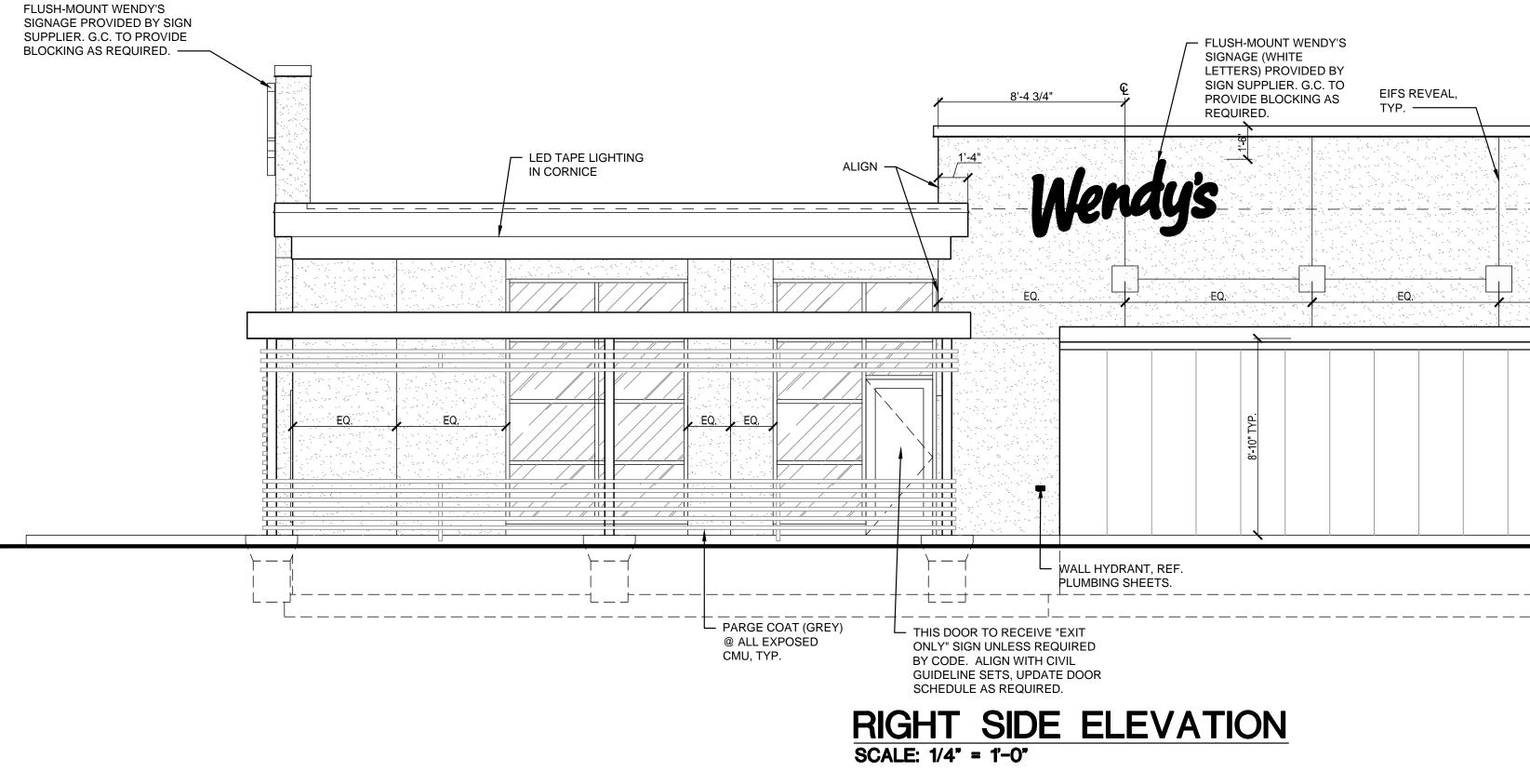


PROPOSED FLOOR PLAN SCALE: 1/4" = 1'-0"

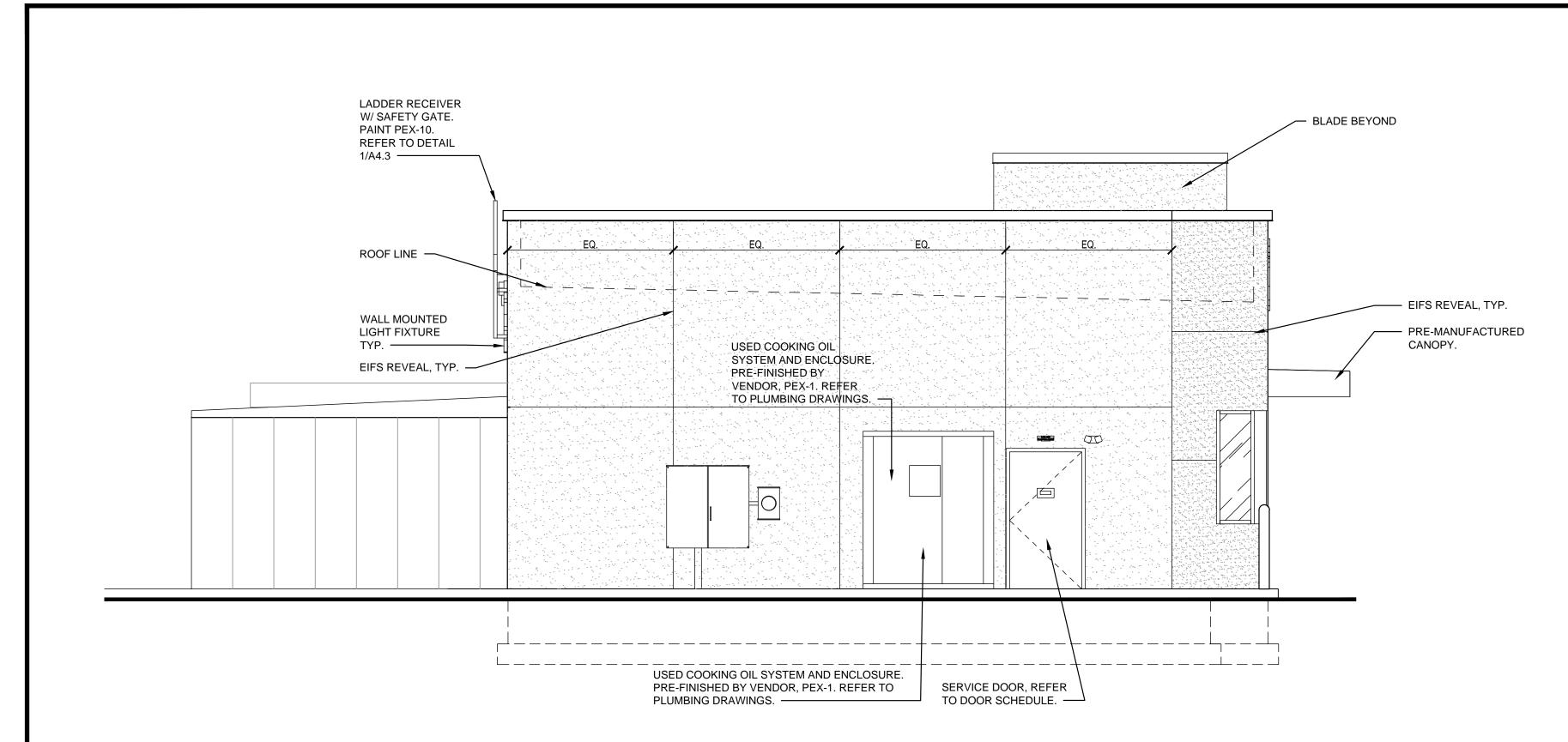
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NO.	BY DATE	DESCRIPTION	
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		Prepared For: 500 NORTH, LLC	
	JEM ₹		
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DES	JEM 8	500 NORTH, LLC FLOOR	
DES BY: DR/	JEM 5	500 NORTH, LLC FLOOR PLAN	
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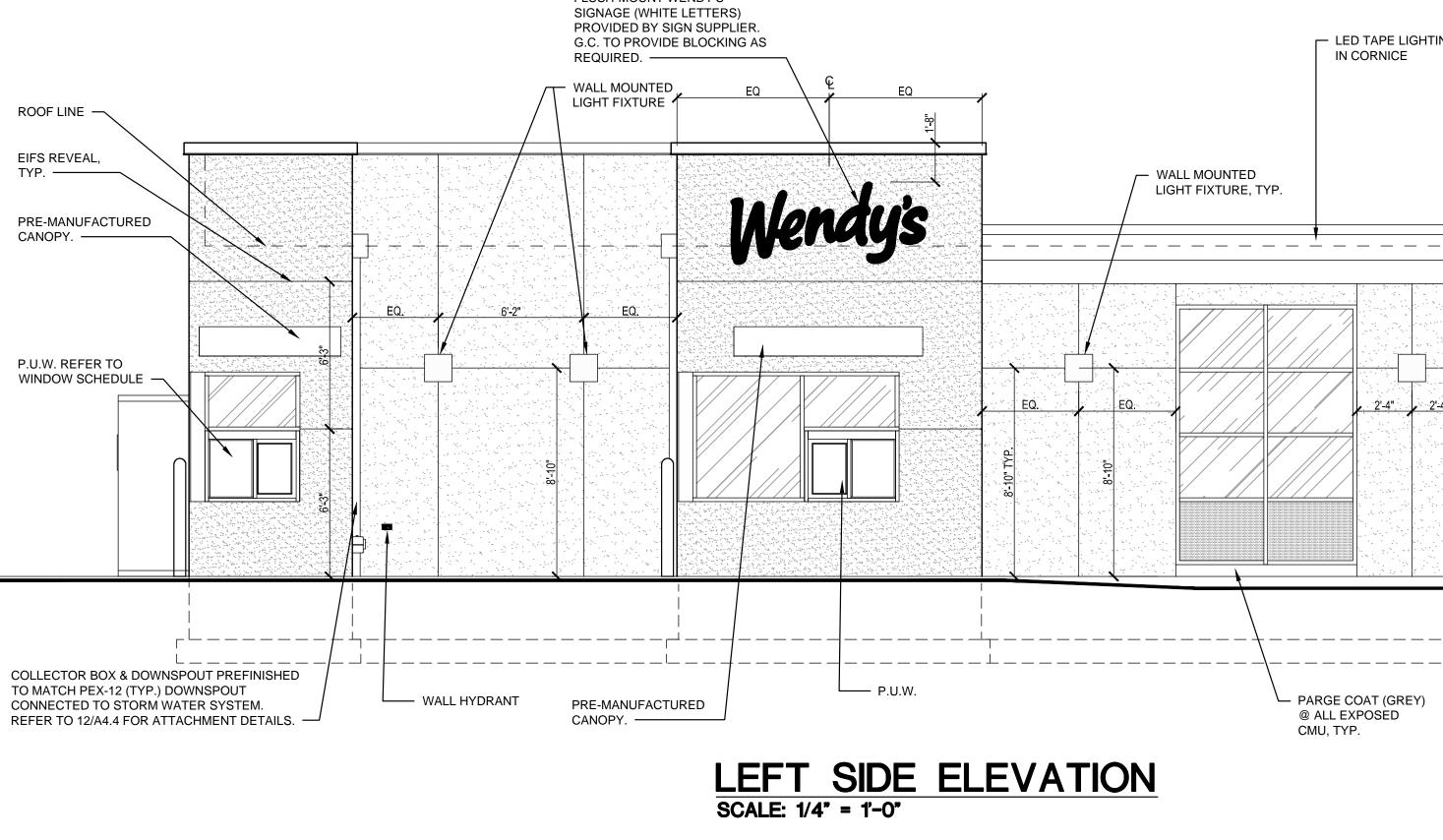
FRONT ELEVATION SCALE: 1/4" = 1'-0"



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ROOF LINE BEYOND LADDER RECEIVER W/ SAFETY GATE. PAINT PEX-10. REFER TO DETAIL 1/A4.4 USED COOKING OIL SYSTEM AND ENCLOSURE. PRE-FINISHED BY VENDOR, PEX-1. REFER TO PLUMBING DRAWINGS.	PROJECT TITLE         COMMERCIAL DEVELOPMENT         A36 & 500 NORTH AVE. BRIDGEPORT, CT 06608         Prepared For:         JEM 500 NORTH, LLC
C/T CABINET & ELECTRIC METER	SHEET TITLE EXTERIOR ELEVATIONS DESIGNED BY: PMR DESIGNED BY: PMR DATE: 10–25–2021 CHECKED BY: PMR DATE: 10–25–2021 CHECKED BY: PMR DATE: 2611 CAD FILE: R:/2611/ARCH_P&Z SEAL SHEET NUMBER A-2.1







FLUSH-MOUNT WENDY'S



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ING	PROJECT TITLE         COMMERCIAL DEVELOPMENT         A36 & 500 NORTH AVE. BRIDGEPORT, CT 06608         Prepared For:         JEM 500 NORTH, LLC
BLADE LIGHTING	SHEET TITLE EXTERIOR ELEVATIONS DESIGNED BY: PMR DESIGNED BY: PMR DATE: 10-25-2021 DRAWN BY: MS CHECKED BY: PMR CHECKED BY: PMR CHECKED BY: PMR CHECKED BY: PMR CHECKED BY: PMR SEAL SEAL SEAL SHEET NUMBER A-2.2

#### SITE ENGINEERING DESIGN REPORT

Proposed Wendy's Bridgeport, Connecticut Job No. 2611

Prepared For: JEM 500 North, LLC

October 28, 2021 *Revised*: December 28, 2021

Prepared By:



WWW.RDSETISD.COM 35 BRENTWOOD AVENUE, FAIRFIELD, CT 06825 TEL: (203)610-6262 • FAX: (203)610-6404

manuel Jose Sto

Manuel J. Silva Project Engineer

C:\Users\user\Documents\Rose Tiso Files\2611\reports\Drainage Report.docx

#### TABLE OF CONTENTS

<u>SECTION</u>	PAGE
Introduction	1
Existing Storm water Runoff	2
Proposed Storm water Drainage	3
Sanitary Sewer	4
FIGURES	
Figure 1 - Existing Drainage Patterns	Attached as C-1
Figure 2 – Proposed Drainage Patterns	Attached as C-2
TABLES	
Table 1	2
Table 2	3
APPENDIX	
Appendix A: Existing and Proposed Stormwater Runoff	
<u>ATTACHMENTS</u>	
C-1 - Pre Development Drainage Patterns	
C-2 - Post Development Drainage Patterns	

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#### **INTRODUCTION:**

JEM 500 north, LLC. is proposing the construction of a new restaurant and a newly reconstructed parking area on two parcels located at 436 & 500 North Avenue in Bridgeport, CT. The proposed restaurant will replace the existing single-story retail building and paved parking area on site.

The property contains a total area of approximately 35,859 square feet. Currently, the site is a developed urban area that is generally impervious. Generally, the site slopes from west to east with a maximum elevation is approximately 43 feet. The minimum elevation is approximately 32 feet.

#### EXISTING STORMWATER RUNOFF

For analysis purposes, the site has been examined as a single drainage area (See Attached Sheet C-1). This single drainage area will be referred to as DA-EX for the balance of this report

The existing site does not have any draining structures or controls; DA-EX drains stormwater to the southwestern corner of the site and onto North Avenue to a series of catch basins in North Avenue. The existing site is completely impervious (96.4%). The proposed design will reduce the impervious area and therefore reduce stormwater runoff quantities.

Peak rates of stormwater runoff, for the 2-year, 10-year, 25-year, and 50-year storm events, have been calculated for the existing site (See Table 1 below). The supporting calculations are included in Appendix A. These calculations are based on the U.S. Soil Conservation Service methodology (TR-55).

These existing flows will later be compared to post-development flows as a means of assessing the impact of the proposed project on surrounding infrastructures.

#### TABLE 1

#### Existing Flows (CFS) Existing Runoff from area to be developed

<u>Da-Ex</u>					
2-year	10-year,	25-year	50-year		
2.88	4.51	5.52	6.30		

#### PROPOSED STORMWATER DRAINAGE

The stormwater control system was designed to minimize the impact on the surrounding infrastructure. This was achieved by routing all catch basins and trench drains to underground stormwater storage on site.

Design details for these systems are presented on Sheet SP-2 (part of the overall Project Documents). The system will drain the one roof on-site, all paved areas, sidewalks, and grassy areas that contribute runoff to the system. The roof and parking lot will be the major elements of the total impervious area on the site. (Calculations included as Appendix A) The roof, grassy areas, sidewalks, and driveways will contribute to the runoff totals seen in table 2. The impervious area (26,570 square feet) for the proposed condition has been reduced from the existing condition's impervious (34,568 square feet). Stormwater flow has been reduced due to the reduction of impervious areas on the site and therefore stormwater volume control is not required. A stormwater infiltration system on the east side of the site has been sized to collect the stormwater quality volume required for the proposed site. (See calculations below)

#### TABLE 2 Proposed Flows (CFS) for new condition (percent reduction)

(-22%)	(-12%)	(-9%)	(-7%)
2.26	3.96	5.01	5.82
2-year	10-year,	25-year	50-year

WATER QUALITY VOLUME COMPUTATION: Site area = 35,860 SF WQv= (P\*Rv\*A); Rv=0.05+0.009\*I Rv= 0.05+0.009\*I= 0.716 WATERSHED INCHES WQv= (0.716"\*35,860)/12=<u>2,140 CF REQUIRED</u> **Provided = 2,200 CF** 

#### SANITARY SEWER

Sanitary Sewer discharge will be through a proposed 6-inch PVC sanitary sewer line to an existing 8 inch sanitary on North Avenue.

Using the technical standards of the <u>Connecticut Public Health Code</u>, the estimated sewage flow is 30 gallons per day per seat in a restaurant. This restaurant development will have 62 seats:

30 gallons per day per seat62 seats x 30 = 1,860 gallons per day average flowAverage Daily Flow = 1.29 gallons per minute

Peak flow estimate = 1.29 gpm x 5 (peaking factor)

= 6.45 gpm peak

= 0.014 cfs peak

#### Other Utilities

All proposed utilities to the site will be through underground utility connections. Electrical service will be from an aboveground electric utility line on North Avenue. Water service will be from an existing water main in North Avenue.

### APPENDIX A STAGE HYDROGRAPHS

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# Hydraflow Table of Contents

#### Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Watershed Model Schematic	1
Hydrograph Return Period Recap	2

### 2 - Year

Summary Report	3
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, ex-da	
Hydrograph No. 2, SCS Runoff, pr-da	
Hydrograph No. 3, Reservoir, UG CHAMBERS	
Pond Report - U.G. CHAMBERS	7

### 5 - Year

Summary Report	. 8
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, ex-da	
Hydrograph No. 2, SCS Runoff, pr-da	
Hydrograph No. 3, Reservoir, UG CHAMBERS	

## 10 - Year

Summary Report	12
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, ex-da	
Hydrograph No. 2, SCS Runoff, pr-da	
Hydrograph No. 3, Reservoir, UG CHAMBERS	

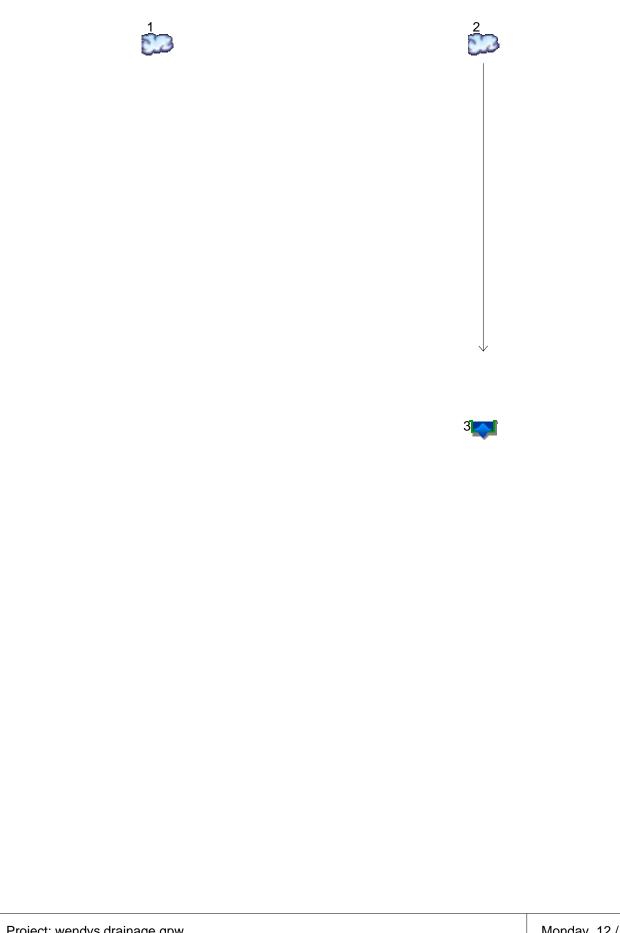
## 25 - Year

Summary Report	16
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, ex-da	
Hydrograph No. 2, SCS Runoff, pr-da	
Hydrograph No. 3, Reservoir, UG CHAMBERS	

## 50 - Year

Summary Report	20
Hydrograph Reports	
Hydrograph No. 1, SCS Runoff, ex-da	
Hydrograph No. 2, SCS Runoff, pr-da	
Hydrograph No. 3, Reservoir, UG CHAMBERS	

# Watershed Model Schematic Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020



1

# Hydrograph Return Period Recap Hydrafiow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

view         opper         type         type <t< th=""><th>Hydrograph Description</th><th colspan="8">Peak Outflow (cfs)</th><th>n Inflow hyd(s)</th><th>Hydrograph type</th><th>lyd. Io.</th></t<>	Hydrograph Description	Peak Outflow (cfs)								n Inflow hyd(s)	Hydrograph type	lyd. Io.
2         SCS Runoff          2.267          3.192         3.960         5.018         5.823          pr-da	Description	100-yr	50-yr	25-yr	10-yr	5-yr	3-yr	2-yr	1-yr	nyu(s)		J.
	ex-da		6.299	5.525	4.512	3.777		2.885			SCS Runoff	
B         Reservoir         2          2.281          3.191         3.957         5.013         5.816          UG CHAMBERS           3          2          2.281          3.191         3.957         5.013         5.816          UG CHAMBERS	pr-da		5.823	5.018	3.960	3.192		2.267			SCS Runoff	2
	UG CHAMBERS		5.816	5.013	3.957	3.191		2.261		2	Reservoir	3

# Hydrograph Summary Report Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	2.885	1	724	9,801				ex-da
2	SCS Runoff	2.267	1	725	7,049				pr-da
2 3	SCS Runoff Reservoir	2.267	1	725	7,049 5,155	2	32.54	2,045	pr-da UG CHAMBERS
Wei	ndys drainag	e.qpw			Return	Period: 2 Yo	ear	Monday, 1	2 / 6 / 2021

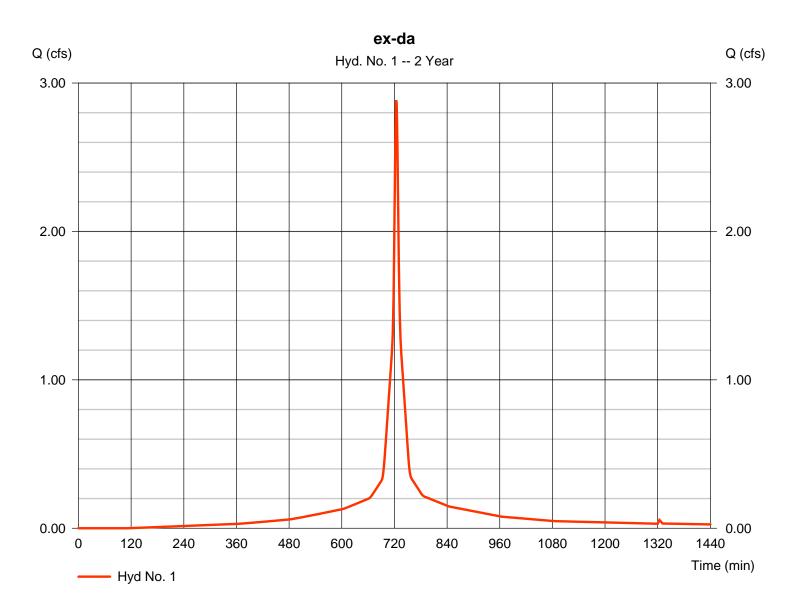
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 1

ex-da

Hydrograph type	= SCS Runoff	Peak discharge	= 2.885 cfs
Storm frequency	= 2 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 9,801 cuft
Drainage area	= 0.830 ac	Curve number	= 97*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 3.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.030 x 61) + (0.800 x 98)] / 0.830



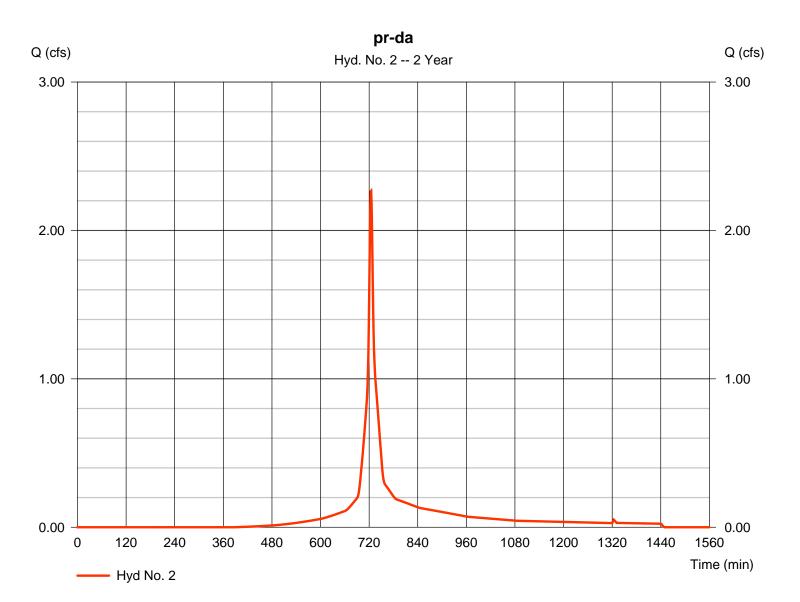
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 2

pr-da

Hydrograph type	= SCS Runoff	Peak discharge	= 2.267 cfs
Storm frequency	= 2 yrs	Time to peak	= 725 min
Time interval	= 1 min	Hyd. volume	= 7,049 cuft
Drainage area	= 0.830 ac	Curve number	= 88*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 3.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.610 x 98) + (0.220 x 61)] / 0.830



5

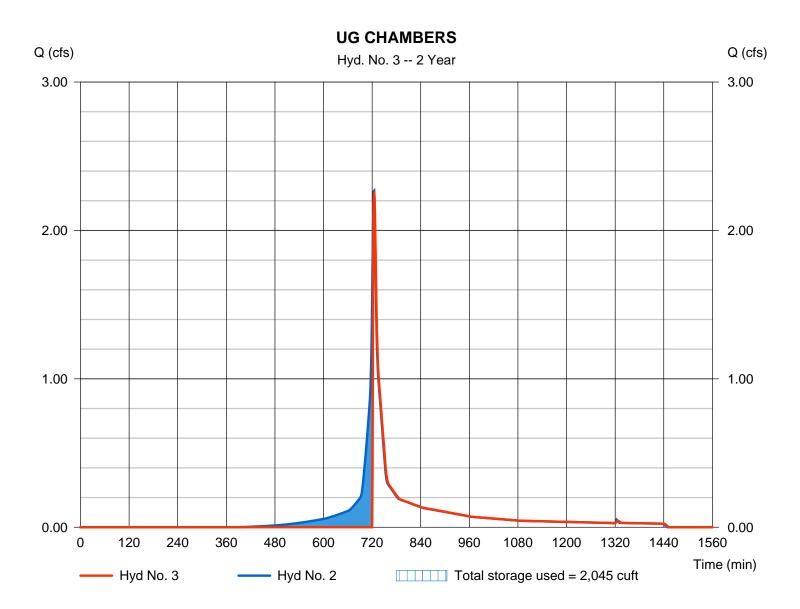
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 3

**UG CHAMBERS** 

Hydrograph type	= Reservoir	Peak discharge	= 2.261 cfs
Storm frequency	= 2 yrs	Time to peak	= 725 min
Time interval	= 1 min	Hyd. volume	= 5,155 cuft
Inflow hyd. No.	= 2 - pr-da	Max. Elevation	= 32.54 ft
Reservoir name	= U.G. CHAMBERS	Max. Storage	= 2,045 cuft

Storage Indication method used.



6

## **Pond Report**

### Pond No. 1 - U.G. CHAMBERS

### **Pond Data**

UG Chambers -Invert elev. = 28.50 ft, Rise x Span = 4.00 x 4.00 ft, Barrel Len = 100.00 ft, No. Barrels = 1, Slope = 0.00%, Headers = No Encasement -Invert elev. = 28.00 ft, Width = 5.00 ft, Height = 5.00 ft, Voids = 66.67%

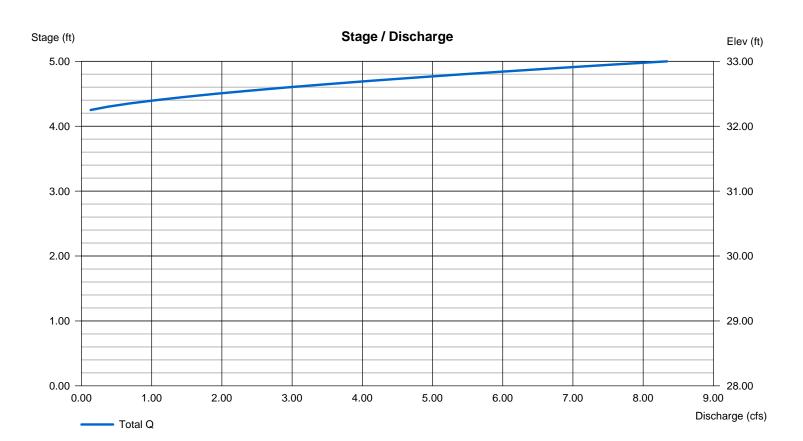
#### Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	28.00	n/a	0	0
0.50	28.50	n/a	167	167
1.00	29.00	n/a	233	400
1.50	29.50	n/a	233	633
2.00	30.00	n/a	233	867
2.50	30.50	n/a	233	1,100
3.00	31.00	n/a	233	1,334
3.50	31.50	n/a	233	1,567
4.00	32.00	n/a	233	1,800
4.50	32.50	n/a	233	2,034
5.00	33.00	n/a	167	2,200

#### **Culvert / Orifice Structures**

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 0.00	Inactive	0.00	0.00	Crest Len (ft)	= 0.00	3.50	0.00	0.00
Span (in)	= 0.00	0.00	0.00	0.00	Crest El. (ft)	= 0.00	32.20	0.00	0.00
No. Barrels	= 0	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 0.00	0.00	0.00	0.00	Weir Type	=	Rect		
Length (ft)	= 0.00	0.00	0.00	0.00	Multi-Stage	= No	No	No	No
Slope (%)	= 0.00	0.00	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by	/ Contour)		
Multi-Stage	= n/a	No	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).



**Weir Structures** 

# Hydrograph Summary Report Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	3.777	1	724	13,015				ex-da
2	SCS Runoff	3.192	1	724	10,048				pr-da
2 3	SCS Runoff Reservoir	3.192	1	724 725	10,048 8,154	2	32.62	2,074	pr-da UG CHAMBERS

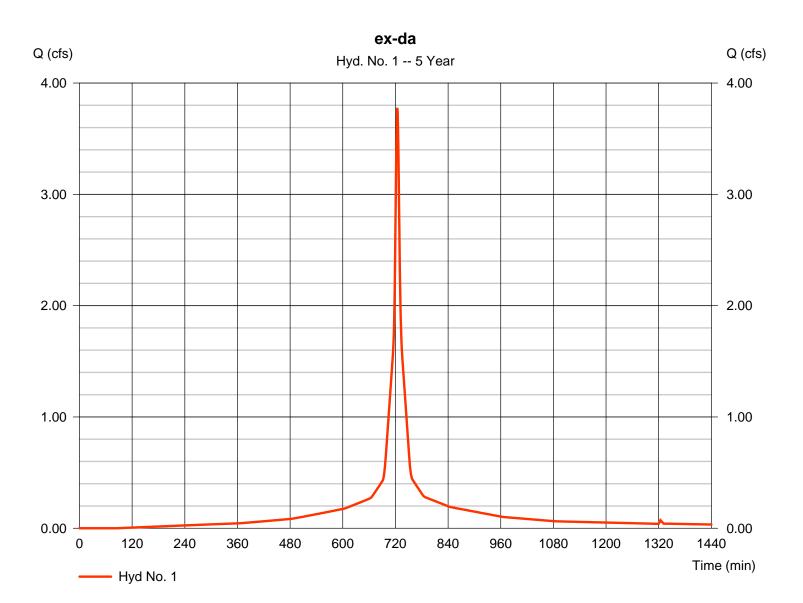
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 1

ex-da

Hydrograph type	= SCS Runoff	Peak discharge	= 3.777 cfs
Storm frequency	= 5 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 13,015 cuft
Drainage area	= 0.830 ac	Curve number	= 97*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 4.54 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.030 x 61) + (0.800 x 98)] / 0.830



Monday, 12 / 6 / 2021

9

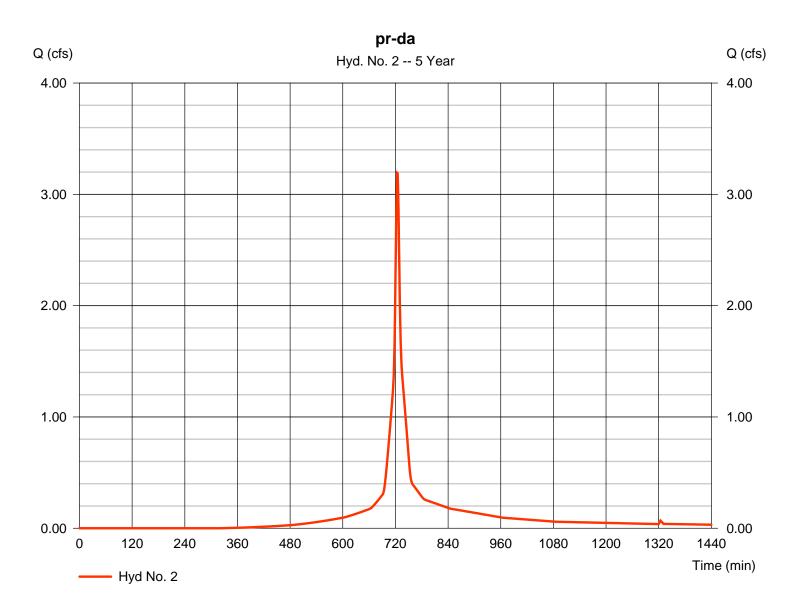
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 2

pr-da

Hydrograph type	= SCS Runoff	Peak discharge	= 3.192 cfs
Storm frequency	= 5 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 10,048 cuft
Drainage area	= 0.830 ac	Curve number	= 88*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 4.54 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.610 x 98) + (0.220 x 61)] / 0.830



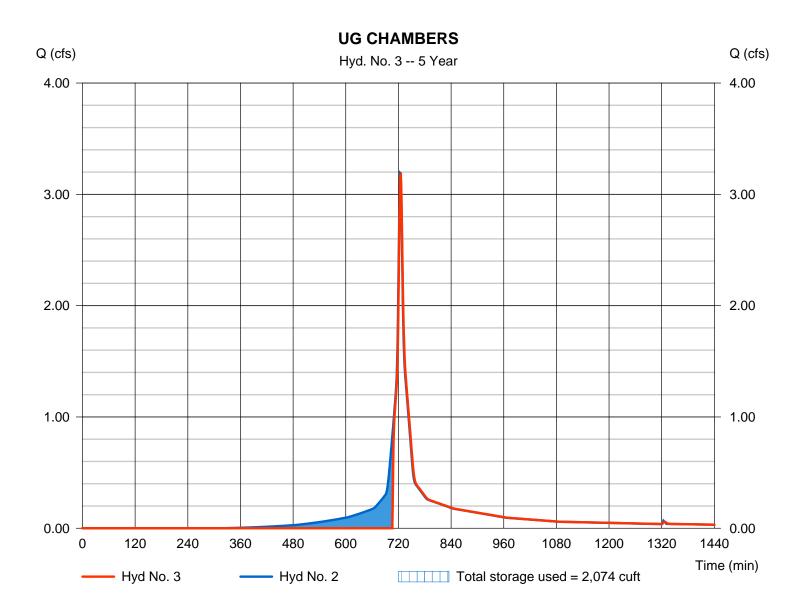
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 3

**UG CHAMBERS** 

Hydrograph type	= Reservoir	Peak discharge	= 3.191 cfs
Storm frequency	= 5 yrs	Time to peak	= 725 min
Time interval	= 1 min	Hyd. volume	= 8,154 cuft
Inflow hyd. No.	= 2 - pr-da	Max. Elevation	= 32.62 ft
Reservoir name	= U.G. CHAMBERS	Max. Storage	= 2,074 cuft

Storage Indication method used.



11

# Hydrograph Summary Report Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	4.512	1	724	15,678				ex-da
2	SCS Runoff	3.960	1	724	12,584				pr-da
23	SCS Runoff Reservoir	3.960		724 725	12,584	2	32.69	2,096	pr-da UG CHAMBERS

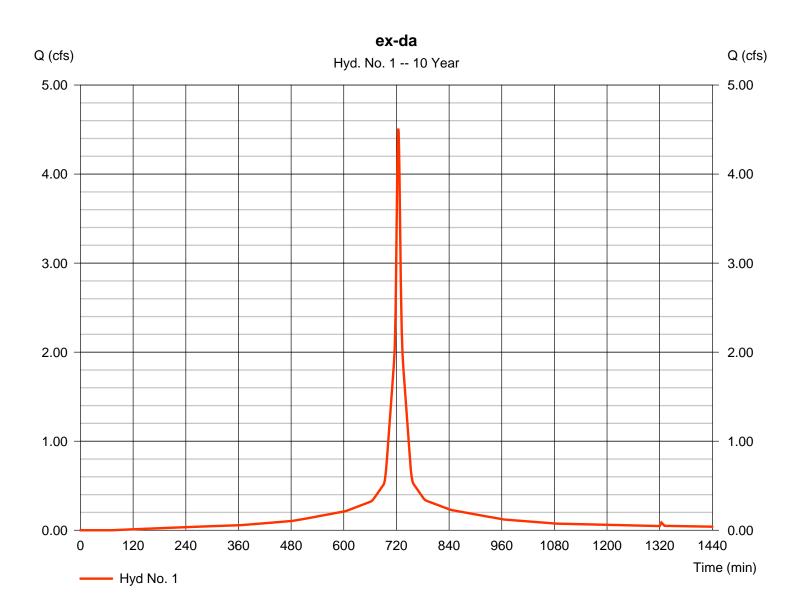
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 1

ex-da

Hydrograph type	= SCS Runoff	Peak discharge	= 4.512 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 15,678 cuft
Drainage area	= 0.830 ac	Curve number	= 97*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 5.40 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.030 x 61) + (0.800 x 98)] / 0.830



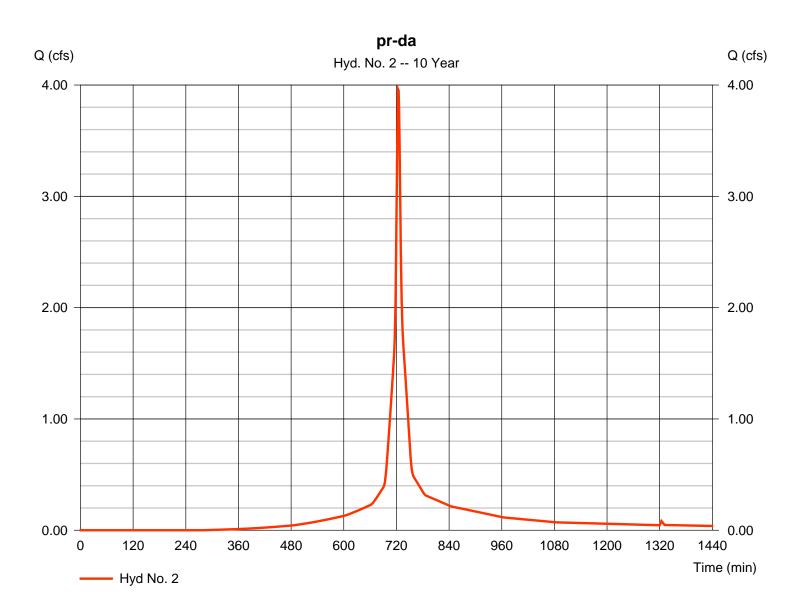
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 2

pr-da

Hydrograph type	= SCS Runoff	Peak discharge	= 3.960 cfs
Storm frequency	= 10 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 12,584 cuft
Drainage area	= 0.830 ac	Curve number	= 88*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 5.40 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484
Drainage area Basin Slope Tc method Total precip.	= 0.830 ac = 0.0 % = User = 5.40 in	Curve number Hydraulic length Time of conc. (Tc) Distribution	= 88* = 0 ft = 6.00 min = Type III

\* Composite (Area/CN) = [(0.610 x 98) + (0.220 x 61)] / 0.830



14

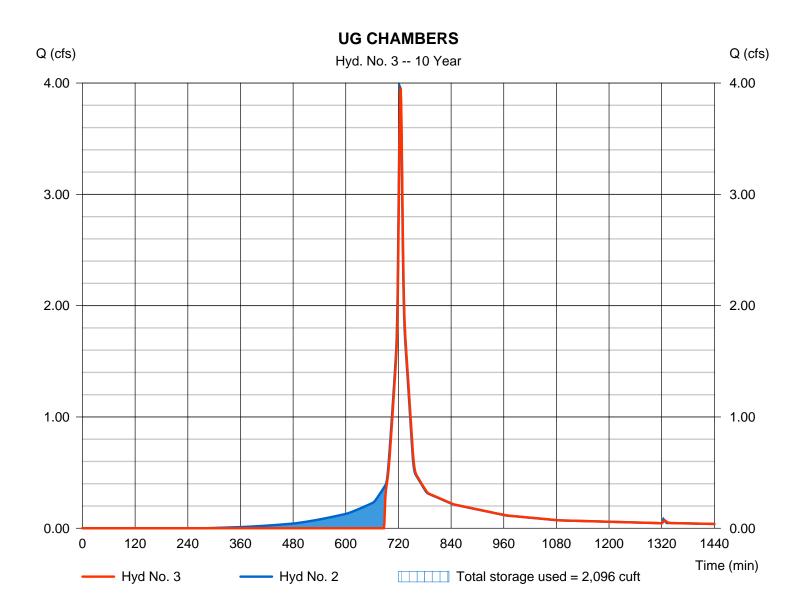
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 3

**UG CHAMBERS** 

Hydrograph type	= Reservoir	Peak discharge	= 3.957 cfs
Storm frequency	= 10 yrs	Time to peak	= 725 min
Time interval	= 1 min	Hyd. volume	= 10,690 cuft
Inflow hyd. No.	= 2 - pr-da	Max. Elevation	= 32.69 ft
Reservoir name	= U.G. CHAMBERS	Max. Storage	= 2,096 cuft

Storage Indication method used.



# Hydrograph Summary Report Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

lyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	5.525	1	724	19,366				ex-da
2	SCS Runoff	5.018	1	724	16,143				pr-da
23	SCS Runoff Reservoir	5.018	1	724 725	16,143	2	32.77	2,124	pr-da UG CHAMBERS
	ndys drainage				Poturo	Period: 25	Voor	Monday, 4	2 / 6 / 2021

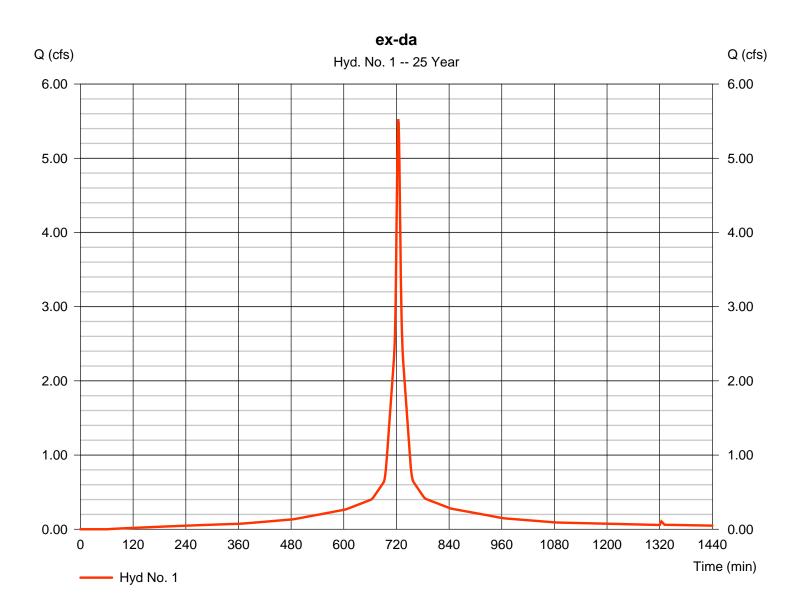
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 1

ex-da

Hydrograph type	= SCS Runoff	Peak discharge	= 5.525 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 19,366 cuft
Drainage area	= 0.830 ac	Curve number	= 97*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 6.59 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.030 x 61) + (0.800 x 98)] / 0.830



17

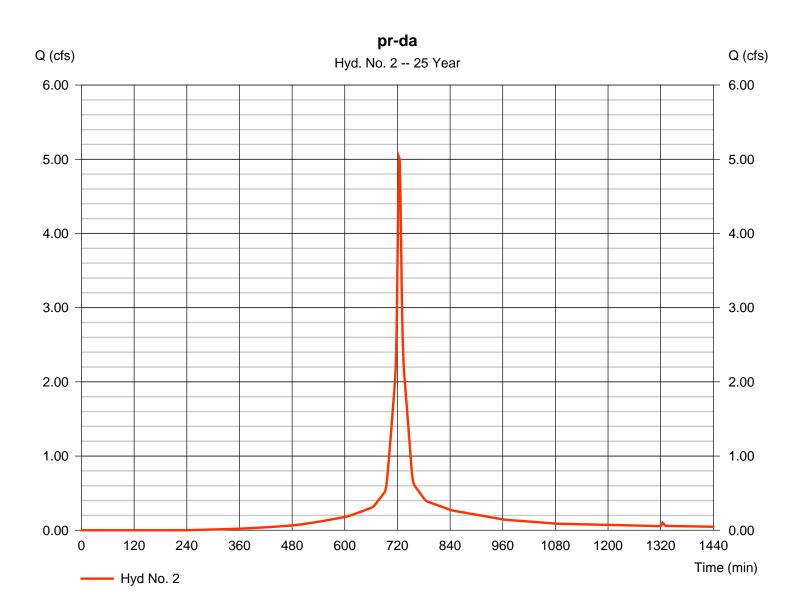
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 2

pr-da

Hydrograph type	= SCS Runoff	Peak discharge	= 5.018 cfs
Storm frequency	= 25 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 16,143 cuft
Drainage area	= 0.830 ac	Curve number	= 88*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 6.59 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.610 x 98) + (0.220 x 61)] / 0.830



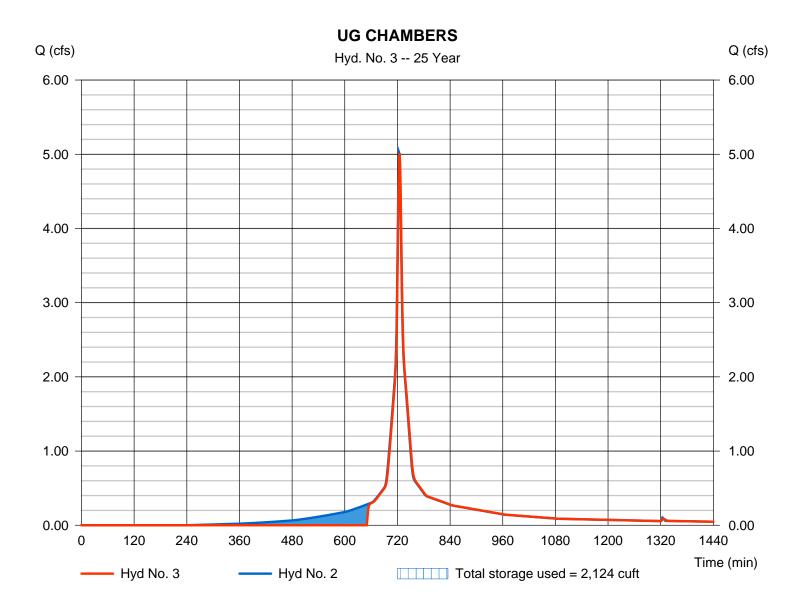
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 3

**UG CHAMBERS** 

= Reservoir	Peak discharge	= 5.013 cfs
= 25 yrs	Time to peak	= 725 min
= 1 min	Hyd. volume	= 14,249 cuft
= 2 - pr-da	Max. Elevation	= 32.77 ft
= U.G. CHAMBERS	Max. Storage	= 2,124 cuft
	= 25 yrs = 1 min = 2 - pr-da	= 25 yrsTime to peak= 1 minHyd. volume= 2 - pr-daMax. Elevation

Storage Indication method used.



# Hydrograph Summary Report Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (cuft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (cuft)	Hydrograph Description
1	SCS Runoff	6.299	1	724	22,188				ex-da
2	SCS Runoff	5.823	1	724	18,891				pr-da
2 3	SCS Runoff Reservoir	5.823	1	724 725	18,891	2	32.83	2,143	pr-da UG CHAMBERS

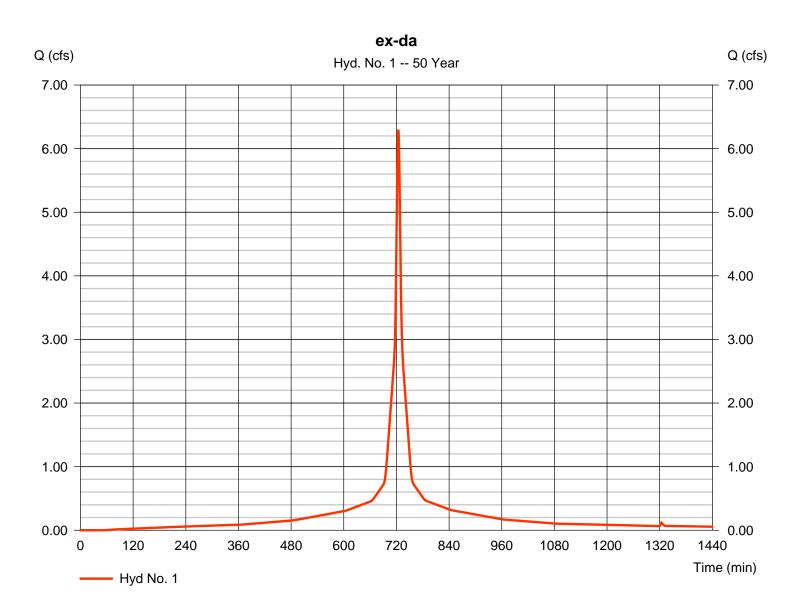
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 1

ex-da

Hydrograph type	= SCS Runoff	Peak discharge	= 6.299 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 22,188 cuft
Drainage area	= 0.830 ac	Curve number	= 97*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 7.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.030 x 61) + (0.800 x 98)] / 0.830



Monday, 12 / 6 / 2021

21

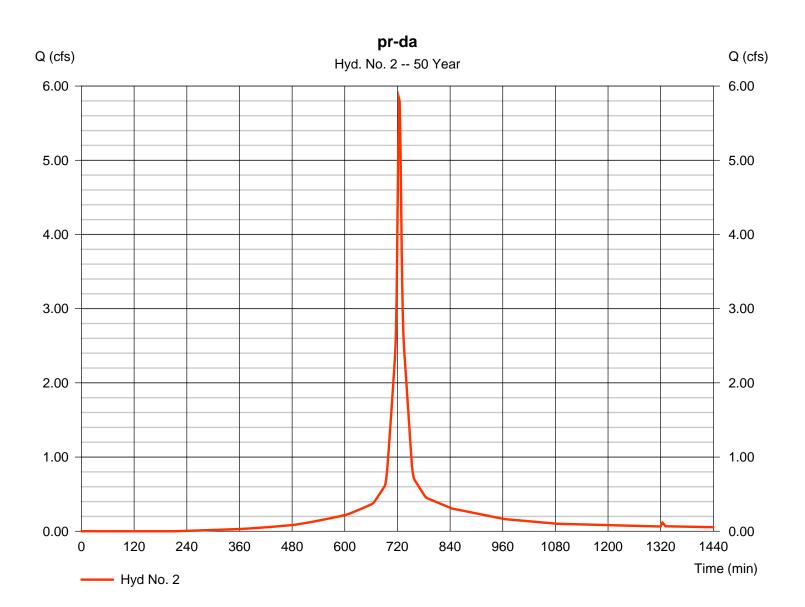
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

## Hyd. No. 2

pr-da

Hydrograph type	= SCS Runoff	Peak discharge	= 5.823 cfs
Storm frequency	= 50 yrs	Time to peak	= 724 min
Time interval	= 1 min	Hyd. volume	= 18,891 cuft
Drainage area	= 0.830 ac	Curve number	= 88*
Basin Slope	= 0.0 %	Hydraulic length	= 0 ft
Tc method	= User	Time of conc. (Tc)	= 6.00 min
Total precip.	= 7.50 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484

\* Composite (Area/CN) = [(0.610 x 98) + (0.220 x 61)] / 0.830



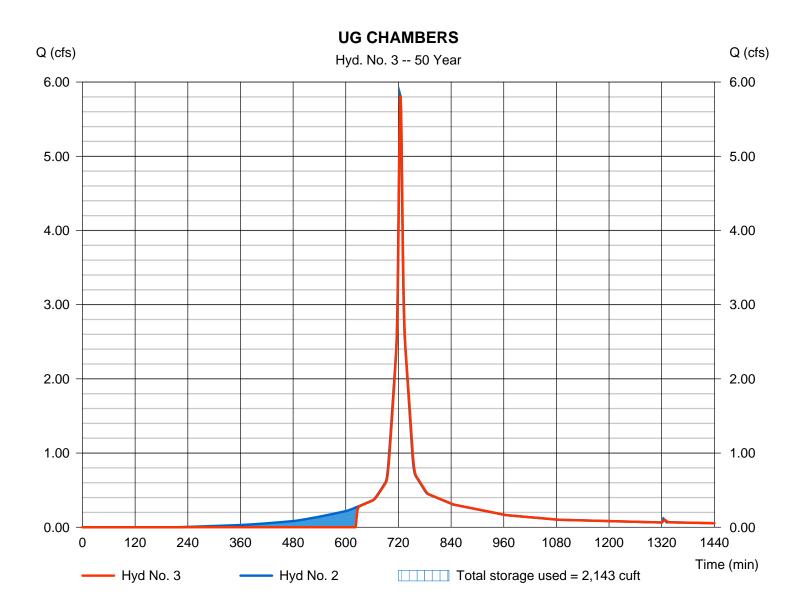
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

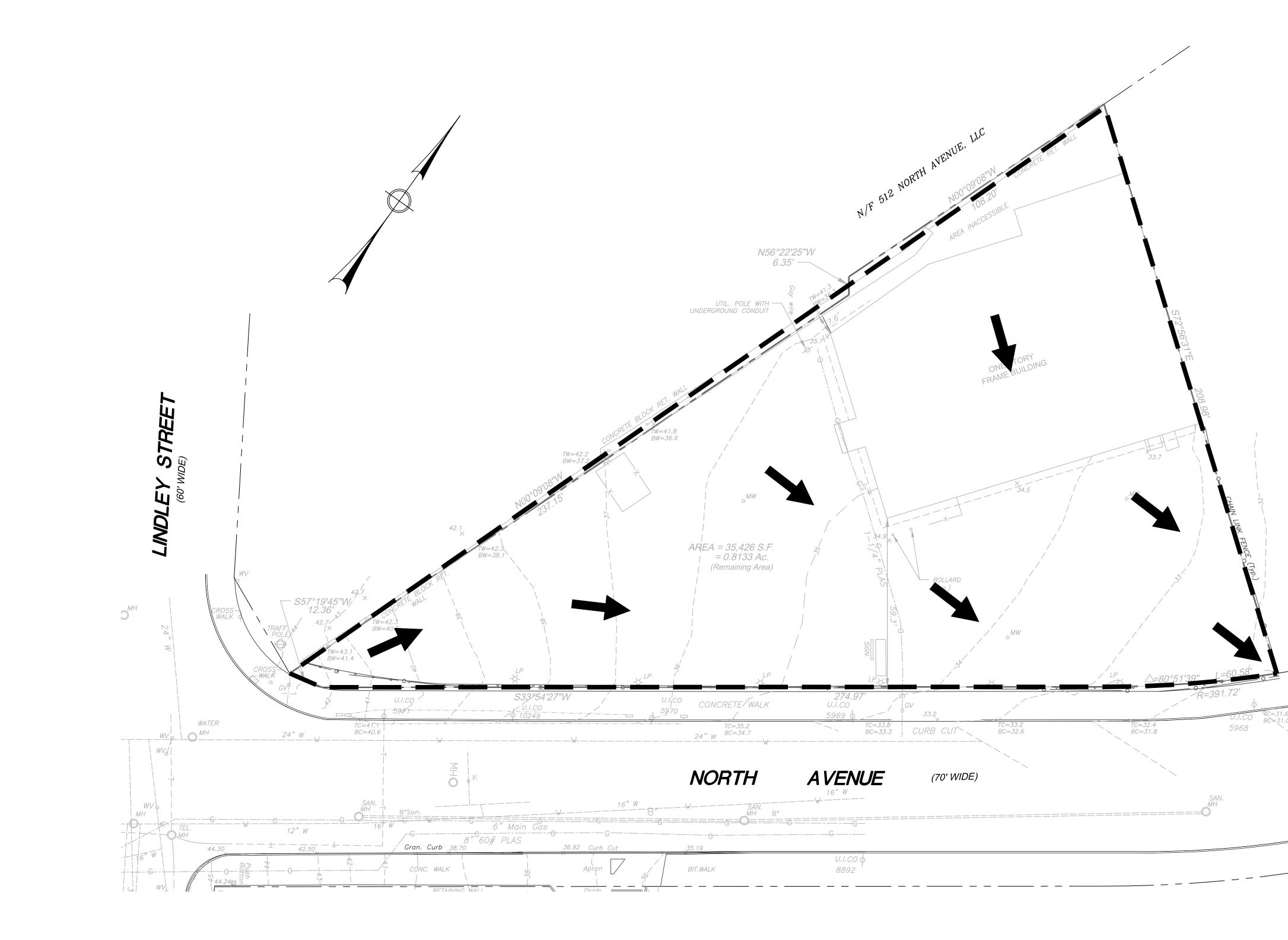
## Hyd. No. 3

**UG CHAMBERS** 

Hydrograph type	= Reservoir	Peak discharge	= 5.816 cfs
Storm frequency	= 50 yrs	Time to peak	= 725 min
Time interval	= 1 min	Hyd. volume	= 16,997 cuft
Inflow hyd. No.	= 2 - pr-da	Max. Elevation	= 32.83 ft
Reservoir name	= U.G. CHAMBERS	Max. Storage	= 2,143 cuft

Storage Indication method used.





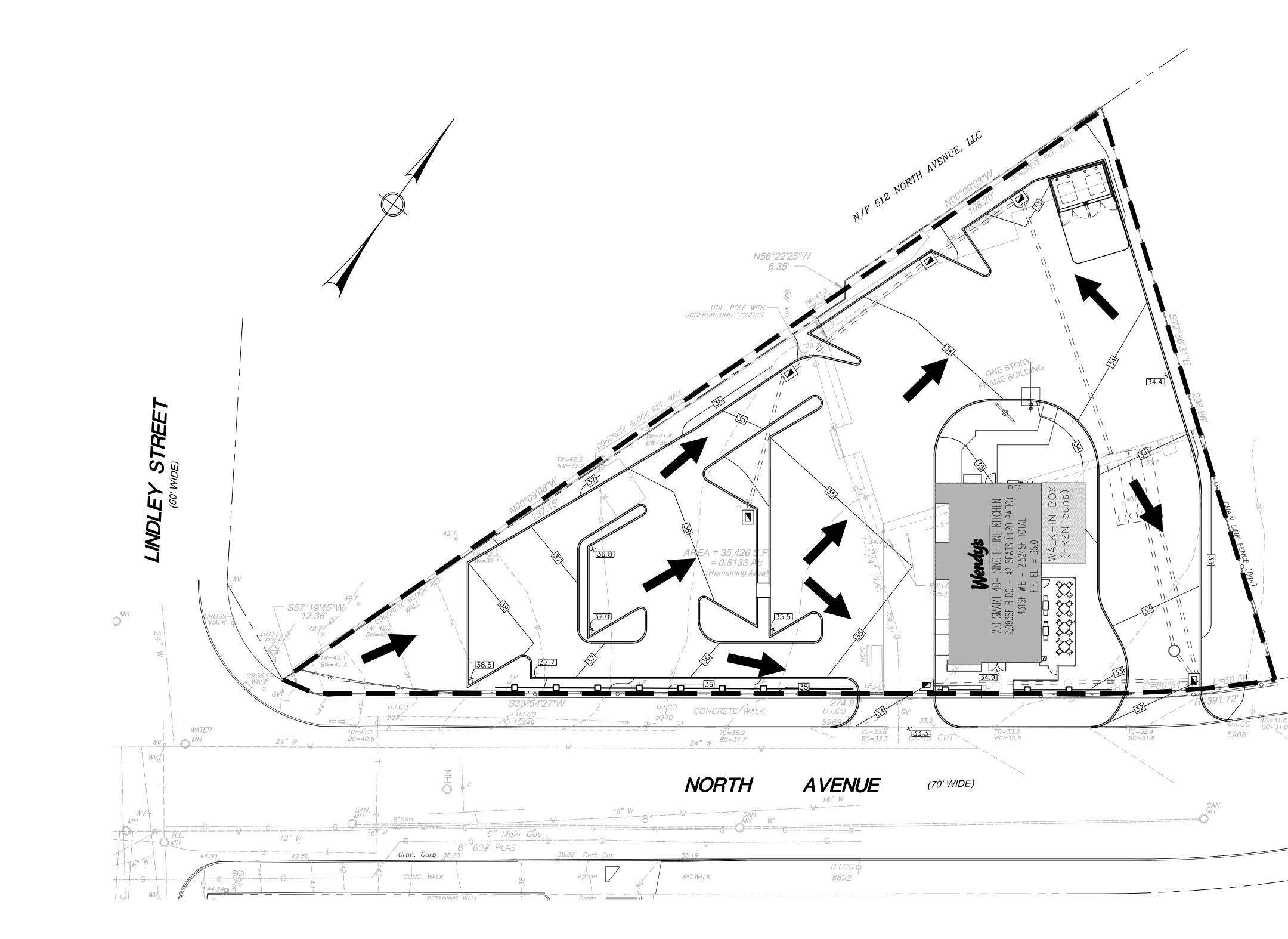
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PROPOSED BUILDING

PROPOSED SPOT ELEVATION EXISTING 2' CONTOUR EXISTING 10' CONTOUR PROPOSED 2' CONTOUR EXISTING CATCH BASIN PROPOSED CATCH BASIN EXISTING MANHOLE PROPOSED MANHOLE EXISTING STORM PIPES PROPOSED STORM PIPES EXISTING SANITARY PIPES PROPOSED SANITARY PIPES

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	BRIDGEPORT, CONNECTICUT
	Prepared For:
	JEM 500 NORTH, LLC
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	DESIGNED
	BY: PMR DRAWN DATE: 10, 15, 21
	BY:SFSCHECKEDPROJECTBY:MJSNUMBER:2611
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EXISTING EDGE OF PAVEMENT PROPOSED EDGE OF PAVEMENT PROPERTY LINE STREAM WETLANDS

PROPOSED BUILDING

PROPOSED SPOT ELEVATION EXISTING 2' CONTOUR EXISTING 10' CONTOUR PROPOSED 2' CONTOUR EXISTING CATCH BASIN PROPOSED CATCH BASIN EXISTING MANHOLE PROPOSED MANHOLE EXISTING STORM PIPES PROPOSED STORM PIPES EXISTING SANITARY PIPES PROPOSED SANITARY PIPES

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## **TRAFFIC IMPACT STUDY**

For

JEM 500 North, LLC Proposed Wendy's Restaurant with Drive-Thru

Property Located at:

436 & 500 North Avenue (U.S. Route 1) Block 2131 – Lot 3 City of Bridgeport, Fairfield County, CT



 1904 Main Street
 245 Main Street, Suite #110

 Lake Como, NJ 07719
 Chester, NJ 07930

 (732) 681-0760

Corey Chase, PE CT PE License #26718

January 5, 2022

4123-99-001TE

CGH



### INTRODUCTION

It is proposed to construct a Wendy's restaurant with drive-thru on a parcel of land located along the west side of North Avenue (U.S. Route 1) between Lindley Street and River Street in the City of Bridgeport, Fairfield County, Connecticut (see Figure 1 in Appendix A). The site is designated as Block 2131 – Lot 3 on the City Tax Maps. The site is currently developed with an industrial building previously occupied by "Medical Laboratory Services, Inc." as well as a food cart known as "La Cabana". It is proposed to raze the site and construct a 2,524 SF Wendy's restaurant with drive-thru ("The Project"). Access to the site is currently provided via one (1) full movement driveway along North Avenue. It is proposed to close the existing access point and construct one (1) ingress only driveway and one (1) right turn egress driveway. Parking will be provided via twenty-five (25) on-site parking spaces.

Dynamic Traffic, LLC has been retained to prepare this study to assess the traffic impact associated with the construction of The Project on the adjacent roadway network. This study documents the methodology, analyses, findings and conclusions of our study and includes:

- A detailed field inspection was conducted to obtain an inventory of existing roadway geometry, traffic control, and location and geometry of existing driveways and intersections.
- Existing traffic data was collected via manual turning movement (MTM) counts during the weekday PM and Saturday midday peak periods at the intersection of North Avenue/Housatonic Avenue and Lindley Street.
- Projections of traffic to be generated by the proposed development were prepared utilizing trip generation data as published by the Institute of Transportation Engineers. Site traffic was then assigned to the adjacent street system based upon the anticipated directional distribution.
- Capacity analyses were conducted for the Existing, No Build, and Build conditions for the study intersections.
- The proposed points of ingress and egress were inspected for adequacy of geometric design, spacing and/or alignment to streets and driveways on the opposite side of the street, relationship to other driveways adjacent to the development, and conformance with accepted design standards.
- The site plan as designed was reviewed for sufficiency in accommodating large wheel base vehicles such as delivery trucks, refuse trucks, and emergency vehicles.
- The parking layout and supply was assessed based on accepted design standards, local requirements, and demand experienced at similar developments.



### **EXISTING CONDITIONS**

A review of the existing roadway conditions near the proposed site was conducted to provide the basis for assessing the traffic impact of the development. This included field investigations of the surrounding roadways and intersections, collection of traffic volume data, and extensive analyses.

### **Existing Roadway Conditions**

The following are descriptions of the roadways in the study area:

<u>North Avenue (U.S. Route 1)</u> is an Urban Principal Arterial roadway under the jurisdiction of the Connecticut Department of Transportation (CTDOT). In the vicinity of the site the posted speed limit is 25 MPH and the roadway generally provides one travel lane in each direction with a general north/south orientation. Curb and sidewalk are provided along both sides of the roadway. North Avenue provides a slightly curved horizontal alignment and an uphill vertical alignment from north to south. The land uses along North Avenue in the vicinity of The Project are a mix of commercial and industrial.

<u>Housatonic Avenue</u> is an Urban Minor Arterial roadway under the jurisdiction of the City of Bridgeport. In the vicinity of the site the posted speed limit is 25 MPH and the roadway provides one travel lane in each direction with a general north/south orientation. Curb and sidewalk are provided along both sides of the roadway. Housatonic Avenue provides a slightly curved horizontal alignment and a relatively flat vertical alignment. The land uses along Housatonic Avenue in the vicinity of The Project are primarily commercial.

<u>Lindley Street</u> is an Urban Minor Arterial roadway under the jurisdiction of the City of Bridgeport. In the vicinity of the site the posted speed limit is 25 MPH and the roadway generally provides one travel lane in each direction with a general east/west orientation. Curb and sidewalk are provided along both sides of the roadway. Lindley Street provides a straight horizontal alignment and a relatively flat vertical alignment. The land uses along Lindley Street in the vicinity of The Project are a mix of commercial, industrial and residential.

### **Existing Traffic Volumes**

Manual turning movement (MTM) counts were conducted on Saturday, December 11, 2021 from 11:00 AM - 2:00 PM as well as on Tuesday, December 14, 2021 from 4:30 - 6:30 PM at the intersection of North Avenue/Housatonic Avenue and Lindley Street.

### **COVID-19 Traffic Count Normalization**

It should be noted that impacts associated with the COVID-19 pandemic may have been in effect as of the time of the traffic counts. As a result, current traffic volumes on the surrounding roadways may be atypical at this time and not entirely representative of "existing" traffic conditions. However, through consultation with CTDOT, traffic volumes in this area have stabilized as of the time the traffic counts were conducted. Thus, no adjustments were applied to the collected data.



Review of the collected traffic data reveals that the weekday evening PSH occurs between 4:30 - 5:30 PM and the Saturday PSH occurs between 1:00 - 2:00 PM. Figure 2, located in Appendix A, show the existing and adjusted peak hour traffic volumes at the study intersections. All traffic counts are contained in Appendix B.

### **Existing Capacity Analysis**

The methodology utilized in the capacity analyses is described in the *Highway Capacity Manual*, published by the Transportation Research Board. In general, the term Level of Service (LOS) is used to provide a "qualitative" evaluation of capacity based upon certain "quantitative" calculations related to empirical values, such as traffic volume and intersection control.

At signalized intersections, factors that affect the various approach capacities include width of approach, number of lanes, signal "green time", turning percentages, truck volumes, etc. However, delays cannot be related to capacity in a simple one-to-one fashion. For example, it is possible to have delays in the Level of Service "F" range without exceeding roadway capacity. Substantial delays can exist without exceeding capacity if one or more of the following conditions exist: long signal cycle lengths; a particular traffic movement experiences a long red time; or progressive movement for a particular lane group is poor. Table I describes the Level of Service ranges for signalized intersections.

An unsignalized (STOP sign controlled) driveway or side street along a through route is seldom critical from an overall capacity standpoint, however, it may be of great significance to the capacity of the minor cross-route, and it may influence the quality of traffic flow on both. When analyzing an unsignalized intersection, it is assumed that both the major street through and right turn movements are unimpeded and have the right-of-way over all side street traffic and left turns from the major street. All other turning movements in the intersection cross, merge with, or are otherwise impeded by major street movements. Traffic delays at unsignalized intersections are determined by sequentially processing these impeded movements. Table II describes the Level of Service ranges for unsignalized (stop controlled) intersections.

	Table I I of Service Criteria gnalized Intersections	 Table II Level of Service Criteria for Unsignalized Intersections		
Level of Service	Average Control Delay (seconds per vehicle)	Level of Service	Average Control Delay (seconds per vehicle)	
А	0.0 to 10.0	а	0.0 to 10.0	
В	10.1 to 20.0	b	10.1 to 15.0	
С	20.1 to 35.0	С	15.1 to 25.0	
D	35.1 to 55.0	d	25.1 to 35.0	
E	55.1 to 80.0	e	35.1 to 50.0	
F	greater than 80.0	f	greater than 50.0	

It should be noted that the analyses within the *Highway Capacity Manual* assume a random arrival for all the movements, which may not be the case if an adjacent traffic signal is present that platoons vehicles, such as the signalized intersection of North Avenue/Housatonic Avenue and Lindley Street.

All capacity analyses were performed utilizing Synchro 11 software. Table III summarizes the existing Levels of Service (LOS) and delays. All capacity analysis calculation worksheets are contained in Appendix C.



Intersection	Direction/Moven	PM PSH	SAT PSH	
North Avenue/Housatonic Avenue & Lindley Street	EB	L	E (55)	D (51)
		Т	E (65)	E (62)
		R	E (78)	E (79)
	WB	L	D (42)	D (40)
	VV D	TR	E (70)	E (62)
	NB (North Ave.)	L	E (60)	E (66)
		Т	C (35)	C (27)
		R	C (25)	C (22)
	NB	L	D (47)	D (46)
	(Housatonic Ave.)	TR	E (71)	E (70)
	C D	L	D (53)	E (56)
	SB	TR	C (30)	C (28)
	Overall	D (47)	D (44)	

Table IIIExisting Levels of Service

A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

The following are discussions pertaining to each of the existing intersections analyzed. It should be noted that the existing percentage of trucks and peak hour factors were used in the existing analysis.

### North Avenue/Housatonic Avenue and Lindley Street

Lindley Street intersects North Avenue/Housatonic Avenue to form a five-leg intersection controlled by a six-phase traffic signal. The eastbound approach of Lindley Street provides a dedicated left turn lane, a dedicated through lane and a dedicated right turn lane. The westbound approach of Lindley Street provides a dedicated left turn lane and a shared through/right turn lane. The northbound approach of North Avenue provides a dedicated left turn lane, a dedicated through lane and a dedicated right turn lane. The northbound approach of Housatonic Avenue provides a dedicated left turn lane and a shared through/right turn lane. The southbound approach of North Avenue provides a dedicated left turn lane and a shared through/right turn lane.

A review of the existing analysis reveals that the intersection operates at overall Levels of Service "D" during the analyzed peak periods. See Table III for the individual movement Levels of Service and delays.



### **FUTURE CONDITIONS**

Traffic volumes and operational analyses were developed for both the No Build and Build conditions. The No Build conditions provide a baseline for assessing the impact of the site development traffic on the roadway system. The process of developing the No Build and Build traffic volumes and the subsequent analyses is outlined below.

Regardless of whether the subject site is developed or not, traffic volumes on the surrounding roadways are expected to increase as a result of developments throughout the region. A growth rate of 0.6% per year, consistent with historical background growth on the surrounding roadway network, was provided by CTDOT and utilized in the analysis.

Through consultation with the City of Bridgeport staff, there is one (1) development in the vicinity of the site that has been approved but not yet constructed that is identified as a significant traffic generator, described below. It was assumed that the background growth rate was adequate to account for the traffic associated with all developments not listed hereafter.

• A self-storage facility containing 900 storage units, located at 141 North Avenue. Projections of the associated traffic volumes were developed using Institute of Transportation Engineers (ITE) publication *Trip Generation*, 11<sup>th</sup> Edition for Land Use Code (LUC) 151 – Mini-Warehouse. The Adjacent Development Traffic Volumes at the study intersections from this development are shown on Figure 3.

Future No Build traffic volumes were developed by applying the background growth rate of 0.6% for two (2) years to the study area roadways existing traffic volumes and adding the traffic volumes associated with the Adjacent Development. Figure 4, in Appendix A, shows the No Build traffic volumes.

### **Traffic Generation**

Trip generation projections for The Project were prepared utilizing trip generation research data as published under Land Use Code 934 – Fast-Food Restaurant with Drive-Through Window in the Institute of Transportation Engineers' (ITE) publication, *Trip Generation*, 11<sup>th</sup> Edition. This publication sets forth trip generation rates based on traffic counts conducted at research sites throughout the country.

According to studies conducted by ITE, traffic associated with LUC 934 is not 100% newly generated. Rather, a portion of the traffic is diverted from the existing traffic stream on the adjacent roadway network. This is because the Wendy's restaurant with drive-thru is not exclusively a destination land use, instead patrons stop on their way to/from other locations such as home or work. ITE identifies a 55% passby traffic percentage, which was used during the evening peak hour. It should be noted that there will realistically be passby traffic during the Saturday midday peak periods as well even though there is no data published by ITE. Therefore, the weekday evening passby percentage of 55% was applied to the Saturday midday peak hour volumes. Table IV below details the traffic volumes associated with the subject project taking into account the passby credits.



Land Use	Тгір Туре	PM PSH			SAT PSH		
		In	Out	Total	In	Out	Total
2,454 SF Wendy's Restaurant with Drive-Thru	Total	43	40	83	71	68	139
	Passby	24	22	46	39	37	76
	New (Primary)	19	18	37	32	31	63

Table IVTrip Generation Considering Passby Traffic

As mentioned previously, the site is currently developed with an industrial building and a food cart which have trip generation potential. However, conservatively no credit was taken for the potential use of the site and all trip generation was considered an increase over vacant land. This accounts for a "worst-case scenario" from a traffic impact perspective.

Once the magnitude of traffic to be generated by the site is known, it is necessary to assign that traffic to the adjacent street system. The distribution of new traffic to the surrounding roadways is based on the location of primary arterial roadways, major signalized intersections and existing traffic patterns. Located in Appendix A, Figures 5-9 illustrate the Primary Traffic Trip Distribution, Primary Site Generated Volumes, Passby Traffic Trip Distribution, Passby Site Generated Volumes and the Total Site Generated Volumes, respectively. The Total Site Generated Volumes assigned to the study area network were then added to the No Build traffic volumes to generate the Build traffic volumes, which are shown in Figure 10.

### Future Capacity Analysis

Operational conditions at the study intersections were analyzed under the No Build and Build conditions and are summarized in Table V below.



	Direction/ Movement		PM PSH		SAT PSH	
Intersection			No Build	Build	No Build	Build
North Avenue/Housatonic Avenue & Lindley Street	EB	L	E (57)	E (59)	D (52)	D (54)
		Т	E (65)	E (65)	E (62)	E (62)
		R	E (79)	E (78)	E (79)	E (78)
	WB	L	D (42)	D (42)	D (40)	D (40)
		TR	E (70)	E (71)	E (63)	E (63)
	NB (North Ave.)	L	E (60)	E (60)	E (66)	E (66)
		Т	D (35)	D (36)	C (29)	C (31)
		R	C (25)	C (25)	C (23)	C (24)
	NB	L	D (47)	D (47)	D (46)	D (46)
	(Housatonic Ave.)	TR	E (72)	E (72)	E (71)	E (71)
	SB	L	D (54)	E (57)	D (54)	E (60)
		TR	C (31)	C (31)	C (28)	C (29)
	Overall		D (48)	D (48)	D (45)	D (45)
North Avenue & North Site Driveway	NB	LT	-	a (8)	-	a (8)
North Avenue & South Site Driveway	EB	R	-	b (11)	-	b (12)

Table VFuture Levels of Service

a (#) - Unsignalized Intersection Level of Service (seconds of delay per vehicle) A (#) - Signalized Intersection Level of Service (seconds of delay per vehicle)

### North Avenue/Housatonic Avenue and Lindley Street

With the addition of site generated traffic, the intersection is anticipated to continue operating at overall Level of Service "D" during the analyzed peak hours. See Table V for the individual movement Levels of Service and delays.

### North Avenue and the North Site Driveway

The north site driveway is proposed to intersect North Avenue to form an unsignalized T-intersection with the site driveway operating as ingress only. The northbound and southbound approaches of North Avenue are proposed to provide a shared left turn/through lane and a shared through/right turn lane, respectively.

As designed, the individual intersection movements are anticipated to operate at Level of Service "A" during the studied peak hours. See Table V for the individual movement Levels of Service and delays.



### North Avenue and the South Site Driveway

The south site driveway is proposed to intersect North Avenue to form an unsignalized T-intersection with the site driveway operating under stop control. The northbound and southbound approaches of North Avenue are each proposed to provide a dedicated through lane. The eastbound approach of the site driveway is proposed to provide a single lane for right turns only.

As designed, the individual intersection movements are anticipated to operate at Level of Service "B" during the studied peak hours. See Table V for the individual movement Levels of Service and delays.



### SITE PLAN

### Site Access and Circulation

The site plan was reviewed with respect to the site access and on-site circulation design. As noted previously, access to The Project will be provided via one (1) ingress only driveway and one (1) right turn egress driveway along North Avenue.

The newly constructed parking lot will be serviced by one-way parking aisles with minimum widths of 18', which satisfy the Ordinance's minimum requirement of 18' for one-way parking aisles with access to 60-degree parking. The drive-thru will operate in a counterclockwise direction with the ability to stack eleven (11) cars in the drive-thru lane. Review of the site plan design indicates that the site can sufficiently accommodate the automobile traffic anticipated.

### Parking

The site as proposed provides 25 parking spaces, which meets the Ordinance requirements. It should also be noted that the drive-thru service will provide customers with a faster, more convenient option for picking up their food than walk-in service. As such, the parking demand of the proposed site will be lessened by providing a drive-thru lane. Based on past experience, it is expected that approximately 2/3's of the customers will utilize the drive-thru system, thus 1/3 will park and walk in.

As can be seen in Table IV, the maximum number of entering trips anticipated is 71, which occurs during the Saturday midday peak hour. Based on the characteristics described above, approximately 1/3 of customers will still park and walk into the restaurant which equates to a maximum parking demand of 24 vehicles. Additionally, the parking spaces will be high-turnover in nature meaning they will not be occupied for long durations. Thus, it is concluded that the proposed 25 spaces will be sufficient to support the maximum anticipated demand of The Project.

It is proposed to provide parking stalls with dimensions of 9'x18', which are consistent with accepted engineering design standards and satisfy the Ordinance minimum requirements. Therefore, the proposed dimensions will adequately accommodate the anticipated vehicle population.



### FINDINGS & CONCLUSIONS

#### Findings

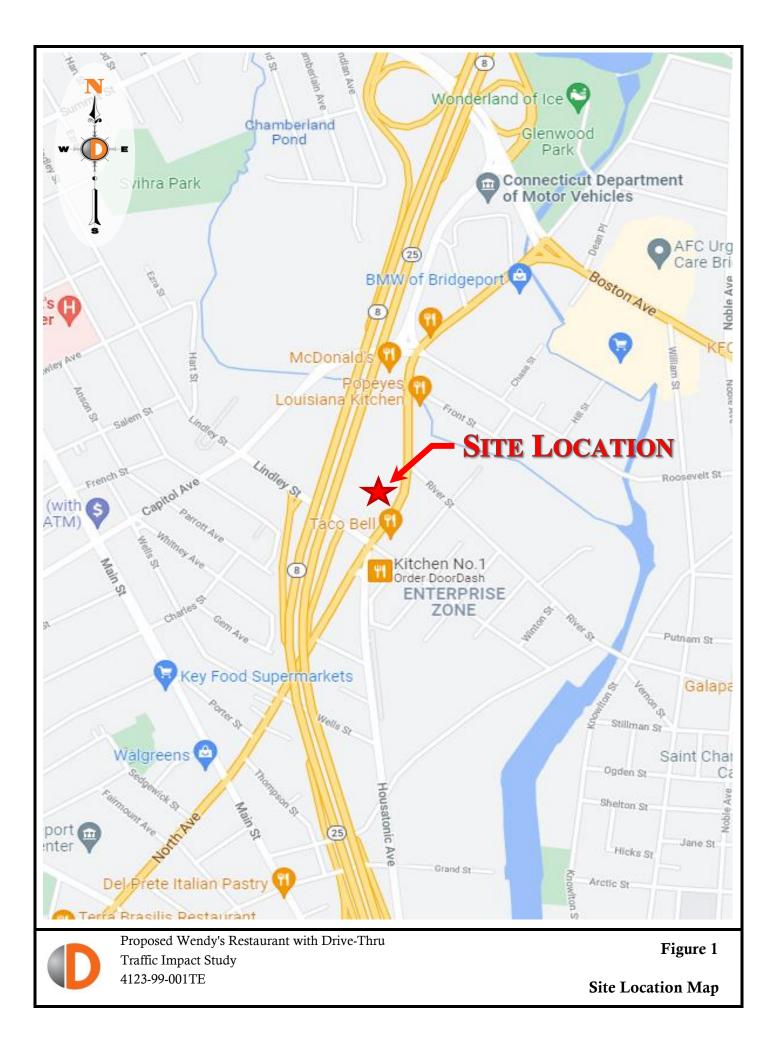
Based upon the detailed analyses as documented herein, the following findings are noted:

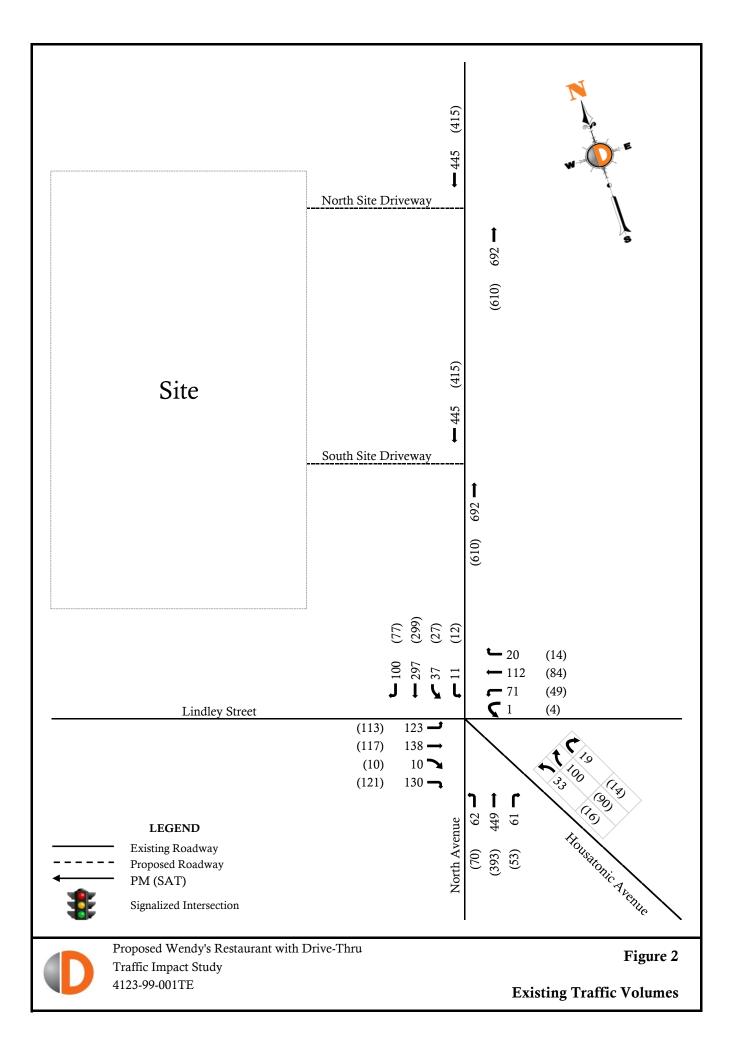
- The proposed 2,524 SF Wendy's restaurant with drive-thru is projected to generate 19 entering trips and 18 exiting trips during the evening peak hour, and 32 entering trips and 31 exiting trips during the Saturday peak hour that are "new" to the adjacent roadway network.
- Access to the site is will be provided via one (1) ingress only driveway and one (1) right turn egress driveway along North Avenue.
- With the addition of site generated traffic, the intersection of North Avenue/Housatonic Avenue and Lindley Street is anticipated to continue operating at overall Level of Service "D" during the analyzed peak hours.
- As designed, the individual intersection movements of North Avenue and the north site driveway are anticipated to operate at Level of Service "A" during the analyzed peak hours.
- As designed, the individual intersection movements of North Avenue and the south site driveway are anticipated to operate at Level of Service "B" during the analyzed peak hours.
- As proposed, The Project's site driveways and internal circulation have been designed to provide for safe and efficient movement of automobiles and large wheel base vehicles.
- The proposed parking supply and design is sufficient to support the maximum anticipated demand and is consistent with past experience at similar developments.

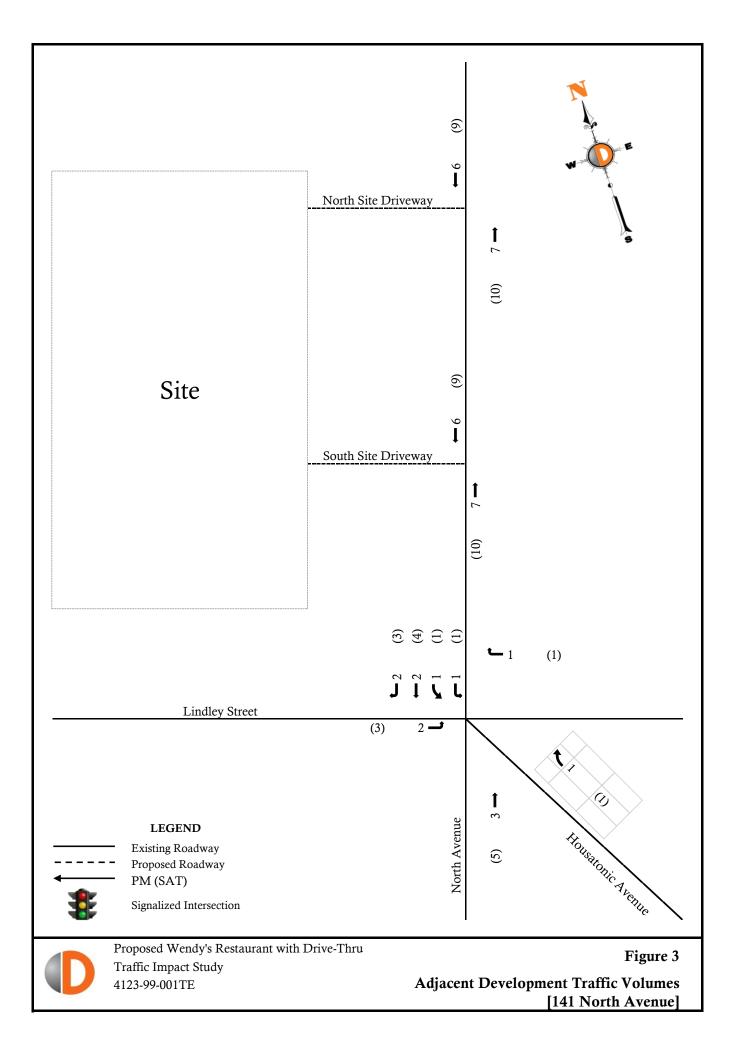
#### Conclusions

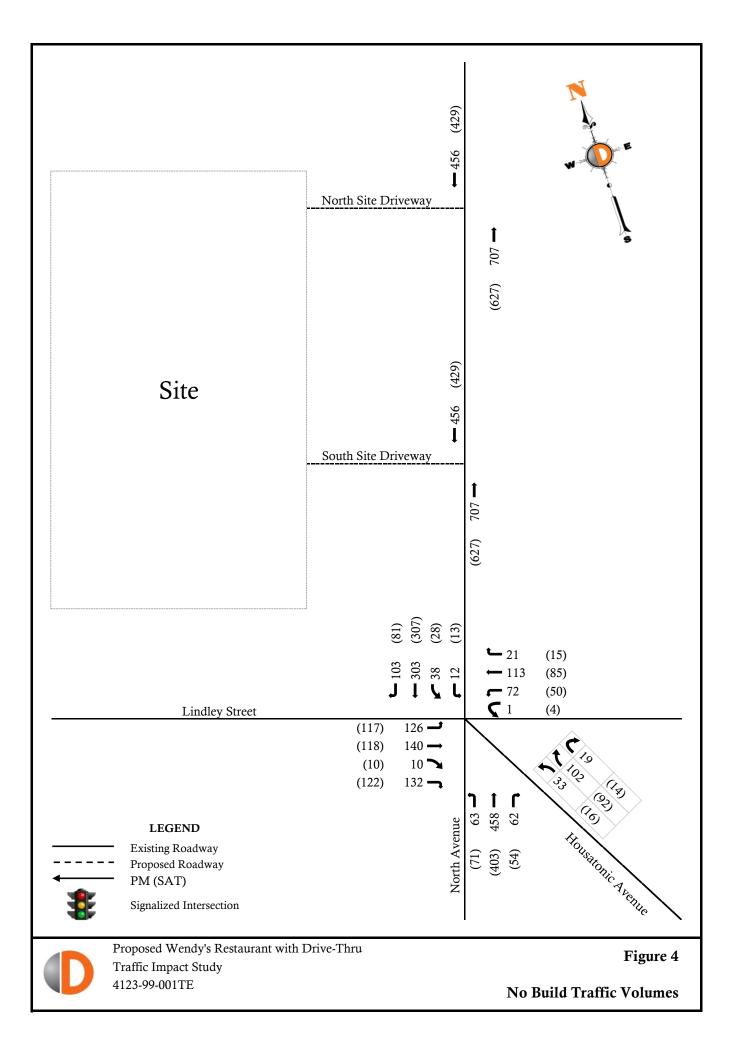
Based upon our Traffic Impact Study as detailed in the body of this report, it is the professional opinion of Dynamic Traffic, LLC that the adjacent street system of the City of Bridgeport and CTDOT will not experience any significant degradation in operating conditions with the construction of The Project. The site driveways are located to provide safe and efficient access to the adjacent roadway system. The site plan as proposed provides for good circulation throughout the site and provides adequate parking to accommodate The Project's needs.

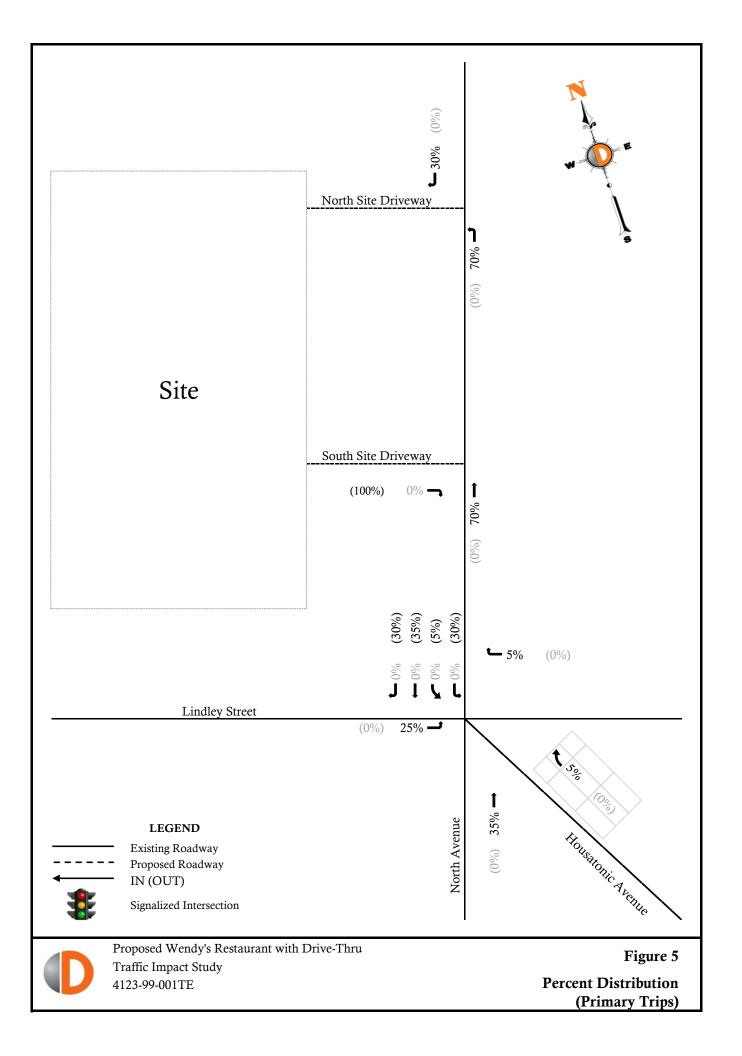
Appendix A Traffic Volume Figures

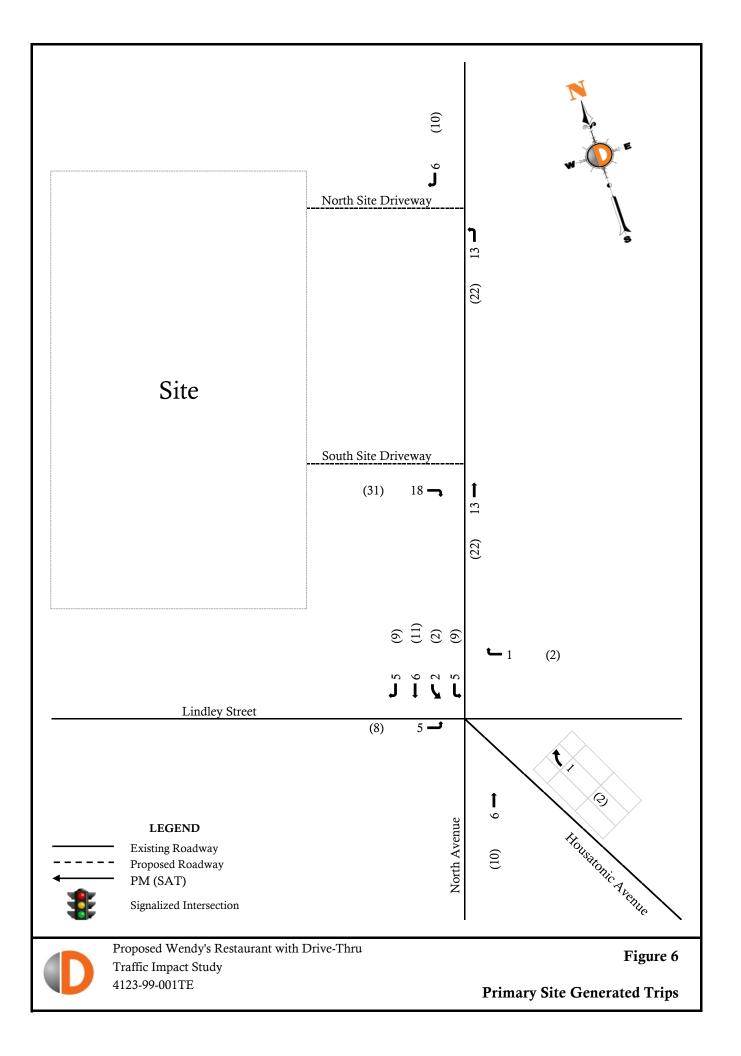


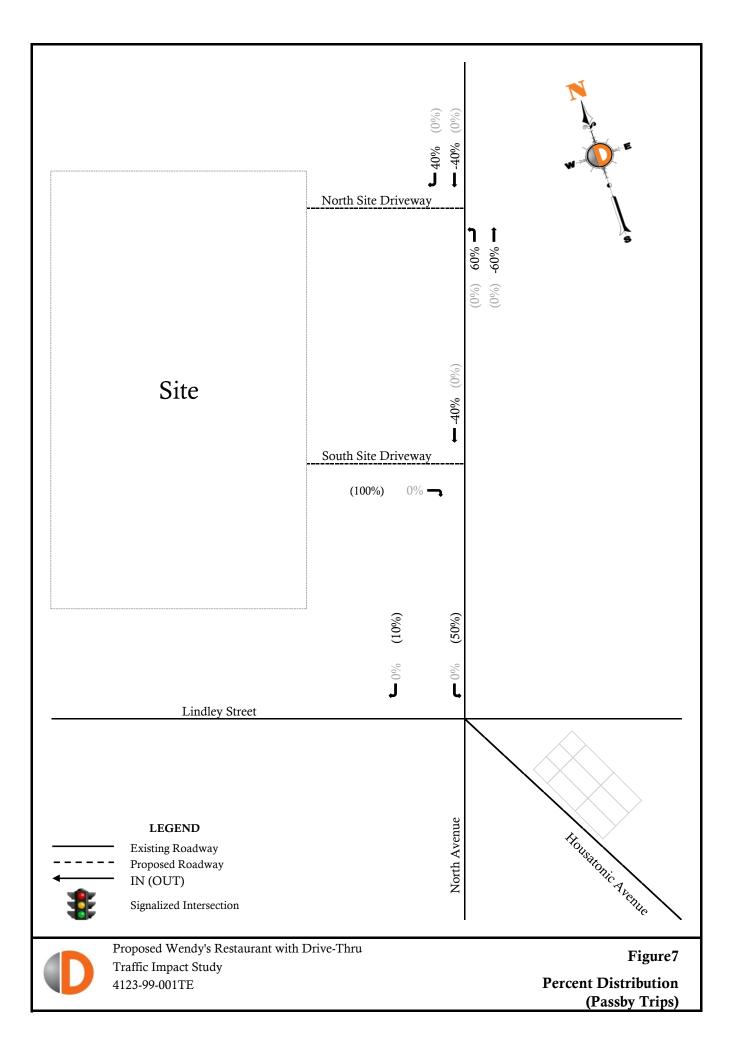


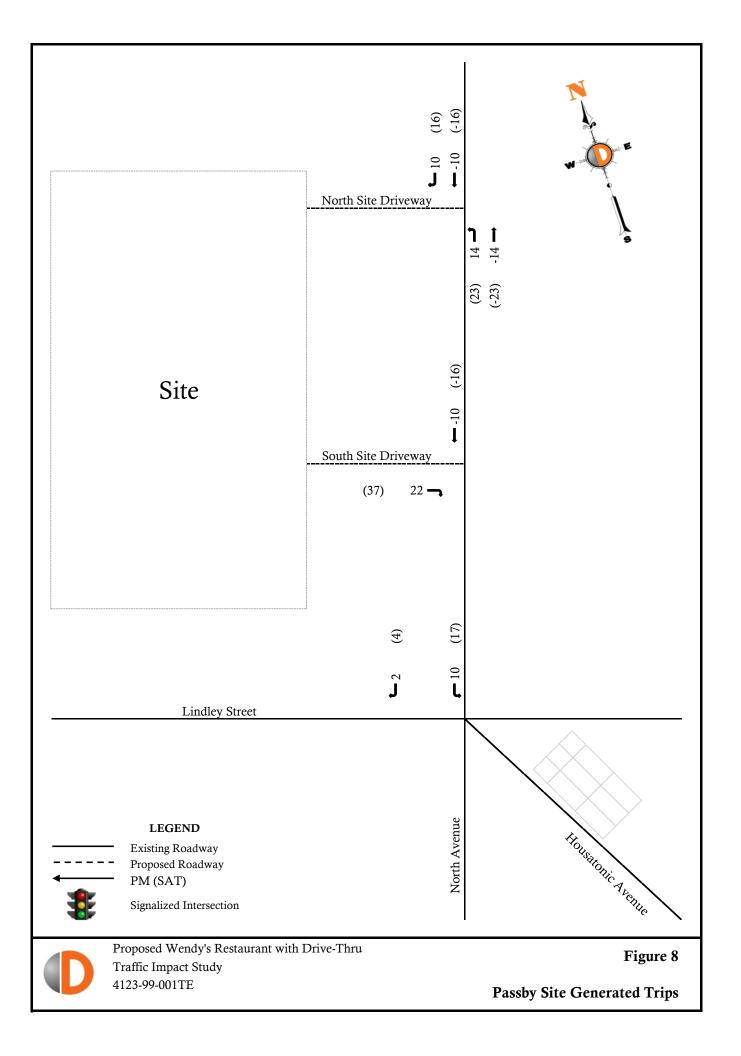


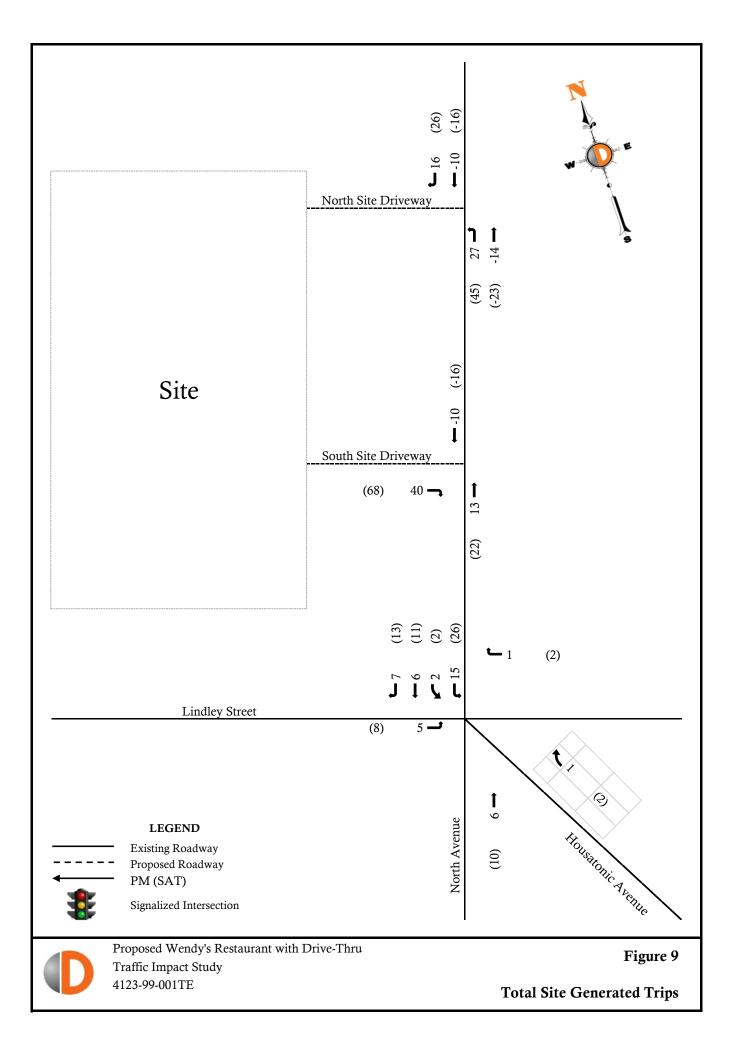


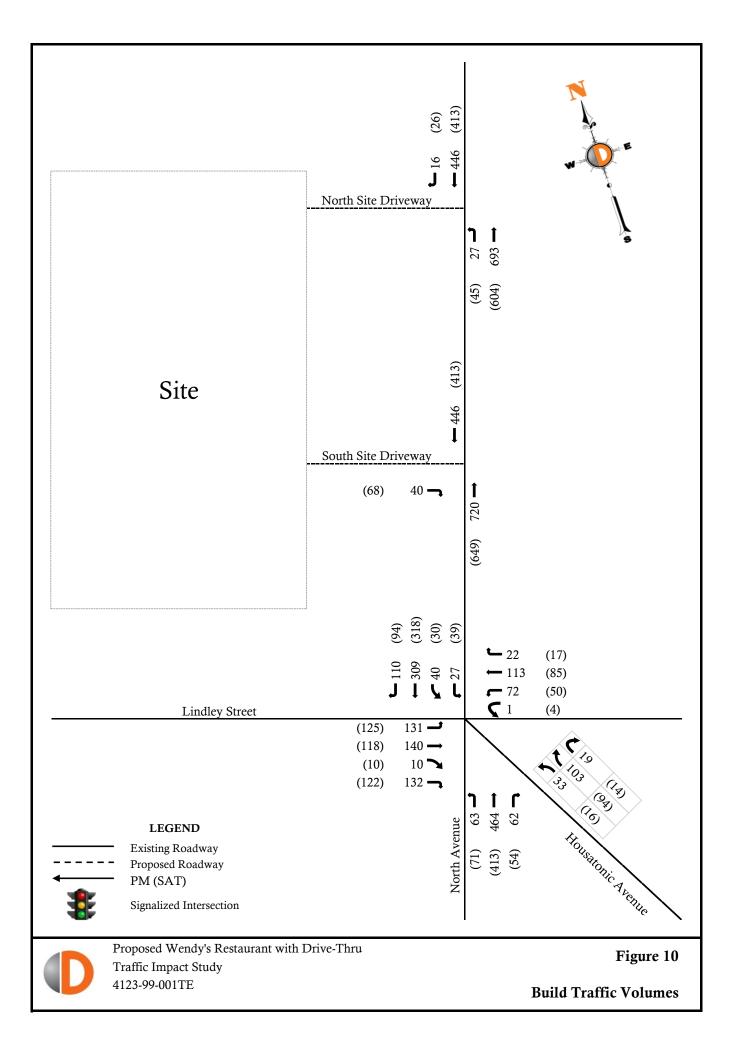












Appendix B Traffic Counts

# Dynamic Traffic, LLC 1904 Main Street, Lake Como, NJ 07719

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite 110, Chester, NJ 07930 732-681-0760

E/W: Lindley St N/S: North Ave/Housatonic Ave Town/County: Bridgeport/Fairfield Job #: 4123-99-001TE File Name : North Ave & LindleySt-Housatonic Ave - PM Site Code : 00000000 Start Date : 12/14/2021 Page No : 1

									Gro	oups	Printe	d- Car	s - Tr	ucks	(SU)	- Tru	cks (1	TT)									_
		L	indle East	y Stre boun						y Stre boun			Ν	lorth		Àve	oute enue ibour	1)/Ho id	usato	nic	1			iue (F nbour	loute Id	1)	
Start Time	Left	Thru	Right to Hous. Ave	Right to Noth Ave	Peds	App. Total	Left to Hous. Ave	Left to North Ave	Thru	Right	Peds	App. Total	North Ave Left	North Ave Thru	North Ave Right	Hous. Ave Left	Hous. Ave Thru	Hous. Ave Right	Peds	App. Total	Left	Thru to Hous. Ave	Thru to North Ave	Right	Peds	App. Total	Int. Total
04:30 PM	36	43	0	41	0	120	0	22	26	6	0	54	22	117	15	11	18	2	0	185	3	10	70	31	2	116	475
04:45 PM	28	29	2	24	0	83	1	20	28	4	0	53	17	110	18	8	26	8	0	187	1	9	83	22	1	116	439
Total	64	72	2	65	0	203	1	42	54	10	0	107	39	227	33	19	44	10	0	372	4	19	153	53	3	232	914
05:00 PM	30	35	4	36	0	105	0	16	25	6	0	47	17	97	12	9	34	5	0	174	1	9	72	31	2	115	441
05:15 PM	29	31	4	29	0	93	0	13	33	4	0	50	6	125	16	5	22	4	0	178	6	9	72	16	3	106	427
05:30 PM	42	36	11	32	0	121	1	12	39	3	0	55	16	104	14	5	27	4	0	170	3	1	80	19	1	104	450
05:45 PM	34	30	5	35	0	104	0	21	22	2	0	45	13	98	11	0	21	6	0	149	3	4	74	19	2	102	400
Total	135	132	24	132	0	423	1	62	119	15	0	197	52	424	53	19	104	19	0	671	13	23	298	85	8	427	1718
06:00 PM	31	32	1	30	0	94	2	15	21	4	0	42	12	96	18	10	28	2	0	166	1	6	83	26	0	116	418
06:15 PM	32	36	4	38	0	110	2	12	24	2	0	40	20	80	12	2	22	4	0	140	5	2	68	19	0	94	384
Grand Total	262	272	31	265	0	830	6	131	218	31	0	386	123	827	116	50	198	35	0	1349	23	50	602	183	11	869	3434
Apprch %	31.6	32.8	3.7	31.9	0		1.6	33.9	56.5	8	0		9.1	61.3	8.6	3.7	14.7	2.6	0		2.6	5.8	69.3	21.1	1.3		
Total %	7.6	7.9	0.9	7.7	0	24.2	0.2	3.8	6.3	0.9	0	11.2	3.6	24.1	3.4	1.5	5.8	1	0	39.3	0.7	1.5	17.5	5.3	0.3	25.3	
Cars	259	264	28	263	0	814	6	131	217	31	0	385	121	822	114	49	197	34	0	1337	23	48	599	183	11	864	3400
% Cars	98.9	97.1	90.3	99.2	0	98.1	100	100	99.5	100	0	99.7	98.4	99.4	98.3	98	99.5	97.1	0	99.1	100	96	99.5	100	100	99.4	99
Trucks (SU)	3	6	3	1	0	13	0	0	1	0	0	1	0	4	2	1	1	1	0	9	0	2	2	0	0	4	27
% Trucks (SU)	1.1	2.2	9.7	0.4	0	1.6	0	0	0.5	0	0	0.3	0	0.5	1.7	2	0.5	2.9	0	0.7	0	4	0.3	0	0	0.5	0.8
Trucks (TT)	0	2	0	1	0	3	0	0	0	0	0	0	2	1	0	0	0	0	0	3	0	0	1	0	0	1	7
% Trucks (TT)	0	0.7	0	0.4	0	0.4	0	0	0	0	0	0	1.6	0.1	0	0	0	0	0	0.2	0	0	0.2	0	0	0.1	0.2

# Dynamic Traffic, LLC 1904 Main Street, Lake Como, NJ 07719

1904 Main Street, Lake Como, NJ 07719 245 Main Street - Suite #110, Chester, NJ 07930 732-681-0760

E/W: Lindley St N/S: North Ave/Housatonic Ave Town/County: Bridgeport/Fairfield Job #: 4123-99-001TE File Name : North Ave & LindleySt-Housatonic Ave - SAT Site Code : 00000000 Start Date : 12/11/2021 Page No : 1

									Gro	oups	Printe	ed- Car	s - Tr	ucks	(SU)	- Truc	cks (T	T)									_
		L	indle East	y Stre boun				L	indle West	/			Ν	lorth	Äveni	``	enue	,	usato	nic	ľ			ue (R nboun		1)	
Start Time	Left	Thru	Right to Hous. Ave	Right to Noth Ave	Peds	App. Total	Left to Hous. Ave	Left to North Ave	Thru	Right	Peds	App. Total	North Ave Left	North Ave Thru	North Ave Right	Hous. Ave Left	Hous. Ave Thru	Hous. Ave Right	Peds	App. Total	Left	Thru to Hous. Ave	Thru to North Ave	Right	Peds	App. Total	Int. Total
11:00 AM	35	24	6	27	0	92	9	4	19	7	0	39	11	78	8	3	19	2	0	121	2	6	55	9	2	74	326
11:15 AM	26	33	0	29	0	88	0	10	16	5	0	31	9	78	13	6	18	2	0	126	4	7	61	21	1	94	339
11:30 AM	41	38	6	20	0	105	0	17	22	5	0	44	12	78	17	5	15	6	0	133	1	6	69	33	2	111	393
11:45 AM	38	25	4	26	0	93	2	8	26	4	0	40	12	88	17	7	26	4	0	154	1	4	72	22	3	102	389
Total	140	120	16	102	0	378	11	39	83	21	0	154	44	322	55	21	78	14	0	534	8	23	257	85	8	381	1447
12:00 PM	34	30	3	31	0	98	2	10	19	4	0	35	10	74	11	4	16	3	0	118	2	10	62	41	1	116	367
12:15 PM	32	18	6	26	0	82	0	18	32	5	0	55	11	93	10	2	17	7	0	140	4	6	73	19	2	104	381
12:30 PM	39	24	2	26	0	91	4	12	24	5	0	45	18	95	11	4	18	5	0	151	9	2	73	28	0	112	399
12:45 PM	23	25	8	31	0	87	1	16	16	1	0	34	16	87	16	6	20	5	0	150	7	5	68	21	0	101	372
Total	128	97	19	114	0	358	7	56	91	15	0	169	55	349	48	16	71	20	0	559	22	23	276	109	3	433	1519
01:00 PM	25	31	2	24	0	82	2	11	25	5	0	43	17	90	12	4	24	2	0	149	4	7	68	19	1	99	373
01:15 PM	22	28	4	33	0	87	1	15	18	2	0	36	8	100	10	6	22	4	0	150	1	9	70	24	1	105	378
01:30 PM	31	29	1	30	0	91	0	12	27	4	0	43	28	110	19	1	20	4	0	182	5	8	69	21	1	104	420
01:45 PM	35	29	3	34	0	101	1	11	14	3	0	29	17	93	12	5	24	4	0	155	2	3	92	13	2	112	397
Total	113	117	10	121	0	361	4	49	84	14	0	151	70	393	53	16	90	14	0	636	12	27	299	77	5	420	1568
Grand Total	381	334	45	337	0	1097	22	144	258	50	0	474	169	1064	156	53	239	48	0	1729	42	73	832	271	16	1234	4534
Apprch %	34.7	30.4	4.1	30.7	0		4.6	30.4	54.4	10.5	0		9.8	61.5	9	3.1	13.8	2.8	0		3.4	5.9	67.4	22	1.3		
Total %	8.4	7.4	1	7.4	0	24.2	0.5	3.2	5.7	1.1	0	10.5	3.7	23.5	3.4	1.2	5.3	1.1	0	38.1	0.9	1.6	18.4	6	0.4	27.2	
Cars	373	331	44	332	0	1080	22	144	251	49	0	466	166	1056	155	53	237	48	0	1715	42	72	823	269	16	1222	4483
% Cars	97.9	99.1	97.8	98.5	0	98.5	100	100	97.3	98	0	98.3	98.2	99.2	99.4	100	99.2	100	0	99.2	100	98.6	98.9	99.3	100	99	98.9
Trucks (SU)	6	2	1	5	0	14	0	0	6	0	0	6	2	7	1	0	1	0	0	11	0	0	9	2	0	11	42
% Trucks (SU)	1.6	0.6	2.2	1.5	0	1.3	0	0	2.3	0	0	1.3	1.2	0.7	0.6	0	0.4	0	0	0.6	0	0	1.1	0.7	0	0.9	0.9
Trucks (TT)	2	1	0	0	0	3	0	0	1	1	0	2	1	1	0	0	1	0	0	3	0	1	0	0	0	1	9
% Trucks (TT)	0.5	0.3	0	0	0	0.3	0	0	0.4	2	0	0.4	0.6	0.1	0	0	0.4	0	0	0.2	0	1.4	0	0	0	0.1	0.2

Appendix C Capacity Analysis

	۶	+	۲	*	5	4	t	*	•	1	*	<b>&gt;</b>
Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Lane Configurations	1	<b>†</b>	1			N.	el el		٦	1	1	
Traffic Volume (vph)	123	138	10	130	1	71	112	20	62	449	61	11
Future Volume (vph)	123	138	10	130	1	71	112	20	62	449	61	11
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	11	11	11	14	14	10	11	11	12
Grade (%)		-2%					1%			-1%		
Storage Length (ft)	0		0			0		0	0		65	
Storage Lanes	1		1			1		0	1		1	
Taper Length (ft)	25					25			25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850				0.977				0.850	
Flt Protected	0.950					0.950			0.950			
Satd. Flow (prot)	1685	1784	1540	0	0	1736	1970	0	1693	1828	1538	0
Flt Permitted	0.359					0.521			0.950			
Satd. Flow (perm)	637	1784	1540	0	0	952	1970	0	1693	1828	1538	0
Right Turn on Red				No				No				
Satd. Flow (RTOR)												
Link Speed (mph)		25					25			25		
Link Distance (ft)		239					270			823		
Travel Time (s)		6.5					7.4			22.4		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	4%	20%	1%	0%	0%	0%	0%	0%	1%	2%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	131	147	149	0	0	77	140	0	66	478	65	0
Turn Type	pm+pt	NA	Perm		pm+pt	pm+pt	NA		Prot	NA	Perm	pm+pt
Protected Phases	7	4			3	3	8		5	2		1
Permitted Phases	4		4		8	8					2	6
Detector Phase	7	4	4		3	3	8		5	2	2	1
Switch Phase												
Minimum Initial (s)	5.0	7.0	7.0		5.0	5.0	7.0		5.0	26.7	26.7	5.0
Minimum Split (s)	8.0	11.2	11.2		8.0	8.0	11.2		11.7	32.9	32.9	11.5
Total Split (s)	11.0	16.2	16.2		11.0	11.0	16.2		18.7	46.9	46.9	18.5
Total Split (%)	9.7%	14.3%	14.3%		9.7%	9.7%	14.3%		16.5%	41.3%	41.3%	16.3%
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.7	3.7	3.7
All-Red Time (s)	0.0	1.2	1.2		0.0	0.0	1.2		3.7	2.5	2.5	2.8
Lost Time Adjust (s)	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	3.0	4.2	4.2			3.0	4.2		6.7	6.2	6.2	
Lead/Lag	Lead	Lag	Lag		Lead	Lead	Lag		Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	Yes	Yes
Recall Mode	None	None	None		None	None	None		None	C-Max	C-Max	None
Act Effct Green (s)	22.0	14.0	14.0			20.2	11.4		9.4	46.0	46.0	
Actuated g/C Ratio	0.19	0.12	0.12			0.18	0.10		0.08	0.40	0.40	
v/c Ratio	0.65	0.67	0.78			0.35	0.71		0.47	0.65	0.10	
Control Delay	55.4	64.6	77.6			41.6	69.5		60.1	34.5	24.9	
Queue Delay	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Delay	55.4	64.6	77.6			41.6	69.5		60.1	34.5	24.9	
LOS	E	E	E			D	E		E	С	С	
Approach Delay		66.3					59.6			36.3		

CGH 12/20/2021

	L,	ţ	~	•	*	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Lane Configurations		100 •	SDR	<u> </u>		1111112
Traffic Volume (vph)	37	297	100	33	100	19
Future Volume (vph)	37	297	100	33	100	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1900	1900	1900	1900	1900	1900
Grade (%)	11	5%	10	-1%	12	12
	265	5%	0	130	0	
Storage Length (ft)					1	
Storage Lanes	1		0	1	1	
Taper Length (ft)	60	1.00	1.00	40	1.00	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.050	0.962		0.050	0.850	
Flt Protected	0.950		-	0.950		
Satd. Flow (prot)	1663	1946	0	1754	1597	0
Flt Permitted	0.950			0.950		
Satd. Flow (perm)	1663	1946	0	1754	1597	0
Right Turn on Red			No			No
Satd. Flow (RTOR)						
Link Speed (mph)		25		25		
Link Distance (ft)		257		629		
Travel Time (s)		7.0		17.2		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	3%	1%	0%	0%	1%	5%
Shared Lane Traffic (%)	0,0		• / •	0,0	.,,,	0,0
Lane Group Flow (vph)	51	422	0	35	126	0
Turn Type	Prot	NA	U	Prot	Prot	0
Protected Phases	1	6		9	9	
Permitted Phases	1	U		5	- J	
Detector Phase	1	6		9	9	
Switch Phase		U		9	9	
	ΕO	06.0		7.0	7.0	
Minimum Initial (s)	5.0	26.2		7.0	7.0	
Minimum Split (s)	11.5	32.7		14.0	14.0	
Total Split (s)	18.5	46.7		21.0	21.0	
Total Split (%)	16.3%	41.1%		18.5%	18.5%	
Yellow Time (s)	3.7	3.7		3.2	3.2	
All-Red Time (s)	2.8	2.8		3.8	3.8	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	6.5		7.0	7.0	
Lead/Lag	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes				
Recall Mode	None	C-Max		None	None	
Act Effct Green (s)	10.7	46.9		12.6	12.6	
Actuated g/C Ratio	0.09	0.41		0.11	0.11	
v/c Ratio	0.33	0.53		0.18	0.72	
Control Delay	53.1	30.2		47.3	70.8	
Queue Delay	0.0	0.0		0.0	0.0	
Total Delay	53.1	30.2		47.3	70.8	
LOS	D	50.2 C		۲.5 D	70.0 E	
Approach Delay	U	32.7		65.7	Ľ	
Approach LOS		С		E		

	٦	-	-*	$\mathbf{r}$	۲	4	+	*	1	Ť	۲	1
Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Queue Length 50th (ft)	82	106	110			47	100		47	301	32	
Queue Length 95th (ft)	#158	#211	#236			90	#184		91	427	64	
Internal Link Dist (ft)		159					190			743		
Turn Bay Length (ft)											65	
Base Capacity (vph)	201	220	190			227	208		178	739	622	
Starvation Cap Reductn	0	0	0			0	0		0	0	0	
Spillback Cap Reductn	0	0	0			0	0		0	0	0	
Storage Cap Reductn	0	0	0			0	0		0	0	0	
Reduced v/c Ratio	0.65	0.67	0.78			0.34	0.67		0.37	0.65	0.10	
Intersection Summary												
Area Type:	Other											
Cycle Length: 113.6												
Actuated Cycle Length: 113	3.6											
Offset: 0 (0%), Referenced	to phase 2:I	<b>NBT</b> and	6:SBTL, S	Start of Y	ellow, Mas	ster Inters	section					
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.78												
Intersection Signal Delay: 4					ntersectior							
Intersection Capacity Utilization	ation 72.3%			10	CU Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume			eue may	be longe	r.							
Queue shown is maxim	um after two	cycles.										
Splits and Phases: 10: N	lorth Avenue		tonic Ave	anua & Li	ndlav Stra	ot						
					naley olle	<u>.</u>						

Ø2 (R)		• Ø1	<b>▲</b> •	<b>19</b>	<b>€</b> ø3	4 <sub>04</sub>
46.9 s		18.5 s	21 s		11 s	16.2 s
<b>Ø</b> 5	Ø6 (R)				▶ Ø7	₩ø8
18.7 s	46.7s				11 s	16.2 s

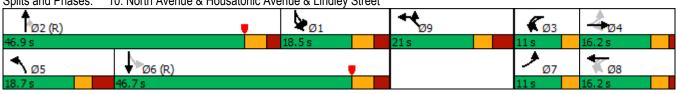
	L.	Ļ	-	•	•	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Queue Length 50th (ft)	35	244		23	89	
Queue Length 95th (ft)	74	359		55	#165	
Internal Link Dist (ft)		177		549		
Turn Bay Length (ft)	265			130		
Base Capacity (vph)	175	802		216	196	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.29	0.53		0.16	0.64	
Intersection Summary						

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Lane Configurations	۲	<b>†</b>	R.			ă.	¢.		5	<b>†</b>	1	
Traffic Volume (vph)	113	117	10	121	4	49	84	14	70	393	53	12
Future Volume (vph)	113	117	10	121	4	49	84	14	70	393	53	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	11	11	11	14	14	10	11	11	12
Grade (%)		-2%					1%			-1%		
Storage Length (ft)	0		0			0		0	0		65	
Storage Lanes	1		1			1		0	1		1	
Taper Length (ft)	25					25			25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850				0.979				0.850	
Flt Protected	0.950					0.950			0.950			
Satd. Flow (prot)	1702	1819	1577	0	0	1736	1957	0	1676	1828	1569	0
Flt Permitted	0.480					0.581			0.950			
Satd. Flow (perm)	860	1819	1577	0	0	1062	1957	0	1676	1828	1569	0
Right Turn on Red				No				No				
Satd. Flow (RTOR)												
Link Speed (mph)		25					25			25		
Link Distance (ft)		239					270			823		
Travel Time (s)		6.5					7.4			22.4		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	2%	0%	0%	0%	0%	1%	0%	1%	1%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	122	126	141	0	0	57	105	0	75	423	57	0
Turn Type	pm+pt	NA	Perm		pm+pt	pm+pt	NA		Prot	NA	Perm	pm+pt
Protected Phases	7	4			3	3	8		5	2		1
Permitted Phases	4		4		8	8					2	6
Detector Phase	7	4	4		3	3	8		5	2	2	1
Switch Phase												
Minimum Initial (s)	5.0	7.0	7.0		5.0	5.0	7.0		5.0	26.7	26.7	5.0
Minimum Split (s)	8.0	11.2	11.2		8.0	8.0	11.2		11.7	32.9	32.9	11.5
Total Split (s)	11.0	16.2	16.2		11.0	11.0	16.2		18.7	46.9	46.9	18.5
Total Split (%)	9.7%	14.3%	14.3%		9.7%	9.7%	14.3%		16.5%	41.3%	41.3%	16.3%
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.7	3.7	3.7
All-Red Time (s)	0.0	1.2	1.2		0.0	0.0	1.2		3.7	2.5	2.5	2.8
Lost Time Adjust (s)	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	3.0	4.2	4.2			3.0	4.2		6.7	6.2	6.2	
Lead/Lag	Lead	Lag	Lag		Lead	Lead	Lag		Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	Yes	Yes
Recall Mode	None	None	None		None	None	None		None	C-Min	C-Min	None
Act Effct Green (s)	20.6	13.0	13.0			18.7	10.6		9.1	53.3	53.3	
Actuated g/C Ratio	0.18	0.11	0.11			0.16	0.09		0.08	0.47	0.47	
v/c Ratio	0.57	0.61	0.78			0.26	0.58		0.56	0.49	0.08	
Control Delay	50.6	61.7	78.7			40.2	62.0		65.5	26.5	21.8	
Queue Delay	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Delay	50.6	61.7	78.7			40.2	62.0		65.5	26.5	21.8	
LOS	D	E	E			D	E		E	C	C	
	-		-			-			-		2	
Approach Delay		64.4					54.4			31.3		

CGH 12/20/2021

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Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Lane Configurations	Ä	4		۲	Ĩ.	
Traffic Volume (vph)	27	299	77	16	90	14
Future Volume (vph)	27	299	77	16	90	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	1500	15	11	12	12
Grade (%)	11	5%	10	-1%	12	12
Storage Length (ft)	265	070	0	130	0	
Storage Lanes	1		0	100	1	
Taper Length (ft)	60		0	40	1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.969	1.00	1.00	0.850	1.00
	0.050	0.909		0.950	0.000	
Fit Protected	0.950	4055	0		4000	0
Satd. Flow (prot)	1701	1955	0	1754	1623	0
Flt Permitted	0.950	105-	-	0.950	1000	-
Satd. Flow (perm)	1701	1955	0	1754	1623	0
Right Turn on Red			No			No
Satd. Flow (RTOR)						
Link Speed (mph)		25		25		
Link Distance (ft)		257		629		
Travel Time (s)		7.0		17.2		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	1%	1%	0%	0%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	42	405	0	17	112	0
Turn Type	Prot	NA		Prot	Prot	
Protected Phases	1	6		9	9	
Permitted Phases		·		v	Ť	
Detector Phase	1	6		9	9	
Switch Phase	1	0		5	5	
Minimum Initial (s)	5.0	26.2		7.0	7.0	
Minimum Split (s)	11.5	32.7		14.0	14.0	
	18.5	32.7 46.7		21.0	21.0	
Total Split (s)						
Total Split (%)	16.3%	41.1%		18.5%	18.5%	
Yellow Time (s)	3.7	3.7		3.2	3.2	
All-Red Time (s)	2.8	2.8		3.8	3.8	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	6.5		7.0	7.0	
Lead/Lag	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes				
Recall Mode	None	C-Min		None	None	
Act Effct Green (s)	8.5	49.7		11.4	11.4	
Actuated g/C Ratio	0.07	0.44		0.10	0.10	
v/c Ratio	0.33	0.47		0.10	0.69	
Control Delay	56.1	27.8		46.1	70.0	
Queue Delay	0.0	0.0		0.0	0.0	
Total Delay	56.1	27.8		46.1	70.0	
LOS	E	27.0 C		-0.1 D	70.0 E	
Approach Delay	L	30.5		66.8	L	
Approach LOS		С		E		

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Queue Length 50th (ft)	77	90	103			34	74		54	244	26	
Queue Length 95th (ft)	133	#169	#217			71	132		101	361	56	
Internal Link Dist (ft)		159					190			743		
Turn Bay Length (ft)											65	
Base Capacity (vph)	215	208	180			231	206		177	857	736	
Starvation Cap Reductn	0	0	0			0	0		0	0	0	
Spillback Cap Reductn	0	0	0			0	0		0	0	0	
Storage Cap Reductn	0	0	0			0	0		0	0	0	
Reduced v/c Ratio	0.57	0.61	0.78			0.25	0.51		0.42	0.49	0.08	
Intersection Summary												
Area Type:	Other											
Cycle Length: 113.6												
Actuated Cycle Length: 113	8.6											
Offset: 0 (0%), Referenced	to phase 2:I	NBT and	6:SBTL, S	Start of Y	ellow, Mas	ster Inters	ection					
Natural Cycle: 80												
Control Type: Actuated-Coc	ordinated											
Maximum v/c Ratio: 0.78												
Intersection Signal Delay: 4					ntersection							
Intersection Capacity Utiliza	ation 65.7%			10	CU Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume			eue may	be longe	r.							
Queue shown is maximu	um after two	cycles.										
Splits and Phases: 10: N	orth Avenue	& Housa	Itonic Ave	enue & Li	ndley Stre	et						



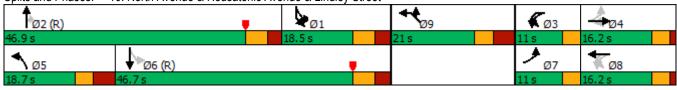
	L.	ŧ.	∢	•	*	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Queue Length 50th (ft)	29	222		11	80	
Queue Length 95th (ft)	65	342		34	139	
Internal Link Dist (ft)		177		549		
Turn Bay Length (ft)	265			130		
Base Capacity (vph)	179	854		216	200	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.23	0.47		0.08	0.56	
Intersection Summary						

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Lane Configurations	<u>م</u>	•	N.			ħ.	el el		5	•	1	
Traffic Volume (vph)	126	140	10	132	1	72	113	21	63	458	62	12
Future Volume (vph)	126	140	10	132	1	72	113	21	63	458	62	12
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	11	11	11	14	14	10	11	11	12
Grade (%)		-2%					1%			-1%		
Storage Length (ft)	0		0			0		0	0		65	
Storage Lanes	1		1			1		0	1		1	
Taper Length (ft)	25					25			25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850				0.977				0.850	
Flt Protected	0.950					0.950			0.950			
Satd. Flow (prot)	1685	1784	1540	0	0	1736	1970	0	1693	1828	1538	0
Flt Permitted	0.352					0.513			0.950			
Satd. Flow (perm)	624	1784	1540	0	0	937	1970	0	1693	1828	1538	0
Right Turn on Red				No				No				
Satd. Flow (RTOR)												
Link Speed (mph)		25					25			25		
Link Distance (ft)		239					270			823		
Travel Time (s)		6.5					7.4			22.4		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	4%	20%	1%	0%	0%	0%	0%	0%	1%	2%	0%
Shared Lane Traffic (%)	.,.	.,.	_0,0	.,.	• , •	• ,•	• / •	• , •	• / •	. , •	_/*	• / •
Lane Group Flow (vph)	134	149	151	0	0	78	142	0	67	487	66	0
Turn Type	pm+pt	NA	Perm	-	pm+pt	pm+pt	NA	-	Prot	NA	Perm	pm+pt
Protected Phases	7	4			3	3	8		5	2		1
Permitted Phases	4		4		8	8	-		-		2	6
Detector Phase	7	4	4		3	3	8		5	2	2	1
Switch Phase												
Minimum Initial (s)	5.0	7.0	7.0		5.0	5.0	7.0		5.0	26.7	26.7	5.0
Minimum Split (s)	8.0	11.2	11.2		8.0	8.0	11.2		11.7	32.9	32.9	11.5
Total Split (s)	11.0	16.2	16.2		11.0	11.0	16.2		18.7	46.9	46.9	18.5
Total Split (%)	9.7%	14.3%	14.3%		9.7%	9.7%	14.3%		16.5%	41.3%	41.3%	16.3%
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.7	3.7	3.7
All-Red Time (s)	0.0	1.2	1.2		0.0	0.0	1.2		3.7	2.5	2.5	2.8
Lost Time Adjust (s)	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	3.0	4.2	4.2			3.0	4.2		6.7	6.2	6.2	
Lead/Lag	Lead	Lag	Lag		Lead	Lead	Lag		Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	Yes	Yes
Recall Mode	None	None	None		None	None	None		None	C-Max	C-Max	None
Act Effct Green (s)	22.0	14.0	14.0			20.3	11.4		9.5	45.9	45.9	
Actuated g/C Ratio	0.19	0.12	0.12			0.18	0.10		0.08	0.40	0.40	
v/c Ratio	0.67	0.68	0.79			0.35	0.72		0.48	0.66	0.11	
Control Delay	56.8	65.2	78.9			41.7	70.2		60.2	35.0	24.9	
Queue Delay	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Delay	56.8	65.2	78.9			41.7	70.2		60.2	35.0	24.9	
LOS	E	E	E			D	E		E	D	C 1.0	
Approach Delay	_	67.4					60.1		_	36.7		
Approach LOS		E					E			D		
		-					-			2		

CGH 12/20/2021 Synchro 11 Report Lanes, Volumes, Timings

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Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Lane Configurations	2002	1	0011	5	1	
Traffic Volume (vph)	38	303	103	33	102	19
Future Volume (vph)	38	303	103	33	102	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	15	15	11	12	12
Grade (%)		5%		-1%		
Storage Length (ft)	265		0	130	0	
Storage Lanes	1		0	1	1	
Taper Length (ft)	60			40		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.962			0.850	
Flt Protected	0.950			0.950		
Satd. Flow (prot)	1664	1946	0	1754	1597	0
Flt Permitted	0.950		-	0.950		-
Satd. Flow (perm)	1664	1946	0	1754	1597	0
Right Turn on Red	1007	10-10	No	TUT	1007	No
Satd. Flow (RTOR)			NU			NU
Link Speed (mph)		25		25		
Link Distance (ft)		257		629		
Travel Time (s)		7.0	0.01	17.2	0.04	
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	3%	1%	0%	0%	1%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	53	432	0	35	129	0
Turn Type	Prot	NA		Prot	Prot	
Protected Phases	1	6		9	9	
Permitted Phases						
Detector Phase	1	6		9	9	
Switch Phase	•	•		•	•	
Minimum Initial (s)	5.0	26.2		7.0	7.0	
Minimum Split (s)	11.5	32.7		14.0	14.0	
Total Split (s)	18.5	46.7		21.0	21.0	
,	16.3%			18.5%	18.5%	
Total Split (%)		41.1%				
Yellow Time (s)	3.7	3.7		3.2	3.2	
All-Red Time (s)	2.8	2.8		3.8	3.8	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	6.5		7.0	7.0	
Lead/Lag	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes				
Recall Mode	None	C-Max		None	None	
Act Effct Green (s)	10.7	46.7		12.7	12.7	
Actuated g/C Ratio	0.09	0.41		0.11	0.11	
v/c Ratio	0.34	0.54		0.18	0.73	
Control Delay	53.5	30.6		47.3	71.9	
Queue Delay	0.0	0.0		0.0	0.0	
	53.5	30.6		47.3	71.9	
Total Delay						
LOS Assessed Delay	D	C		D	E	
Approach Delay		33.1		66.6		
Approach LOS		С		E		

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Queue Length 50th (ft)	84	108	111			47	102		47	308	32	
Queue Length 95th (ft)	#166	#216	#240			91	#187		93	438	64	
Internal Link Dist (ft)		159					190			743		
Turn Bay Length (ft)											65	
Base Capacity (vph)	200	220	190			226	208		178	737	621	
Starvation Cap Reductn	0	0	0			0	0		0	0	0	
Spillback Cap Reductn	0	0	0			0	0		0	0	0	
Storage Cap Reductn	0	0	0			0	0		0	0	0	
Reduced v/c Ratio	0.67	0.68	0.79			0.35	0.68		0.38	0.66	0.11	
Intersection Summary												
Area Type:	Other											
Cycle Length: 113.6												
Actuated Cycle Length: 11	3.6											
Offset: 0 (0%), Referenced	I to phase 2:	NBT and	6:SBTL, S	Start of Y	ellow, Mas	ster Inters	ection					
Natural Cycle: 80												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay:					ntersectior							
Intersection Capacity Utiliz	ation 73.2%			10	CU Level o	of Service	D					
Analysis Period (min) 15												
# 95th percentile volume			eue may	be longe	r.							
Queue shown is maxim	um after two	cycles.										
Califo and Dhasaay 10: N	larth Avar		tonia Au	0 1 :		at						
Splits and Phases: 10: N	North Avenue	a Housa	ITONIC AVE	enue & Li	nuley Stre	et						



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Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Queue Length 50th (ft)	36	252		23	91	
Queue Length 95th (ft)	77	370		55	#172	
Internal Link Dist (ft)		177		549		
Turn Bay Length (ft)	265			130		
Base Capacity (vph)	175	800		216	196	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.30	0.54		0.16	0.66	
Intersection Summary						

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Lane Configurations	<u>۲</u>	<b>†</b>	1			N.	ef 🕺		<u> </u>	•	1	
Traffic Volume (vph)	117	118	10	122	4	50	85	15	71	403	54	13
Future Volume (vph)	117	118	10	122	4	50	85	15	71	403	54	13
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	11	11	11	14	14	10	11	11	12
Grade (%)		-2%					1%			-1%		
Storage Length (ft)	0		0			0		0	0		65	
Storage Lanes	1		1			1		0	1		1	
Taper Length (ft)	25					25			25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850				0.978				0.850	
Flt Protected	0.950					0.950			0.950			
Satd. Flow (prot)	1702	1819	1577	0	0	1736	1956	0	1676	1828	1569	0
Flt Permitted	0.472					0.577			0.950			
Satd. Flow (perm)	845	1819	1577	0	0	1054	1956	0	1676	1828	1569	0
Right Turn on Red				No				No				
Satd. Flow (RTOR)												
Link Speed (mph)		25					25			25		
Link Distance (ft)		239					270			823		
Travel Time (s)		6.5					7.4			22.4		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	2%	0%	0%	0%	0%	1%	0%	1%	1%	0%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	126	127	142	0	0	58	107	0	76	433	58	0
Turn Type	pm+pt	NA	Perm		pm+pt	pm+pt	NA		Prot	NA	Perm	pm+pt
Protected Phases	7	4			3	3	8		5	2		1
Permitted Phases	4		4		8	8					2	6
Detector Phase	7	4	4		3	3	8		5	2	2	1
Switch Phase												
Minimum Initial (s)	5.0	7.0	7.0		5.0	5.0	7.0		5.0	26.7	26.7	5.0
Minimum Split (s)	8.0	11.2	11.2		8.0	8.0	11.2		11.7	32.9	32.9	11.5
Total Split (s)	11.0	16.2	16.2		11.0	11.0	16.2		18.7	46.9	46.9	18.5
Total Split (%)	9.7%	14.3%	14.3%		9.7%	9.7%	14.3%		16.5%	41.3%	41.3%	16.3%
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.7	3.7	3.7
All-Red Time (s)	0.0	1.2	1.2		0.0	0.0	1.2		3.7	2.5	2.5	2.8
Lost Time Adjust (s)	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	3.0	4.2	4.2			3.0	4.2		6.7	6.2	6.2	
Lead/Lag	Lead	Lag	Lag		Lead	Lead	Lag		Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	Yes	Yes
Recall Mode	None	None	None		None	None	None		None	C-Min	C-Min	None
Act Effct Green (s)	20.6	13.0	13.0			18.8	10.6		9.2	49.5	49.5	
Actuated g/C Ratio	0.18	0.11	0.11			0.17	0.09		0.08	0.44	0.44	
v/c Ratio	0.59	0.61	0.79			0.27	0.59		0.56	0.54	0.08	
Control Delay	51.6	61.8	79.0			40.3	62.6		65.6	29.4	23.1	
Queue Delay	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Delay	51.6	61.8	79.0			40.3	62.6		65.6	29.4	23.1	
LOS	D	Е	Е			D	E		Е	С	С	
Approach Delay		64.7					54.7			33.6		
Approach LOS		E					D			С		

CGH 12/20/2021 Synchro 11 Report Lanes, Volumes, Timings

	L,	Ŧ	~	*	*	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Lane Configurations	Ä	- <u></u>		5	1	
Traffic Volume (vph)	28	307	81	16	92	14
Future Volume (vph)	28	307	81	16	92	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	1300	1500	1500	1300	1300	1300
Grade (%)	11	5%	15	-1%	12	12
Storage Length (ft)	265	J /0	0	130	0	
Storage Lanes	205		0	130	1	
Taper Length (ft)	60		0	40	I	
	1.00	1.00	1.00	1.00	1.00	1.00
Lane Util. Factor	1.00		1.00	1.00		1.00
Frt Filt Desite start	0.050	0.969		0.050	0.850	
FIt Protected	0.950	4055	^	0.950	4000	^
Satd. Flow (prot)	1701	1955	0	1754	1623	0
Flt Permitted	0.950			0.950		
Satd. Flow (perm)	1701	1955	0	1754	1623	0
Right Turn on Red			No			No
Satd. Flow (RTOR)						
Link Speed (mph)		25		25		
Link Distance (ft)		257		629		
Travel Time (s)		7.0		17.2		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	1%	1%	0%	0%	0%
Shared Lane Traffic (%)				- / -		
Lane Group Flow (vph)	44	417	0	17	114	0
Turn Type	Prot	NA	Ū	Prot	Prot	Ū
Protected Phases	1	6		9	9	
Permitted Phases	1	U		5	5	
Detector Phase	1	6		9	9	
Switch Phase	I	0		9	9	
	۶O	00.0		7.0	7.0	
Minimum Initial (s)	5.0	26.2		7.0	7.0	
Minimum Split (s)	11.5	32.7		14.0	14.0	
Total Split (s)	18.5	46.7		21.0	21.0	
Total Split (%)	16.3%	41.1%		18.5%	18.5%	
Yellow Time (s)	3.7	3.7		3.2	3.2	
All-Red Time (s)	2.8	2.8		3.8	3.8	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	6.5		7.0	7.0	
Lead/Lag	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes				
Recall Mode	None	C-Min		None	None	
Act Effct Green (s)	9.6	49.5		11.5	11.5	
Actuated g/C Ratio	0.08	0.44		0.10	0.10	
v/c Ratio	0.31	0.49		0.10	0.70	
Control Delay	54.2	28.2		46.1	70.7	
Queue Delay	0.0	0.0		0.0	0.0	
	54.2	28.2		46.1	70.7	
Total Delay						
LOS Annuar de Delau	D	C		D	E	
Approach Delay		30.7		67.5		
Approach LOS		С		E		

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Queue Length 50th (ft)	79	91	104			35	76		54	250	27	
Queue Length 95th (ft)	137	#170	#219			72	134		102	372	57	
Internal Link Dist (ft)		159					190			743		
Turn Bay Length (ft)											65	
Base Capacity (vph)	214	209	180			231	206		177	797	684	
Starvation Cap Reductn	0	0	0			0	0		0	0	0	
Spillback Cap Reductn	0	0	0			0	0		0	0	0	
Storage Cap Reductn	0	0	0			0	0		0	0	0	
Reduced v/c Ratio	0.59	0.61	0.79			0.25	0.52		0.43	0.54	0.08	
Intersection Summary												
Area Type:	Other											
Cycle Length: 113.6												
Actuated Cycle Length: 113												
Offset: 0 (0%), Referenced	to phase 2:I	NBT and	6:SBTL, S	Start of Y	ellow, Mas	ster Inters	ection					
Natural Cycle: 80												
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay: 4					ntersectior							
Intersection Capacity Utiliza	ation 66.0%			IC	CU Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume			eue may	be longei	r.							
Queue shown is maximu	um after two	cycles.										
Splits and Phases: 10: N	orth Avenue	e & Housa	Itonic Ave	enue & Li	ndley Stre	et						

Ø2 (R)		• • • ø1	<b>+</b> € <sub>Ø9</sub>	<b>€</b> Ø3	404
46.9 s		18.5 s	21 s	11 s	16.2 s
<b>1</b> Ø5	Ø6 (R)				<b>*</b> Ø8
18.7 s	46.7 s			11 s	16.2 s

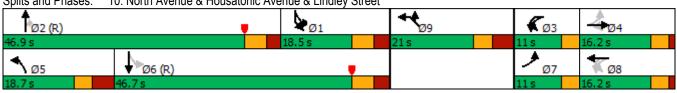
	L.	Ļ	∢	•	•	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Queue Length 50th (ft)	30	231		11	81	
Queue Length 95th (ft)	68	354		34	142	
Internal Link Dist (ft)		177		549		
Turn Bay Length (ft)	265			130		
Base Capacity (vph)	179	852		216	200	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.25	0.49		0.08	0.57	
Intersection Summary						

	≯	-	7	7	5	4	ł	*	1	1	1	1
Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Lane Configurations	ሻ	•	R.			Ă	4Î		ሻ	<b>†</b>	*	
Traffic Volume (vph)	131	140	10	132	1	72	113	22	63	464	62	27
Future Volume (vph)	131	140	10	132	1	72	113	22	63	464	62	27
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	11	11	11	14	14	10	11	11	12
Grade (%)		-2%					1%			-1%		
Storage Length (ft)	0		0			0		0	0		65	
Storage Lanes	1		1			1		0	1		1	
Taper Length (ft)	25					25			25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850				0.976				0.850	
Flt Protected	0.950					0.950			0.950			
Satd. Flow (prot)	1685	1784	1540	0	0	1736	1968	0	1693	1828	1538	0
Flt Permitted	0.351		1010	Ŭ	Ŭ	0.514	1000	Ŭ	0.950	1020	1000	Ű
Satd. Flow (perm)	622	1784	1540	0	0	939	1968	0	1693	1828	1538	0
Right Turn on Red	022		1010	No	Ű	000	1000	No	1000	1020	1000	, in the second s
Satd. Flow (RTOR)												
Link Speed (mph)		25					25			25		
Link Distance (ft)		239					270			823		
Travel Time (s)		6.5					7.4			22.4		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	1%	4%	20%	1%	0%	0%	0%	0%	0%	1%	2%	0%
Shared Lane Traffic (%)	170	170	2070	170	0,0	070	0,0	0,0	0,0	170	270	070
Lane Group Flow (vph)	139	149	151	0	0	78	143	0	67	494	66	0
Turn Type	pm+pt	NA	Perm	v	pm+pt	pm+pt	NA	v	Prot	NA	Perm	pm+pt
Protected Phases	ριπ-ρι 7	4	i onn		3	3	8		5	2	T OIIII	1 1
Permitted Phases	4	•	4		8	8	Ū		Ŭ	-	2	6
Detector Phase	7	4	4		3	3	8		5	2	2	1
Switch Phase	•	•	•		Ū	Ŭ	Ū		Ŭ	-	-	
Minimum Initial (s)	5.0	7.0	7.0		5.0	5.0	7.0		5.0	26.7	26.7	5.0
Minimum Split (s)	8.0	11.2	11.2		8.0	8.0	11.2		11.7	32.9	32.9	11.5
Total Split (s)	11.0	16.2	16.2		11.0	11.0	16.2		18.7	46.9	46.9	18.5
Total Split (%)	9.7%	14.3%	14.3%		9.7%	9.7%	14.3%		16.5%	41.3%	41.3%	16.3%
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.7	3.7	3.7
All-Red Time (s)	0.0	1.2	1.2		0.0	0.0	1.2		3.7	2.5	2.5	2.8
Lost Time Adjust (s)	0.0	0.0	0.0		0.0	0.0	0.0		0.0	0.0	0.0	2.0
Total Lost Time (s)	3.0	4.2	4.2			3.0	4.2		6.7	6.2	6.2	
Lead/Lag	Lead	Lag	Lag		Lead	Lead	Lag		Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	Yes	Yes
Recall Mode	None	None	None		None	None	None		None	C-Max	C-Max	None
Act Effct Green (s)	22.0	14.1	14.1		None	20.3	11.4		9.5	45.8	45.8	None
Actuated g/C Ratio	0.19	0.12	0.12			0.18	0.10		0.08	0.40	0.40	
v/c Ratio	0.69	0.12	0.72			0.10	0.73		0.00	0.40	0.40	
Control Delay	58.7	64.8	78.3			41.7	70.9		60.2	35.5	24.9	
Queue Delay	0.0	04.0	0.0			41.7 0.0	0.0		0.2	0.0	24.9	
Total Delay	58.7	64.8	78.3			41.7	70.9		60.2	35.5	24.9	
LOS	50.7 E	04.0 E	70.3 E				70.9 E		60.2 E	35.5 D	24.9 C	
	E	E 67.5	E			D	E 60.6		E	37.0	U	
Approach Delay												
Approach LOS		E					E			D		

CGH 01/05/2022 Synchro 11 Report Lanes, Volumes, Timings

	L,	Ļ	~	*	*	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Lane Configurations	3	1 <u>.</u>		5	1	
Traffic Volume (vph)	40	309	110	33	103	19
Future Volume (vph)	40	309	110	33	103	19
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	1500	15	11	12	12
Grade (%)	11	5%	15	-1%	12	12
Storage Length (ft)	265	070	0	130	0	
Storage Lanes	1		0	130	1	
Taper Length (ft)	60		0	40	1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
	1.00		1.00	1.00		1.00
Frt Fit Drotostad	0.050	0.961		0.050	0.850	
Fit Protected	0.950	4044	^	0.950	4507	^
Satd. Flow (prot)	1671	1944	0	1754	1597	0
Flt Permitted	0.950			0.950		_
Satd. Flow (perm)	1671	1944	0	1754	1597	0
Right Turn on Red			No			No
Satd. Flow (RTOR)						
Link Speed (mph)		25		25		
Link Distance (ft)		257		629		
Travel Time (s)		7.0		17.2		
Peak Hour Factor	0.94	0.94	0.94	0.94	0.94	0.94
Heavy Vehicles (%)	3%	1%	0%	0%	1%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	72	446	0	35	130	0
Turn Type	Prot	NA	-	Prot	Prot	
Protected Phases	1	6		9	9	
Permitted Phases		v		5	0	
Detector Phase	1	6		9	9	
Switch Phase	1	0		5		
Minimum Initial (s)	5.0	26.2		7.0	7.0	
	5.0 11.5	32.7		14.0	14.0	
Minimum Split (s)						
Total Split (s)	18.5	46.7		21.0	21.0	
Total Split (%)	16.3%	41.1%		18.5%	18.5%	
Yellow Time (s)	3.7	3.7		3.2	3.2	
All-Red Time (s)	2.8	2.8		3.8	3.8	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	6.5		7.0	7.0	
Lead/Lag	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes				
Recall Mode	None	C-Max		None	None	
Act Effct Green (s)	10.8	46.6		12.7	12.7	
Actuated g/C Ratio	0.10	0.41		0.11	0.11	
v/c Ratio	0.45	0.56		0.18	0.73	
Control Delay	57.4	31.1		47.2	72.1	
Queue Delay	0.0	0.0		0.0	0.0	
Total Delay	57.4	31.1		47.2	72.1	
LOS	E	C		D	E	
Approach Delay		34.8		66.8		
Approach LOS		С		E		

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Queue Length 50th (ft)	88	108	111			47	102		47	314	32	
Queue Length 95th (ft)	#176	#216	#240			91	#190		93	447	64	
Internal Link Dist (ft)		159					190			743		
Turn Bay Length (ft)											65	
Base Capacity (vph)	200	221	191			227	207		178	736	619	
Starvation Cap Reductn	0	0	0			0	0		0	0	0	
Spillback Cap Reductn	0	0	0			0	0		0	0	0	
Storage Cap Reductn	0	0	0			0	0		0	0	0	
Reduced v/c Ratio	0.69	0.67	0.79			0.34	0.69		0.38	0.67	0.11	
Intersection Summary												
Area Type: C	Other											
Cycle Length: 113.6												
Actuated Cycle Length: 113.6	6											
Offset: 0 (0%), Referenced to	phase 2:I	NBT and	6:SBTL, S	Start of Y	ellow, Mas	ster Inters	ection					
Natural Cycle: 80												
Control Type: Actuated-Coor	dinated											
Maximum v/c Ratio: 0.79												
Intersection Signal Delay: 48					ntersectior							
Intersection Capacity Utilizati	ion 73.9%			10	CU Level o	of Service	D					
Analysis Period (min) 15												
# 95th percentile volume ex	•		eue may	be longe	r.							
Queue shown is maximun	n after two	cycles.										
Splits and Phases: 10: Nor	rth Avenue		tonic Ave	anua & Ii	ndlev Stra	ot						



	L.	Ļ	∢	•	*	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Queue Length 50th (ft)	50	262		23	92	
Queue Length 95th (ft)	98	385		55	#173	
Internal Link Dist (ft)		177		549		
Turn Bay Length (ft)	265			130		
Base Capacity (vph)	176	797		216	196	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.41	0.56		0.16	0.66	
Intersection Summary						

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ŧ	et e	
Traffic Vol, veh/h	0	0	27	693	446	16
Future Vol, veh/h	0	0	27	693	446	16
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	,# 0	-	-	0	0	-
Grade, %	0	-	-	-1	2	-
Peak Hour Factor	25	25	98	98	98	98
Heavy Vehicles, %	0	0	2	1	1	2
Mvmt Flow	0	0	28	707	455	16

Major/Minor	Minor2	l	Major1	Maj	or2					
Conflicting Flow All	1226	463	471	0	-	0				
Stage 1	463	-	-	-	-	-				
Stage 2	763	-	-	-	-	-				
Critical Hdwy	6.4	6.2	4.12	-	-	-				
Critical Hdwy Stg 1	5.4	-	-	-	-	-				
Critical Hdwy Stg 2	5.4	-	-	-	-	-				
Follow-up Hdwy	3.5	3.3	2.218	-	-	-				
Pot Cap-1 Maneuver	199	603	1091	-	-	-				
Stage 1	638	-	-	-	-	-				
Stage 2	464	-	-	-	-	-				
Platoon blocked, %				-	-	-				
Mov Cap-1 Maneuver	191	603	1091	-	-	-				
Mov Cap-2 Maneuver	191	-	-	-	-	-				
Stage 1	611	-	-	-	-	-				
Stage 2	464	-	-	-	-	-				

Approach	EB	NB	SB
HCM Control Delay, s	0	0.3	0
HCM LOS	А		

Minor Lane/Major Mvmt	NBL	NBT EI	3Ln1	SBT	SBR
Capacity (veh/h)	1091	-	-	-	-
HCM Lane V/C Ratio	0.025	-	-	-	-
HCM Control Delay (s)	8.4	0	0	-	-
HCM Lane LOS	А	А	А	-	-
HCM 95th %tile Q(veh)	0.1	-	-	-	-

Intersection						
Int Delay, s/veh	0.4					
				NDT	ODT	000
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1		<b>↑</b>	- <b>†</b>	
Traffic Vol, veh/h	0	40	0	720	446	0
Future Vol, veh/h	0	40	0	720	446	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	-2	2	-
Peak Hour Factor	98	98	98	98	98	98
Heavy Vehicles, %	2	2	0	1	1	0
Mvmt Flow	0	41	0	735	455	0
	0	- 1	U	100	400	U

Major/Minor	Minor2	Ν	lajor1	Ма	ijor2	
Conflicting Flow All	-	455	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	-	-	-	-
Pot Cap-1 Maneuver	0	605	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	-	605	-	-	-	-
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	11.4		0		0	
HCM LOS	В					

HCM LOS	В		
Minor Lane/Major Mvmt	NBT EBLn1	SBT	
Capacity (veh/h)	- 605	-	
HCM Lane V/C Ratio	- 0.067	-	
HCM Control Delay (s)	- 11.4	-	
HCM Lane LOS	- B	-	

0.2

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HCM 95th %tile Q(veh)

#### 4123-99-001TE

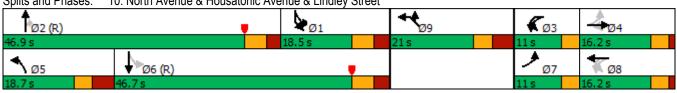
	٦	-	۲	*	۶.	4	+	•	1	1	1	1
Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Lane Configurations	<u>م</u>	•	N.			ĽV	el el		5	•	1	
Traffic Volume (vph)	125	118	10	122	4	50	85	17	71	413	54	39
Future Volume (vph)	125	118	10	122	4	50	85	17	71	413	54	39
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	10	11	11	11	11	11	14	14	10	11	11	12
Grade (%)		-2%					1%			-1%		
Storage Length (ft)	0		0			0		0	0		65	
Storage Lanes	1		1			1		0	1		1	
Taper Length (ft)	25					25			25			
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850				0.975				0.850	
Flt Protected	0.950					0.950			0.950			
Satd. Flow (prot)	1702	1819	1577	0	0	1736	1950	0	1676	1828	1569	0
Flt Permitted	0.461					0.583			0.950			
Satd. Flow (perm)	826	1819	1577	0	0	1065	1950	0	1676	1828	1569	0
Right Turn on Red				No				No				
Satd. Flow (RTOR)												
Link Speed (mph)		25					25			25		
Link Distance (ft)		239					270			823		
Travel Time (s)		6.5					7.4			22.4		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0%	2%	0%	0%	0%	0%	1%	0%	1%	1%	0%	0%
Shared Lane Traffic (%)	0,0	270	0,0	0,0	0,0	0,0	170	0,0	170	170	0,0	0,10
Lane Group Flow (vph)	134	127	142	0	0	58	109	0	76	444	58	0
Turn Type	pm+pt	NA	Perm	•	pm+pt	pm+pt	NA	Ť	Prot	NA	Perm	pm+pt
Protected Phases	ې ور 7	4			3	3	8		5	2		ې ۲۰۱۱ 1
Permitted Phases	4	•	4		8	8	•		•	_	2	6
Detector Phase	7	4	4		3	3	8		5	2	2	1
Switch Phase					-	-	-		-			
Minimum Initial (s)	5.0	7.0	7.0		5.0	5.0	7.0		5.0	26.7	26.7	5.0
Minimum Split (s)	8.0	11.2	11.2		8.0	8.0	11.2		11.7	32.9	32.9	11.5
Total Split (s)	11.0	16.2	16.2		11.0	11.0	16.2		18.7	46.9	46.9	18.5
Total Split (%)	9.7%	14.3%	14.3%		9.7%	9.7%	14.3%		16.5%	41.3%	41.3%	16.3%
Yellow Time (s)	3.0	3.0	3.0		3.0	3.0	3.0		3.0	3.7	3.7	3.7
All-Red Time (s)	0.0	1.2	1.2		0.0	0.0	1.2		3.7	2.5	2.5	2.8
Lost Time Adjust (s)	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Lost Time (s)	3.0	4.2	4.2			3.0	4.2		6.7	6.2	6.2	
Lead/Lag	Lead	Lag	Lag		Lead	Lead	Lag		Lead	Lead	Lead	Lag
Lead-Lag Optimize?	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	Yes	Yes
Recall Mode	None	None	None		None	None	None		None	C-Min	C-Min	None
Act Effct Green (s)	20.7	13.1	13.1			18.7	10.6		9.2	48.9	48.9	
Actuated g/C Ratio	0.18	0.12	0.12			0.16	0.09		0.08	0.43	0.43	
v/c Ratio	0.64	0.60	0.78			0.27	0.60		0.56	0.56	0.09	
Control Delay	54.2	61.6	78.2			40.3	63.4		65.6	30.5	23.7	
Queue Delay	0.0	0.0	0.0			0.0	0.0		0.0	0.0	0.0	
Total Delay	54.2	61.6	78.2			40.3	63.4		65.6	30.5	23.7	
LOS	04.2 D	E	70.2 E			40.3 D	03.4 E		05.0 E	50.5 C	23.7 C	
Approach Delay	J	65.0	L			U	55.4		L	34.4	U	
Approach LOS		05.0 E					55.4 E			04.4 C		
		Ľ					L			U		

CGH 01/05/2022 Synchro 11 Report Lanes, Volumes, Timings

	L,	ţ	~	Ł	•	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Lane Configurations	2002	1		5	1 IIII	
Traffic Volume (vph)	30	318	94	16	94	14
Future Volume (vph)	30	318	94	16	94	14
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	11	1500	15	11	12	12
Grade (%)		5%	10	-1%	12	14
Storage Length (ft)	265	0,0	0	130	0	
Storage Lanes	1		0	100	1	
Taper Length (ft)	60		Ū	40	1	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	1.00	0.966	1.00	1.00	0.850	1.00
Flt Protected	0.950	0.000		0.950	0.000	
Satd. Flow (prot)	1701	1949	0	1754	1623	0
Flt Permitted	0.950	1343	U	0.950	1025	0
Satd. Flow (perm)	1701	1949	0	1754	1623	0
Right Turn on Red	1701	10-10	No	11.34	1023	No
Satd. Flow (RTOR)			NU			NU
Link Speed (mph)		25		25		
Link Distance (ft)		257		629		
Travel Time (s)		7.0		17.2		
Peak Hour Factor	0.93	0.93	0.93	0.93	0.93	0.93
Heavy Vehicles (%)	0.93	0.93	0.93	0.93	0.93	0.93
Shared Lane Traffic (%)	U 70	1 70	1 70	070	076	076
( )	74	443	0	17	116	0
Lane Group Flow (vph)	74 Prot	443 NA	U	Prot		U
Turn Type Protected Phases	Prot 1	NA 6		9	Prot 9	
Protected Phases Permitted Phases		0		9	9	
	1	6		9	9	
Detector Phase	T	0		9	9	
Switch Phase	ΕŌ	<u> </u>		7.0	7.0	
Minimum Initial (s)	5.0	26.2		7.0	7.0	
Minimum Split (s)	11.5	32.7		14.0	14.0	
Total Split (s)	18.5	46.7		21.0	21.0	
Total Split (%)	16.3%	41.1%		18.5%	18.5%	
Yellow Time (s)	3.7	3.7		3.2	3.2	
All-Red Time (s)	2.8	2.8		3.8	3.8	
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	
Total Lost Time (s)	6.5	6.5		7.0	7.0	
Lead/Lag	Lag	Lag				
Lead-Lag Optimize?	Yes	Yes				
Recall Mode	None	C-Min		None	None	
Act Effct Green (s)	10.2	49.4		11.5	11.5	
Actuated g/C Ratio	0.09	0.43		0.10	0.10	
v/c Ratio	0.49	0.52		0.10	0.70	
Control Delay	59.8	29.0		46.0	71.4	
Queue Delay	0.0	0.0		0.0	0.0	
Total Delay	59.8	29.0		46.0	71.4	
LOS	E	С		D	E	
Approach Delay		33.4		68.2		
Approach LOS		С		E		

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Lane Group	EBL	EBT	EBR	EBR2	WBL2	WBL	WBT	WBR	NBL	NBT	NBR	SBL2
Queue Length 50th (ft)	85	91	104			35	77		54	255	26	
Queue Length 95th (ft)	#145	#170	#219			72	136		102	390	58	
Internal Link Dist (ft)		159					190			743		
Turn Bay Length (ft)											65	
Base Capacity (vph)	212	210	182			232	205		177	787	676	
Starvation Cap Reductn	0	0	0			0	0		0	0	0	
Spillback Cap Reductn	0	0	0			0	0		0	0	0	
Storage Cap Reductn	0	0	0			0	0		0	0	0	
Reduced v/c Ratio	0.63	0.60	0.78			0.25	0.53		0.43	0.56	0.09	
Intersection Summary												
Area Type:	Other											
Cycle Length: 113.6												
Actuated Cycle Length: 11	13.6											
Offset: 0 (0%), Reference	d to phase 2:	<b>NBT</b> and	6:SBTL, S	Start of Y	ellow, Ma	ster Inters	ection					
Natural Cycle: 80												
Control Type: Actuated-Co	oordinated											
Maximum v/c Ratio: 0.78												
Intersection Signal Delay:				lr	ntersectior	n LOS: D						
Intersection Capacity Utiliz	zation 66.6%			10	CU Level o	of Service	С					
Analysis Period (min) 15												
# 95th percentile volume	e exceeds cap	bacity, qu	eue may	be longe	r.							
Queue shown is maxim	num after two	cycles.										
Splits and Phases: 10: I	North Avenue	e & Housa	Itonic Ave	enue & Li	ndley Stre	et						



	L.	Ļ	∢	•	*	4
Lane Group	SBL	SBT	SBR	NWL	NWR	NWR2
Queue Length 50th (ft)	53	250		11	83	
Queue Length 95th (ft)	101	381		34	143	
Internal Link Dist (ft)		177		549		
Turn Bay Length (ft)	265			130		
Base Capacity (vph)	179	847		216	200	
Starvation Cap Reductn	0	0		0	0	
Spillback Cap Reductn	0	0		0	0	
Storage Cap Reductn	0	0		0	0	
Reduced v/c Ratio	0.41	0.52		0.08	0.58	
Intersection Summary						

Intersection						
Int Delay, s/veh	0.4					
int Delay, S/Ven	0.4					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			ŧ	el el	
Traffic Vol, veh/h	0	0	45	604	413	26
Future Vol, veh/h	0	0	45	604	413	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	-1	2	-
Peak Hour Factor	25	25	96	96	96	96
Heavy Vehicles, %	0	0	2	1	1	2
Mvmt Flow	0	0	47	629	430	27

Major/Minor	Minor2	l	Major1	Мај	jor2	
Conflicting Flow All	1167	444	457	0	-	0
Stage 1	444	-	-	-	-	-
Stage 2	723	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.12	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.218	-	-	-
Pot Cap-1 Maneuver	216	618	1104	-	-	-
Stage 1	651	-	-	-	-	-
Stage 2	484	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	202	618	1104	-	-	-
Mov Cap-2 Maneuver	202	-	-	-	-	-
Stage 1	609	-	-	-	-	-
Stage 2	484	-	-	-	-	-

Approach	EB	NB	SB
HCM Control Delay, s	0	0.6	0
HCM LOS	А		

Minor Lane/Major Mvmt	NBL	IBL NBT EBLn1		SBT	SBR
Capacity (veh/h)	1104	-	-	-	-
HCM Lane V/C Ratio	0.042	-	-	-	-
HCM Control Delay (s)	8.4	0	0	-	-
HCM Lane LOS	А	А	Α	-	-
HCM 95th %tile Q(veh)	0.1	-	-	-	-

Intersection						
Int Delay, s/veh	0.7					
					0.D.T	
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations		1		<b>↑</b>	<b>↑</b>	
Traffic Vol, veh/h	0	68	0	649	413	0
Future Vol, veh/h	0	68	0	649	413	0
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized		None	-	None		None
Storage Length	-	0	-	-	-	-
Veh in Median Storage,	# 0	-	-	0	0	-
Grade, %	0	-	-	-2	2	-
Peak Hour Factor	96	96	96	96	96	96
Heavy Vehicles, %	2	2	2	1	1	2
Mvmt Flow	0	71	0	676	430	0
	v		v	010	100	v

Major/Minor	Minor2	Ν	1ajor1	Ма	ajor2	
Conflicting Flow All	-	430	-	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.22	-	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.318	-	-	-	-
Pot Cap-1 Maneuver	0	625	0	-	-	0
Stage 1	0	-	0	-	-	0
Stage 2	0	-	0	-	-	0
Platoon blocked, %				-	-	
Mov Cap-1 Maneuver	• -	625	-	-	-	-
Mov Cap-2 Maneuver	· -	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s			0		0	
HCM LOS	B		v		U	

Minor Lane/Major Mvmt	NBT EBLn1	SBT
Capacity (veh/h)	- 625	-
HCM Lane V/C Ratio	- 0.113	-
HCM Control Delay (s)	- 11.5	-
HCM Lane LOS	- B	-
HCM 95th %tile Q(veh)	- 0.4	-

1-	CI	TY OF BRIDG	EPORT	File No.	
A BAR	PLANNING &	& ZONING			· · · · · · · · · · · · · · · · · · ·
	NAME OF APPLICANT: 547 Ellswort	h NavCapMan	LLC		
	Is the Applicant's name Trustee of Reco	rd? Yes_	No	Χ.	
	If yes, a sworn statement disclosing the Address of Property: <u>543-545, 547, 54</u>				ing.
	(number)	(street	(sta	te)	(zip code)
	Assessor's Map Information: Block No.	11/217	Lot	No. <u>17, 18, 19</u>	9 & 31
. 3	Amendments to Zoning Regulations: (in	dicate) Article: _	N/A	Section:	
	(Attach copies of Amendment)				
	Description of Property (Metes & Bound 42.16' x 251.96' x 206.78'	s): <u>See submit</u>	ed survey; 56.20' x 1	10.77' x 59.00	' x 103.15' x 50.09
. 1	Existing Zone Classification: R-CC				
	Zone Classification requested: N/A				
	Describe Proposed Development of Pro	perty: Propose	d construction of res	idential multi-f	amily apartment
	dwelling to contain 123 dwelling units				
į	Approval(s) requested: <u>Coastal Site Pl</u>	an Review and	Site Plan Review		
7		1			
	Signature:	n		Date: 12/2	23/2021
	Print Name:				
	If signed by Agent, state capacity (Lawy	er, Developer, e			
	If signed by Agent, state capacity (Lawy	· · ·	Print Name:	d Fairfield C	T 06824
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u>	sso & Rizio, LL	Print Name: C, 10 Sasco Hill Roa		Г 06824
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u>	so & Rizio, LL Cell: <u>203-5</u>	Print Name:	d, Fairfield, C Fax:	Г 06824
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u>	so & Rizio, LL Cell: <u>203-5</u>	Print Name: C, 10 Sasco Hill Roa		Г 06824
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u>	sso & Rizio, LL( Cell: <u>203-5</u> m	Print Name: C, 10 Sasco Hill Roa 20-4603	Fax:	Г 06824
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u>	so & Rizio, LL Cell: <u>203-5</u>	Print Name: C, 10 Sasco Hill Roa 20-4603	Fax:	Γ 06824
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received	bso & Rizio, LL( Cell: 203-5 m Date:	Print Name:	Fax:	
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u>	SSO & Rizio, LL0         Cell:       203-5         m         Date:	Print Name: C, 10 Sasco Hill Roa 20-4603 Clen	Fax: k: T <u>H COMPLETE</u>	ED CHECKLIST
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo	SSO & Rizio, LL0         Cell:       203-5         m         Date:	Print Name: C, 10 Sasco Hill Roa 20-4603 Clean N PERSON AND WIT A-2 Site Survey	Fax: -k: - <u>H COMPLETE</u>	ED CHECKLIST Building Floor Pla
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan	SSO & Rizio, LLO         Cell:       203-5         m         Date:	Print Name: C, 10 Sasco Hill Roa 20-4603 Clean M PERSON AND WIT A-2 Site Survey Drainage Plan	Fax: k: T <u>H COMPLETE</u>	ED CHECKLIST Building Floor Pla Building Elevation
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan Written Statement of Development a	BSO & Rizio, LLO         Cell:       203-5         m         Date:         E SUBMITTED I         rm         and Use	Print Name: C, 10 Sasco Hill Roa 20-4603 Cler N PERSON AND WIT A-2 Site Survey Drainage Plan Property Owner	Fax: rk: f <u>H COMPLETE</u> s List	ED CHECKLIST Building Floor Pla
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan	BSO & Rizio, LLO         Cell:       203-5         m         Date:         E SUBMITTED I         rm         and Use	Print Name: C, 10 Sasco Hill Roa 20-4603 Cler N PERSON AND WIT A-2 Site Survey Drainage Plan Property Owner	Fax: rk: f <u>H COMPLETE</u> s List	E <u>D CHECKLIST</u> Building Floor Pla Building Elevatio
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan Written Statement of Development a Cert. of Incorporation & Organization	and Use n and First Repo	Print Name: C, 10 Sasco Hill Roa 20-4603 Cler M PERSON AND WIT A-2 Site Survey Drainage Plan Property Owner' port (Corporations & LL	Fax: rk: <i>T<u>H COMPLETE</u> ∎ s List □ C's)</i>	ED CHECKLIST Building Floor Pla Building Elevation Fee
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan Written Statement of Development a Cert. of Incorporation & Organization	and Use n and First Repo	Print Name: C, 10 Sasco Hill Roa 20-4603 Cler N PERSON AND WIT A-2 Site Survey Drainage Plan Property Owner	Fax: *k: T <u>H COMPLETE</u> ■ s List □ C's) A <u>PPLICATION</u>	ED CHECKLIST Building Floor Pla Building Elevation Fee
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan Written Statement of Development a Cert. of Incorporation & Organization <u>PROPER</u> 547 Ellsworth NavCapMan LLC	and Use n and First Repo	Print Name: C, 10 Sasco Hill Roa 20-4603 Clean M PERSON AND WIT A-2 Site Survey Drainage Plan Property Owner ort (Corporations & LL MDORSEMENT OF A	Fax: *k: T <u>H COMPLETE</u> ■ s List □ C's) A <u>PPLICATION</u>	ED CHECKLIST Building Floor Pla Building Elevation Fee
	If signed by Agent, state capacity (Lawy Mailing Address: <u>c/o Chris Russo, Rus</u> Phone: <u>203-528-0590</u> <b>E-mail Address</b> : <u>Chris@russorizio.co</u> \$Fee received <u>THIS APPLICATION MUST BE</u> Completed & Signed Application Fo Completed Site / Landscape Plan Written Statement of Development a Cert. of Incorporation & Organization	and Use n and First Repo	Print Name: C, 10 Sasco Hill Roa 20-4603 Cler M PERSON AND WIT A-2 Site Survey Drainage Plan Property Owner' port (Corporations & LL	Fax: *k: T <u>H COMPLETE</u> ■ s List □ C's) A <u>PPLICATION</u>	ED CHECKLIST Building Floor Pla Building Elevatio Fee

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December 23, 2021

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Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, CT 06604

Re: Petition for Site Plan Review and Coastal Site Plan Review – 543-545, 547, 549 & 557 Ellsworth Street

Dear Mr. Buckley:

Please accept this Petition to the Bridgeport Planning and Zoning Commission for Site Plan Review and Coastal Site Plan Review on behalf of my client, 547 Ellsworth NavCapMan LLC, for the properties located at 543-545, 547, 549 & 557 Ellsworth Street (the "Site") in the R-CC Zone.

#### Proposed Development & Use

The Petitioner proposes to construct a single residential multi-family apartment dwelling on the Site with associated Site improvements. The Site is located entirely within the R-CC Zone and the coastal boundary. It only has frontage on Ellsworth Street. The Site currently abuts a large apartment building to its north, a Dunkin Donuts to its south, the Wakeman Boys and Girls Club and Burroughs Community Center across the street, and multi-family dwellings to its rear.

Multi-family dwellings are a permitted use within the R-CC Zone. The Site currently contains a mix of single-family and three-family dwellings. The Petitioner proposes to demolish the existing buildings and structures on the Site. The Petitioner proposes to construct a six-story apartment building containing One hundred and twenty-three (123) residential dwelling units.

The Site will be accessed via Ellsworth Street. The Petition proposes a Two (2) level garage for a total of One hundred and thirty-five (135) off-street parking spaces, which is in compliance with the Regulations. A predominant amount of the proposed parking will be located under cover. A number of other amenities are proposed for the Site, including a mail room, deck, gym, office space and community area on the main floor of the proposed building. The residential floors will be accessed via Three (3) stairwells and an elevator. The proposed building will contain Twenty-six (26) studio, Fifty-four (54) one-bedroom and Forty-three (43) two-bedroom dwelling units. A typical studio dwelling unit will contain a full kitchen, living/dining room and open bedroom area, walk-in/storage closet and full bath. A typical one-bedroom dwelling unit will contain a private bedroom with a walk-in closet in addition to the studio unit features. The two-bedroom dwelling units will feature an additional bedroom and full bath.

The submitted elevations show a variety of materials and colors consistent with apartment design found in new construction throughout the City and surrounding area. The Site will be connected via public sidewalks to the convenient Fairfield Avenue corridor. A significant amount of landscaping will be added to the Site with plantings along the rear property line and street trees along the frontage. Existing structures along the rear property line will also be removed. The Petition will be a tremendous improvement to the Site and neighborhood to provide new construction housing to Bridgeport residents.

#### Site Plan Review

The Petition satisfies the Section 14-2-5 Site Plan Review standards of the Regulations. The design of the proposed buildings and landscaping create a harmonious building-street interaction providing a tremendous improvement to the existing streetscape. The scale and proportion of the buildings conform to the R-CC Zone Development Standards as it is fully compliant with the Regulations. The Petition proposes significant landscaping along the rear property line and street frontage. The proposed multi-family residential dwelling use and its density are permitted in the R-CC Zone. The proposed use and building replace dated dwellings on an underutilized Site. The Site directly abuts another high-density apartment building, so the proposed use will be in conformity with the area.

As stated above, the proposed design of the building and its proximity to the Fairfield Avenue corridor will be a great asset for residents of the neighborhood. The Petition proposes more adequate off-street parking and accessible spaces as required under the Regulations. This parking will mainly be located in a covered garage. The Petition conforms to the permitted standards under the Regulations.

#### Coastal Site Plan Review

The Petition also complies with Section 14-3 of the Regulations regarding coastal site plan review. While the Site is located within the coastal boundary, it is over Nine hundred feet (900') from Ash Creek, which is the nearest coastal resource. Dozens of buildings and multiple streets and blocks exist between the coastal resource and the Site. It has no connection to the coastal resource but for being included within its boundary. There are no natural features associated with the coastal resource on the Site. As stated above, the Petition fully complies with the site plan review standards of the Regulations. The Petition poses no danger or threat to coastal resources and it has no potential adverse impacts. The proposed building and Site improvements will all be constructed in accordance with current codes and regulations, including appropriate stormwater drainage systems. Appropriate sediment and erosion controls, such as silt fencing and anti-tracking aprons, will be utilized during construction and stockpiles will be located at the rear of the Site.

For these reasons, we respectfully request approval of the Petition to construct a multi-family residential apartment dwelling containing One hundred and twenty-three (123) dwelling units with associated Site improvements on the Site in the R-CC Zone.

Sincerely,

Christopher Russo

LIST OF PROPERTIES WITHIN 100' OF 543-545, 547, 549 & 557 ELLSWORTH ST.

I .	138 SCOFIELD AV #140	128 SCOFIELD AVE #130	116 SCOFIELD AV #120	575 ELLSWORTH ST	557 ELLSWORTH ST	106 SCOFIELD AV #110	2550 FAIRFIELD AV	68 SCOFIELD AV #70	78 SCOFIELD AV #82	543 ELLSWORTH ST #545	90 SCOFIELD AV #92	547 ELLSWORTH ST	98 SCOFIELD AV #100	549 ELLSWORTH ST	2592 FAIRFIELD AV #2594	58 SCOFIELD AV #60	2578 FAIRFIELD AV #2580	48 SCOFIELD AV	2468 FAIRFIELD AV	PROPERTY ADDRESS	
	HOUSING AUTHORITY CITY OF BRIDGEPORT	HOUSING AUTHORITY CITY OF BPT	RAMOS TALI	ROCKRODGE LIVING LLC	547 ELLSWORTH NAVCAPMAN LLC	SYTNYK VICTOR & MARIYA	NKJC, LLC	NESTOR N NKWO	STEVENS DAVID J	547 ELLSWORTH NAVCAPMAN LLC	HABANSKY KATE J	547 ELLSWORTH NAVCAPMAN LLC	SANTOS DAGOBERTO	547 ELLSWORTH NAVCAPMAN LLC	SPEIGEL REAL ESTATE HOLDINGS LLC	TRI-STATE EAST BPT MINGT LLC	COLLINS SAGIO EDMARIE BROWN	PHELAN CHRISTOPHER & GLUNZ LOUIS IV	WAKEMAN BOYS & GIRLS CLUB CORP	OWNER	
	<b>150 HIGHLAND AVE</b>	376 EAST WASHINGTON AVE	PO BOX 10970	1 BRADFORD ST	2 ENTERPRISE DR STE 406	20 TIMBER LANE	22 MEADOW BROOK ROAD	68 SCOFIELD AVE # 70	666 COURTLAND AVENUE	2 ENTERPRISE DR STE 406	90 SCOFIELD AVE # 92	2 ENTERPRISE DR STE 406	<b>10 GREENWOOD AVE</b>	2 ENTERPRISE DR STE 406	31 MAPLE LANE	244 BENNETT ST	2578 FAIRFIELD AVE #2580	48 SCOFIELD AVE	<b>385 CENTER STREET</b>	MAILING ADDRESS	
	BRIDGEPORT	BRIDGEPORT	STAMFORD	BRISTOL	SHELTON	STAMFORD	NEWTOWN	BRIDGEPORT	BRIDGEPORT	SHELTON	BRIDGEPORT	SHELTON	PORT CHESTER	SHELTON	WESPORT	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	SOUTHPORT	CITY	
	ŋ	CT	CI	RI	CT	CT	CT	CI	Ŋ	CT	CI	CI	NY	CI	CT	CI	CI	Ŋ	CI	STATE	
	06604	06608	06904	02809	06484	06905	06470	06605	06605	06484	06605	06484	10573	06484	06880	06605	06605	06605	06890	: ZIP	

onlineBusinessSearch

#### 547 ELLSWORTH NAVCAPMAN LLC ACTIVE

2 ENTERPRISE DRIVE SUITE 406, SHELTON, CT, 06484, United States

BUSINESS DETAILS ~	
Business Details	
General Information	_
Business Name 547 ELLSWORTH NAVCAPMAN LLC	
Business status ACTIVE	
Citizenship/place of formation Domestic/Connecticut	
Business address 2 ENTERPRISE DRIVE SUITE 406, SHELTON, CT, 06484, United States	
Annual report due 3/31/2022	
NAICS code Lessors of Residential Buildings and Dwellings (531110)	
Business ALEI 1115573	
Date formed 8/20/2013	
Business type LLC	
Mailing address 2 ENTERPRISE DRIVE SUITE 406, SHELTON, CT, 06484, United States	
Last report filed 2021	
NAICS sub code 531110	
Principal Details	-
Principal Name NAVCAPMAN LLC	
Principal Title	
MANAGER/MEMBER Principal Business address 1023 MAIN STREET, 2ND FLOOR, BRIDGEPORT, CT, 06604, United States	

#### 12/28/21, 12:25 PM

onlineBusinessSearch

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^

Principal Name CARNOUSTIE NAVCAPMAN LLC

Principal Title MEMBER

Principal Business address 1023 MAIN STREET, 2ND FLOOR, BRIDGEPORT, CT, 06604, United States

Principal Name TURNBERRY AMERICAS LLC

Principal Title MEMBER

Principal Business address 315 WEST 57TH STREET, APT. 7K, NEW YORK, NY, 10019, United States

#### Agent details

Agent name NEIL A. LIPPMAN

Agent Business address 200 CONNECTICUT AVENUE, NORWALK, CT, 06854, United States

Agent Mailing address 200 CONNECTICUT AVENUE, NORWALK, CT, 06854, United States

Agent Residence addresss 144 RED OAK ROAD , FAIRFIELD, CT, 06825, United States

#### **Filing History**

ß	Business Formation - Certificate of Organization 0004927963 Filing date: 8/20/2013
	Volume Type B
	Volume 1839
	Start page 1575
	Pages 2
	Date generated 8/20/2013
B	Interim Notice - Interim Notice 0005115630 Filing date: 6/2/2014
	Volume Type B



## 54CITY OF BRIDGEPORT

Application Form **Municipal Coastal Site Plan Review** For Projects Located Fully or Partially Within the Coastal Boundary

Please complete this form in accordance with the attached instructions (CSPR-INST-11/99) and submit it with the appropriate plans to the Zoning office.

#### Section I: Applicant Identification

#### Section II: Project Site Plans

Please provide project site plans that clearly and accurately depict the following information, and check the appropriate boxes to indicate that the plans are included in this application:

KProject location

KExisting and proposed conditions, including buildings and grading

KCoastal resources on and contiguous to the site

 $\Gamma$  High tide line [as defined in CGS Section 22a-359(c)] and mean high water mark elevation

contours (for parcels abutting coastal waters and/or tidal wetlands only)

XSoil erosion and sediment controls

K Stormwater treatment practices

K Ownership and type of use on adjacent properties

KReference datum (i.e., National Geodetic Vertical Datum, Mean Sea Level, etc.)

#### Section III: Written Project Information

Please check the appropriate box to identify the plan or application that has resulted in this Coastal Site Plan Review:

R Site Plan for Zoning Compliance

Γ Subdivision or Resubdivision

Γ Special Permit or Special Exception

Γ Variance

Γ Municipal Project (CGS Section 8-24)

#### Part I: Site Information

 Street Address or Geographical Description: 543-545, 547, 549 & 557 Ellsworth Street City or Town: Bridgeport 2. Is project or activity proposed at a waterfront site (includes tidal wetlands frontage)? ΓYES KNO Name of on-site, adjacent or downstream coastal, tidal or navigable waters, if applicable: Ash Creek is located over 900' from the Site. There is no adjacent water. 4. Identify and describe the existing land use on and adjacent to the site. Include any existing structures, municipal zoning classification, significant features of the project site: The Site currently contains Three (3) single-family dwellings and a three-family dwelling along with several accessory structures. The Site is located in the R-CC Zone. A multi-family residential apartment building is located to the North of the Site, a drive-through Dunkin Donuts restaurant is to the south, a Wakeman Boys and Girls Club is across the street, and multi-family dwellings are located to the rear of the Site. 5. Indicate the area of the project site: 1.06 acres or square feet (circle one) 6. Check the appropriate box below to indicate total land area of disturbance of the project or activity (please also see Part II.B. regarding proposed stormwater best management practices): Г Project or activity will disturb 5 or more total acres of land area on the site. It may be eligible for registration for the Department of Environmental Protection's (DEP) General Permit for the Discharge of Stormwater and Dewatering Wastewaters Associated with **Construction Activities** X Project or activity will disturb one or more total acres but less than 5 total acres of land area. A soil erosion and sedimentation control plan must be submitted to the municipal land use agency reviewing this application. Г Project or activity will not disturb 1 acre total of land area. Stormwater management controls may be required as part of the coastal site plan review. 7. Does the project include a shoreline flood and erosion control structure as defined in CGS section 22a-109(d) X Yes □ No

#### Part II.A.: Description of Proposed Project or Activity

Describe the proposed project or activity including its purpose and related activities such as site clearing, grading, demolition, and other site preparations; percentage of increase or decrease in impervious cover over existing conditions resulting from the project; phasing, timing and method of proposed construction; and new uses and changes from existing uses (attach additional pages if necessary):

The Petitioner proposes to demolish the existing buildings on the Site and construct a six-story apartment building containing One hundred and twenty-three (123)

residential dwelling units. The Petitioner will construct a two-level garage to provide sufficient parking for the development. The proposed grading is shown on the submitted plan. The proposed building and site coverage is below the maximum standards of the zone under the Zoning Regulations. The development will be completed in one phase

in an anticipated Twenty-four (24) months of construction.

#### Part II.B.: Description of Proposed Stormwater Best Management Practices

Describe the stormwater best management practices that will be utilized to ensure that the volume of runoff generated by the first inch of rainfall is retained on-site, especially if the site or stormwater discharge is adjacent to tidal wetlands. If runoff cannot be retained on-site, describe the site limitations that prevent such retention and identify how stormwater will be treated before it is discharged from the site. Also demonstrate that the loadings of total suspended solids from the site will be reduced by 80 percent on an average annual basis, and that post-development stormwater runoff rates and volumes will not exceed pre-development runoff rates and volumes (attach additional pages if necessary).

Storm water run-off from the building and the driveway and parking areas will be treated with a subsurface system. The primary stormwater treatment will be implemented as to Stormwater Best Management Practice.

#### Part III: Identification of Applicable Coastal Resources and Coastal Resource Policies

Identify the coastal resources and associated policies that apply to the project by placing a check mark in the appropriate box(es) in the following table.

Coastal Resources	On-site	Adjacent	Off-site but within the influence of project	Not Applicable
General Coastal Resources* - Definition: CGS Section 22a-93(7); Policy: CGS Section 22a-92(a)(2)	X	$\times$	$\times$	
Beaches & Dunes - Definition: CGS Section 22a-93(7)(C); Policies: CGS Sections 22a-92-(b)(2)(C) and 22a-92(c)(1)(K)				X
Bluffs & Escarpments - Definition: CGS Section 22a-93(7)(A); Policy: CGS Section 22a-92(b)(2)(A)		5		X
Coastal Hazard Area - Definition: CGS Section 22a-93(7)(H); Policies: CGS Sections 22a-92(a)(2), 22a-92(a)(5), 22a-92(b)(2)(F), 22a- 92(b)(2)(J), and 22a-92(c)(2)(B)	ti.			×
Coastal Waters, Estuarine Embayments, Nearshore Waters, Offshore Waters - Definition: CGS Sections 22a-93(5), 22a-93(7)(G), and 22a- 93(7)(K), and 22a-93(7)(L) respectively; Policies: CGS Sections 22a-92(a)(2) and 22a-92(c)(2)(A)				X
Developed Shorefront - Definition: CGS Section 22a-93(7)(I); Policy: 22a-92(b)(2)(G)				X
Freshwater Wetlands and Watercourses - Definition: CGS Section 22a-93(7)(F); Policy: CGS Section 22a-92(a)(2)				X
Intertidal Flats - Definition: CGS Section 22a-93(7)(D); Policies: 22a-92(b)(2)(D) and 22a-92(c)(1)(K)		~		×
Islands - Definition: CGS Section 22a-93(7)(J); Policy: CGS Section 22a-92(b)(2)(H)				×
Rocky Shorefront - Definition: CGS Section 22a-93(7)(B); Policy: CGS Section 22a-92(b)(2)(B)		<u>N</u>		×
Shellfish Concentration Areas - Definition: CGS Section 22a-93(7)(N); Policy: CGS Section 22a-92(c)(1)(I)				X
Shorelands - Definition: CGS Section 22a-93(7)(M); Policy: CGS Section 22a-92(b)(2)(I)				X
Tidal Wetlands - Definition: CGS Section 22a-93(7)(E); Policies: CGS Sections 22a-92(a)(2), 22a-92(b)(2)(E), and 22a- 92(c)(1)(B)				×

\* General Coastal Resource policy is applicable to all proposed activities

#### Part IV: Consistency with Applicable Coastal Resource Policies and Standards

Describe the location and condition of the coastal resources identified in Part III above and explain how the proposed project or activity is consistent with all of the applicable coastal resource policies and standards; also see adverse impacts assessment in Part VII.A below (attach additional pages if necessary): Ash Creek, which is the closest coastal resource to the Site, is located over 900' from the Site. The proposed project complies with CGS Sec. 22a-92(a)(1) "...by promoting economic growth without significantly disrupting the environment...", with CGS Sec. 22a-92(b)(2)(F) "...manage coastal hazard areas to minimize hazards to property..." and with CGS Sec. 22a-92(c)(2)(B) "...maintain patterns of water circulation in the placement of drainage control structures..."

#### Part V: Identification of Applicable Coastal Use and Activity Policies and Standards

Identify all coastal policies and standards in or referenced by CGS Section 22a-92 applicable to the proposed project or activity:

- X General Development\* CGS Sections 22a-92(a)(1), 22a-92(a)(2), and 22a-92(a)(9)
- 9 Water-Dependent Uses\*\* CGS Sections 22a-92(a)(3) and 22a-92(b)(1)(A);
   Definition CGS Section 22a-93(16)
- 9 Ports and Harbors CGS Section 22a-92(b)(1)(C)
- 9 Coastal Structures and Filling CGS Section 22a-92(b)(1)(D)
- 9 Dredging and Navigation CGS Sections 22a-92(c)(1)(C) and 22a-92(c)(1)(D)
- 9 Boating CGS Section 22a-92(b)(1)(G)
- 9 Fisheries CGS Section 22a-92(c)(1)(I)
- 9 Coastal Recreation and Access CGS Sections 22a-92(a)(6), 22a-92(C)(1)(j) and 22a-92(c)(1)(K)
- 9 Sewer and Water Lines CGS Section 22a-92(b)(1)(B)
- 9 Fuel, Chemicals and Hazardous Materials CGS Sections 22a-92(b)(1)(C), 22a-92(b)(1)(E) and 22a-92(c)(1)(A)
- 9 Transportation CGS Sections 22a-92(b)(1)(F), 22a-92(c)(1)(F), 22a-92(c)(1)(G), and 22a-92(c)(1)(H)
- 9 Solid Waste CGS Section 22a-92(a)(2)
- 9 Dams, Dikes and Reservoirs CGS Section 22a-92(a)(2)
- 9 Cultural Resources CGS Section 22a-92(b)(1)(J)
- 9 Open Space and Agricultural Lands CGS Section 22a-92(a)(2)

<sup>\*</sup> General Development policies are applicable to all proposed activities

<sup>\*\*</sup> Water-dependent Use policies are applicable to all activities proposed at waterfront sites, including those with tidal wetlands frontage.

#### Part VI: Consistency With Applicable Coastal Use Policies And Standards

Explain how the proposed activity or use is consistent with all of the applicable coastal use and activity policies and standards identified in Part V. For projects proposed at waterfront sites (including those with tidal wetlands frontage), particular emphasis should be placed on the evaluation of the project's consistency with the water-dependent use policies and standards contained in CGS Sections 22a-92(a)(3) and 22a-92(b)(1)(A) -- also see adverse impacts assessment in Part VII.B below (attach additional pages if necessary):

No adverse impacts were determined on adjacent coastal resources. Stormwater treatment is proposed which will help reduce

erosion impacts as well as provide water infiltration. This project will be limited to the confines of the Site and will be completed within

Twenty-four (24) months. All disturbed pervious areas will be loamed, seeded and planted upon completion of construction.

#### Part VII.A.: Identification of Potential Adverse Impacts on Coastal Resources

#### Please complete this section for all projects.

Identify the adverse impact categories below that apply to the proposed project or activity. The Aapplicable≅ column **must** be checked if the proposed activity has the **potential** to generate any adverse impacts as defined in CGS Section 22a-93(15). If an adverse impact may result from the proposed project or activity, please use Part VIII to describe what project design features may be used to eliminate, minimize, or mitigate the potential for adverse impacts.

Potential Adverse Impacts on Coastal Resources	Applicable	Not Applicable
Degrading tidal wetlands, beaches and dunes, rocky shorefronts, and bluffs and escarpments through significant alteration of their natural characteristics or functions - CGS Section 22a-93(15)(H)		×
Increasing the hazard of coastal flooding through significant alteration of shoreline configurations or bathymetry, particularly within high velocity flood zones - CGS Section 22a-93(15)(E)		
Degrading existing circulation patterns of coastal water through the significant alteration of patterns of tidal exchange or flushing rates, freshwater input, or existing basin characteristics and channel contours - CGS Section 22a-93(15)(B)		×
Degrading natural or existing drainage patterns through the significant alteration of groundwater flow and recharge and volume of runoff - CGS Section 22a-93(15)(D)		×
Degrading natural erosion patterns through the significant alteration of littoral transport of sediments in terms of deposition or source reduction - CGS Section 22a-93(15)(C)		×
Degrading visual quality through significant alteration of the natural features of vistas and view points - CGS Section 22a-93(15)(F)		×
Degrading water quality through the significant introduction into either coastal waters or groundwater supplies of suspended solids, nutrients, toxics, heavy metals or pathogens, or through the significant alteration of temperature, pH, dissolved oxygen or salinity - CGS Section 22a-93(15)(A)		X
Degrading or destroying essential wildlife, finfish, or shellfish habitat through significant alteration of the composition, migration patterns, distribution, breeding or other population characteristics of the natural species or significant alterations of the natural components of the habitat - CGS Section 22a-93(15)(G)		×

#### Part VII.B.: Identification of Potential Adverse Impacts on Water-dependent Uses

Please complete the following two sections only if the project or activity is proposed at a waterfront site:

 Identify the adverse impact categories below that apply to the proposed project or activity. The Aapplicable≅ column must be checked if the proposed activity has the potential to generate any adverse impacts as defined in CGS Section 22a-93(17). If an adverse impact may result from the proposed project or activity, use Part VIII to describe what project design features may be used to eliminate, minimize, or mitigate the potential for adverse impacts.

Potential Adverse Impacts on Future Water-dependent Development Opportunities and Activities	Applicable	Not Applicable
Locating a non-water-dependent use at a site physically suited for or planned for location of a water-dependent use - CGS Section 22a-93(17)		X
Replacing an existing water-dependent use with a non-water- dependent use - CGS Section 22a-93(17)	ų	X
Siting a non-water-dependent use which would substantially reduce or inhibit existing public access to marine or tidal waters - CGS Section 22a-93(17)		X

2. Identification of existing and/or proposed Water-dependent Uses

Describe the features or characteristics of the proposed activity or project that qualify as waterdependent uses as defined in CGS Section 22a-93(16). If general public access to coastal waters is provided, please identify the legal mechanisms used to ensure public access in perpetuity, and describe any provisions for parking or other access to the site and proposed amenities associated with the access (e.g., boardwalk, benches, trash receptacles, interpretative signage, etc.)\*: There is no proposed activity that will qualify as a water-dependent use as there is no adjacent water within 900' of the Site.

\*If there are no water-dependent use components, describe how the project site is not appropriate for the development of a water-dependent use.

#### Part VIII: Mitigation of Potential Adverse Impacts

Explain how all potential adverse impacts on coastal resources and/or future water-dependent development opportunities and activities identified in Part VII have been avoided, eliminated, or minimized (attach additional pages if necessary):

No adverse impacts were determined on adjacent coastal resources. Stormwater treatment

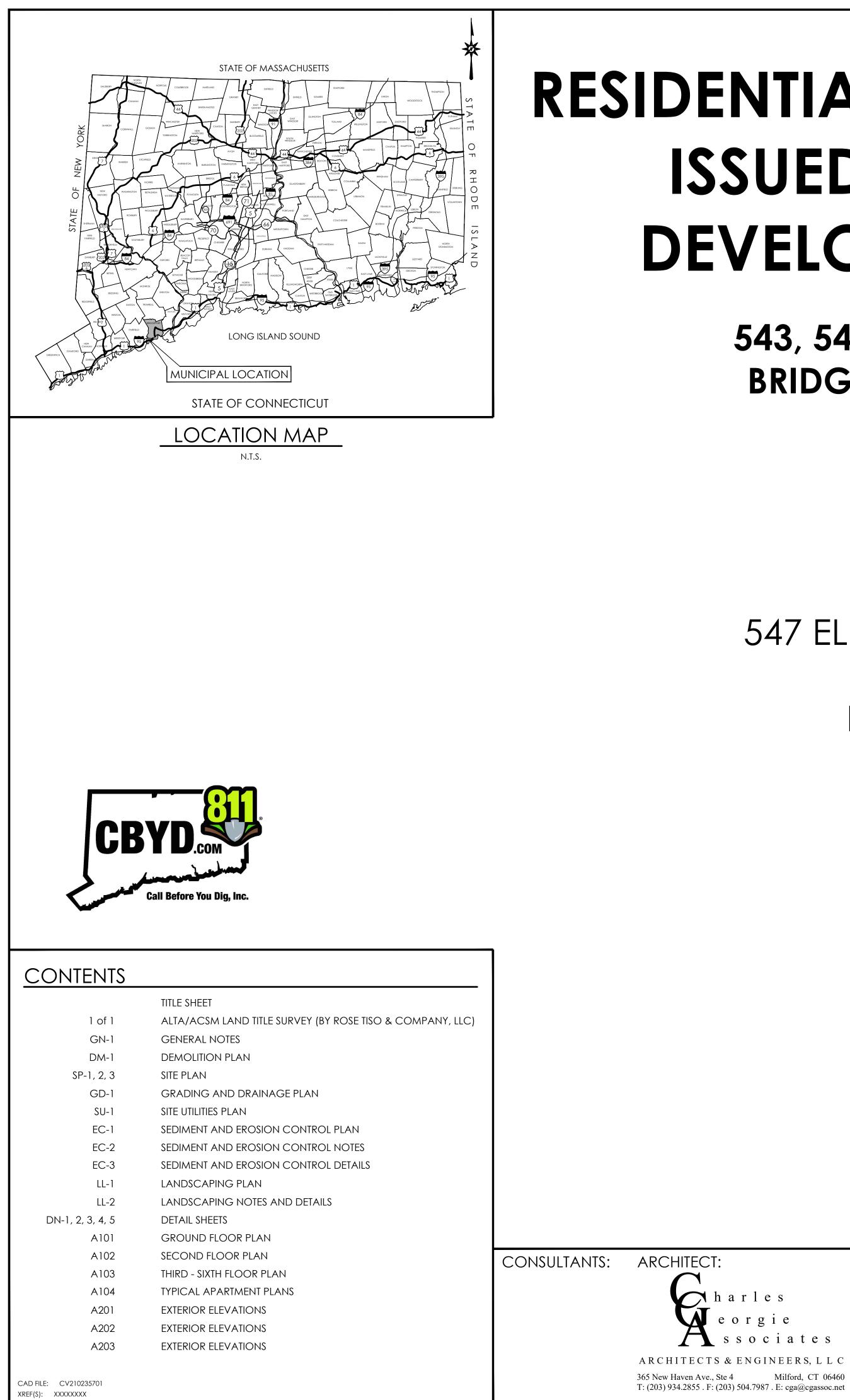
is proposed which will help reduce erosion impacts as well as provide water infiltration. New

lawn areas will also reduce erosion and provide storm water infiltration.

#### Part IX: Remaining Adverse Impacts

Explain why any remaining adverse impacts resulting from the proposed activity or use have not been mitigated and why the project as proposed is consistent with the Connecticut Coastal Management Act (attach additional pages if necessary):

There will be no remaining adverse impacts resulting from the proposed activity.



🗘 2021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

# **RESIDENTIAL DEVELOPMENT PLANS ISSUED FOR LOCAL LAND DEVELOPMENT PERMITTING**

## 543, 547, 549, 557 ELLSWORTH STREET **BRIDGEPORT, CONNECTICUT 06605**

PREPARED FOR: 547 ELLSWORTH NAVCAPMAN, LLC 547 ELLSWORTH STREET BRIDGEPORT, CT 06605

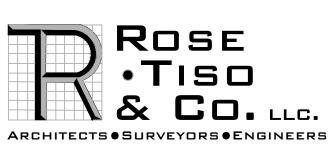
PREPARED BY:



ARCHITECTURE ENGINEERING ENVIRONMENTAL LAND SURVEYING

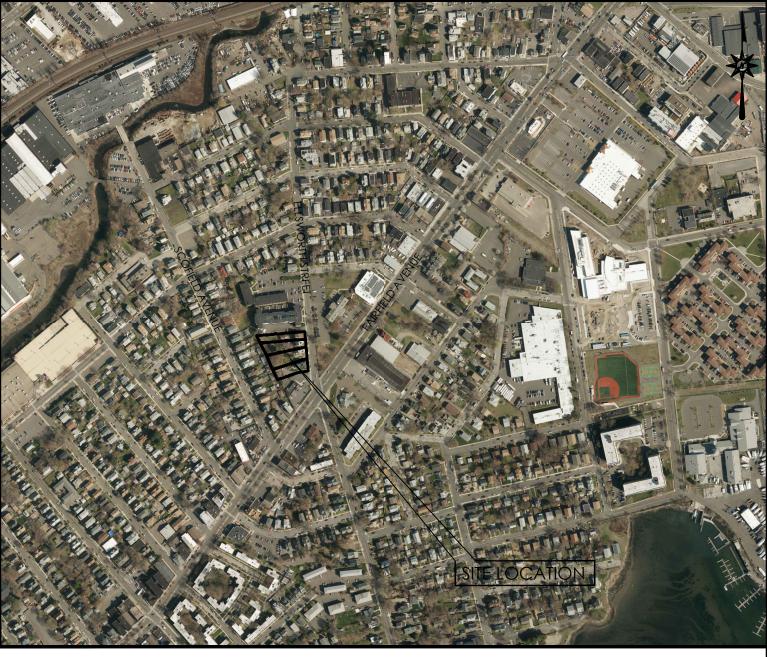
100 CONSTITUTION PLAZA, 10TH FLOOR HARTFORD, CONNECTICUT 06103 (860) 249-2200 (860) 249-2400 Fax

SURVEYOR:



WWW.ROSETISO.COM 35 BRENTWOOD AVENUE, FAIRFIELD, CT 06825 TEL: (203)610-6262 • FAX: (203)610-6404

FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION



VICINITY MAP SCALE: 1''=500

DEVELOPER:

547 ELLSWORTH NAVCAPMAN, LLC 547 ELLSWORTH STREET BRIDGEPORT, CT 06605

OWNER:

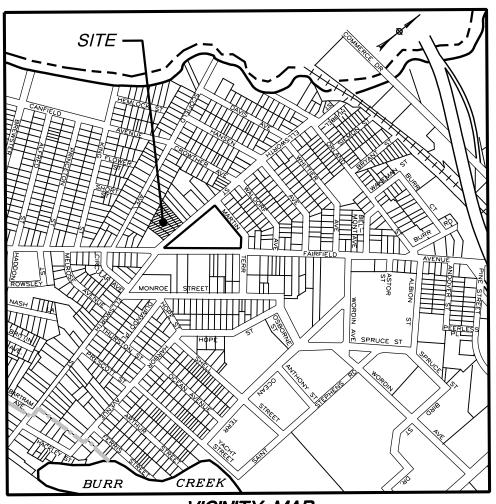
547 ELLSWORTH NAVCAPMAN, LLC 2 ENTERPRISE DRIVE, STE 406 SHELTON, CT 06484



## DATES

ISSUE DATE:

DECEMBER 23, 2021



VICINITY MAP SCALE 1"=800'

#### NOTES:

1. THIS SURVEY HAS BEEN PREPARED IN ACCORDANCE WITH THE REGULATIONS OF CONNECTICUT STATE AGENCIES, SECTIONS 20-300b-1 THROUGH 20-300b-20, "THE MINIMUM STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT", ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. ON SEPT. 26, 1996. THE TYPE OF SURVEY IS A PROPERTY AND TOPOGRAPHIC SURVEY. IT IS A RESURVEY CONFORMING TO CLASS A-2 AND CLASS T-2 ACCURACY STANDARDS.

2. ADDITIONAL PROPERTY CORNER MONUMENTATION NOT SET.

3. ELEVATIONS ARE BASED ON THE CITY OF BRIDGEPORT VERTICAL DATUM. BRIDGEPORT VERTICAL DATUM IS 13.51' = 0 N.G.V.D. 1929.

4. PROPERTY IS SITUATED IN A RESIDENCE "C" ZONE

5. PROPERTY IS SITUATED IN "ZONE X", PER FLOOD INSURANCE RATE MAP, "FAIRFIELD COUNTY, CONNECTICUT, PANEL 436 OF 626, MAP NUMBER 09001C0436G, REVISED JULY 8, 2013" PREPARED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY.

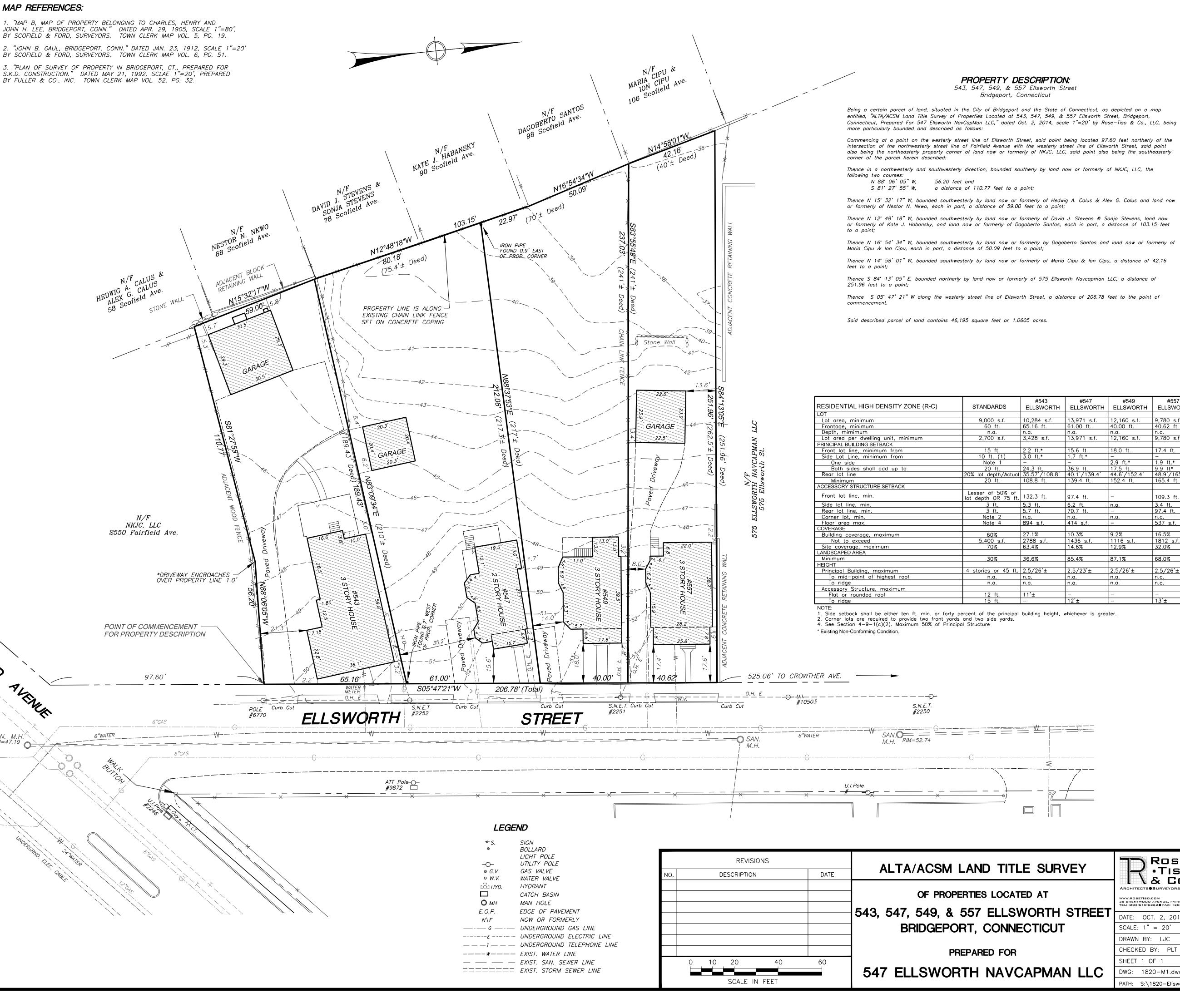
6. UTILITIES DEPICTED HEREON REPRESENT ACTUAL FIELD LOCATIONS OF ALL APPARENT FEATURES (I.E. MANHOLES, CATCH BASINS, GAS VALVES, WATER VALVES, ETC.). THE LOCATIONS OF UNDERGROUND FACILITIES, STRUCTURES, AND UTILITIES HAVE BEEN PLOTTED FROM FIELD OBSERVATIONS WHENEVER POSSIBLE AND BY THE USE OF AVAILABLE SURVEYS AND RECORDS, AND THEREFORE MUST BE CONSIDERED APPROXIMATE ONLY. THERE MAY BE OTHERS, THE EXISTENCE OF WHICH IS PRESENTLY NOT KNOWN. FIELD VERIFICATION IS REQUIRED PRIOR TO CONSTRUCTION.

7. THERE IS NO VISIBLE EVIDENCE OF CEMETERIES OR BURIAL GROUNDS; NO OBSERVED EVIDENCE OF CURRENT EARTH MOVING WORK, BUILDING CONSTRUCTION OR ADDITIONS: THERE ARE NO PROPOSED CHANGES IN STREET RIGHT OF WAY LINES; NO OBSERVED EVIDENCE OF RECENT STREET OR SIDEWALK CONSTRUCTION OR REPAIRS; NO OBSERVED EVIDENCE OF SITE USE AS A SOLID WASTE DUMP, SUMP, OR SANITARY LANDFILL.

#### MAP REFERENCES:

1. "MAP B, MAP OF PROPERTY BELONGING TO CHARLES, HENRY AND JOHN H. LEE, BRIDGEPORT, CONN." DATED APR. 29, 1905, SCALE 1"=80', BY SCOFIELD & FORD, SURVEYORS. TOWN CLERK MAP VOL. 5, PG. 19. 2. "JOHN B. GAUL, BRIDGEPORT, CONN." DATED JAN. 23, 1912, SCALE 1"=20"

3. "PLAN OF SURVEY OF PROPERTY IN BRIDGEPORT, CT., PREPARED FOR S.K.D. CONSTRUCTION." DATED MAY 21, 1992, SCLAE 1"=20', PREPARED BY FULLER & CO., INC. TOWN CLERK MAP VOL. 52, PG. 32.



FAIRFIELD PLINUM SAN. M.H. RIM=47.19

This survey is made for the benefit of: M&T Bank, its sucessors and/or assigns, First American Title Insurance Company and 547 Ellsworth NavCapMan LLC

This is to certify that this map and the survey on which it is based were made in accordance with the 2011 Minimum Standard Detail Requirements for ALTA/ACSM Land Title Surveys jointly established and adopted by ALTA and NSPS and includes Items 2, 3, 4, 6(b), 7(a), 7(b)(1), 7(c), 8, 9, 10(a), 11(a), 13, 14, 16, 17, 18 of Table A thereof. The field work was completed on August 4, 2014 and was updated on September 6, 2014.

NOTE: DRIVEWAY ENCROACHES OVER SOUTHERLY PROPERTY LINE 1.0'

TO MY KNOWLEDGE AND BELIEF THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON.

PHILIP L. TÍSO, L.S. CONN: LIC. No. 12324 NO CERTIFICATION IS EXPRESSED OR IMPLIED UNLESS JHIS MAP BEARS THE SIGNATURE AND THE EMBOSSED SEAL OF THE "ABOVE" NAMED LAND SURVEYOR.

Pl	ROP	ERT	Y	DES	<b>SCRIPTI</b>	ON:
54 <i>3</i> ,	54 <i>7</i> ,	549,	&	557	Ellsworth	Street
	D,	idaan	ort	Con	nontiquit	

Being a certain parcel of land, situated in the City of Bridgeport and the State of Connecticut, as depicted on a map entitled, "ALTA/ACSM Land Title Survey of Properties Located at 543, 547, 549, & 557 Ellsworth Street, Bridgeport, Connecticut, Prepared For 547 Ellsworth NavCapMan LLC," dated Oct. 2, 2014, scale 1"=20' by Rose-Tiso & Co., LLC, being

Commencing at a point on the westerly street line of Ellsworth Street, said point being located 97.60 feet northerly of the intersection of the northwesterly street line of Fairfield Avenue with the westerly street line of Ellsworth Street, said point also being the northeasterly property corner of land now or formerly of NKJC, LLC, said point also being the southeasterly

Thence in a northwesterly and southwesterly direction, bounded southerly by land now or formerly of NKJC, LLC, the

Thence N 15' 32' 17" W, bounded southwesterly by land now or formerly of Hedwig A. Calus & Alex G. Calus and land now

Thence S 05' 47' 21" W along the westerly street line of Ellsworth Street, a distance of 206.78 feet to the point of

		#543	#547	#549	#557
RESIDENTIAL HIGH DENSITY ZONE (R-C)	STANDARDS	ELLSWORTH	ELLSWORTH	ELLSWORTH	ELLSWORTH
LOT					
Lot area, minimum	9,000 s.f.	10,284 s.f.	13,971 s.f.	12,160 s.f.	9,780 s.f.
Frontage, minimum	60 ft.	65.16 ft.	61.00 ft.	40.00 ft.	40.62 ft.
Depth, mimimum	n.a.	n.a.	n.a.	n.a.	n.a.
Lot area per dwelling unit, minimum	2,700 s.f.	3,428 s.f.	13,971 s.f.	12,160 s.f.	9,780 s.f.
PRINCIPAL BUILDING SETBACK					
Front lot line, minimum from	15 ft.	2.2 ft.*	15.6 ft.	18.0 ft.	17.4 ft.
Side Lot Line, minimum from	10 ft. (1)	3.0 ft.*	1.7 ft.*	-	-
One side	Note 1	-	-	2.9 ft.*	1.9 ft.*
Both sides shall add up to	20 ft.	24.3 ft.	36.9 ft.	17.5 ft.	9.9 ft*
Rear lot line	20% lot depth/Actual	35.57'/108.8'	40.1'/139.4'	44.6'/152.4'	48.9'/165.4'
Minimum	20 ft.	108.8 ft.	139.4 ft.	152.4 ft.	165.4 ft.
ACCESSORY STRUCTURE SETBACK					
Front lot line, min.	Lesser of 50% of lot depth OR 75 ft.	132.3 ft.	97.4 ft.	_	109.3 ft.
Side lot line, min.	3 ft.	5.3 ft.	6.2 ft.	n.a.	3.4 ft.
Rear lot line, min.	3 ft.	5.7 ft.	70.7 ft.	-	97.4 ft.
Corner lot, min.	Note 2	n.a.	n.a.	n.a.	n.a.
Floor area max.	Note 4	894 s.f.	414 s.f.	-	537 s.f.
COVERAGE					
Building coverage, maximum	60%	27.1%	10.3%	9.2%	16.5%
Not to exceed	5,400 s.f.	2788 s.f.	1436 s.f.	1116 s.f.	1812 s.f.
Site coverage, maximum	70%	63.4%	14.6%	12.9%	32.0%
LANDSCAPED AREA					
Minimum	30%	36.6%	85.4%	87.1%	68.0%
HEIGHT					
Principal Building, maximum	4 stories or 45 ft.	2.5/26'±	2.5/23'±	2.5/26'±	2.5/26'±
To mid-point of highest roof	n.a.	n.a.	n.a.	n.a.	n.a.
To ridge	n.a.	n.a.	n.a.	n.a.	n.a.
Accessory Structure, maximum					
Flat or rounded roof	12 ft.	11'±	-	-	-
To ridge	15 ft.	-	12'±	-	13'±
NOTE					

1. Side setback shall be either ten ft. min. or forty percent of the principal building height, whichever is greater.

	DATE	ALTA/ACSM LAND TITLE SURVEY	Rose •Tiso & Co
			ARCHITECTS SURVEYORS ENGINEERS
		543, 547, 549, & 557 ELLSWORTH STREET	DATE: OCT. 2, 2014
		BRIDGEPORT, CONNECTICUT	SCALE: 1" = 20' DRAWN BY: LJC
		PREPARED FOR	CHECKED BY: PLT
40	60		SHEET 1 OF 1
		547 ELLSWORTH NAVCAPMAN LLC	DWG: 1820-M1.dwg
<u>E</u> T			PATH: S:\1820-Ellsworth-\dwg

20%STRUCTON SHALL COMPLY WITH THE PROCET SPECIFICATION MANUAL: MUNIPPAL STANDARDS AND SPECIFICATIONS CONNECTICUT RINENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS. 2010 AND STATUSATIONS. AND STATUS BUILDING CODE WITH LARGE REFERENCED SANCH HERARCHY. IF SPECIFICATIONS ARE IN CONFLICT, THE WORK STRINGENT SPECIFICATION SHALL APPLY, ALL CONTRACTOR SHALL WERFY ALL AND BUILDING CONNECS WITH ALL APPLICABLE CSH., HERENAL, STRE MOD LICAL REGULATIONS. TO OTHER DAYS BY OTHER DISCIPLES, DETAILS AND PROJECT MANUAL FOR ADDITIONAL INFORMATION. THE CONTRACTOR SHALL WERFY ALL AND BUILDING CONDITIONS IN THE FIELD AND CONTACT THE CON LEXENCE AND ADDITION CIT IF THERE ARE ANY QUESTIONS OR CONFLICTS BOND THE CONSTRUCTION DOCUMENTS AND APPCILATE DOCUMENTORS, SO THAT APPORPTATE REVISIONS CAN BE MAD PROVED TO BIDDING. ANY LICT BETWEEN THE DISAMINGS AND SPECIFICATIONS SHALL BE CONFIRMED WITH THE OWNER'S CONSTRUCTION MANAGER PRIOR TO BIDDING. ANY LICT BETWEEN THE DISAMING SAND SPECIFICATIONS SHALL BE CONFIRMED WITH THE OWNER'S CONSTRUCTION MANAGER PRIOR TO BIDDING. ANY LICT BETWEEN TESTSING UTILITIES SERVICING FACILITIES OCCUPIED AND USED BY THE OWNER AND THE LOCAL MUNICIPALITIES. INTERRUPTIONS SHALL ONLY R ATTER ACCEPTINGS HAVE EEDS AUTHORIZED IN WRITING BY THE OWNER AND THE LOCAL MUNICIPALITIES. CONTRACTOR TO BECOME DISTINGTOR SHALL ABDIC BY ALL OSAL FEDERAL, STATE, AND LICCAL REGULATIONS MEIN OPERATING CONSERS CONTRACTOR FORCE COMPANY TO AREARCIDENT STOR PROFENS ASSESSION OF ALL CONSTRUCTION (INCLUDING UNDERGROUND UTILITES AND STORMMAYER BY TO THE BOOK OF CONSTRUCTION. REVERTION OF ALL PROVIDE AS-BUILT RECORD DRAWINGS OF ALL CONSTRUCTION (INCLUDING UNDERGROUND UTILITES AND STORMWAYER BY) TO THE OWNER AT THE EDD OF CONSTRUCTION. REVENTOR OF REVERSIBLE FOR STREAMENT AND STATE BEDDING AND TREAMENT AND STREAMENT AND CONTRACTOR SHALL PROVED AS-BUILT RECORD DRAWINGS OF MELANS OF THE WORK, ADD STRE RESPONSIBILITY. CONTRACTOR SHALL DUDY TO CONTRACT BY STEE SATELY MEASURES TO BE EMPLOYED DURING CONSTRUCTION. THE ARCHITEC	<ul> <li>51.</li> <li>52.</li> <li>53.</li> <li>54.</li> <li>55.</li> <li>56.</li> <li>57.</li> <li>58.</li> <li>59.</li> <li>60.</li> <li>61.</li> <li>62.</li> <li>63.</li> </ul>	REMOVE AND DISPOSE OF ANY SIDEWAL APPROVED OFF SITE LANDFILL, BY AN A THE CONTRACTOR SHALL SECURE ALL F THE CONTRACTOR SHALL POST BONDS PERMITS AND DISPOSAL OF ALL BUILDIN ASBESTOS OR HAZARDOUS MATERIAL, II THE CONTRACTOR SHALL CUT AND PLU STREET LINE OR AT THE MAIN, AS REQ SERVICES MAY NOT BE SHOWN ON THIS SERVICE PIPING TO BE REMOVED, CUT O THE CONTRACTOR SHALL PROTECT ALL CONTRACTOR DISTURBED PINS, MONUME OF THE CONTRACTOR SHALL PROTECT ALL CONTRACTOR DISTURBED PINS, MONUME OF THE CONTRACTOR. THE DEMOLITION CONTRACTOR SHALL S OR UNTIL THE COMMENCEMENT OF WOR ENGINEER OR OWNER'S REPRESENTATIVE SEDIMENTATION CONTROLS AND FOR INS PLAN, AT THAT TIME. THE CONTRACTOR SHALL PUMP OUT BL DISPOSAL AREA BY A LICENSED WASTE IF IMPACTED OR CONTAMINATED SOIL IS SOIL AND NOTIFY THE OWNER AND/OR LOCATION UNTIL FURTHER INSTRUCTED EXISTING WATER SERVICES SHALL BE DI PROVIDER. REMOVE EXISTING ONSITE W OR AS REQUIRED BY THE WATER UTILIT EXISTING SANITARY LATERAL SHALL BE SANITARY UTILITY PROVIDER REQUIREME REQUIRED BY THE SANITARY UTILITY PR DOMESTIC GAS SERVICES SHALL BE CAN REQUIRED BY THE SANITARY UTILITY PR DOMESTIC GAS SERVICES SHALL BE CAN REQUIRED BY THE SANITARY UTILITY PR DOMESTIC GAS SERVICES SHALL BE CAN REQUIRED BY THE SANITARY UTILITY PR DOMESTIC GAS SERVICES SHALL BE CAN REQUIRED BY THE SANITARY UTILITY PR DOMESTIC GAS SERVICES SHALL BE CAN REQUIREMENTS. WORK TO BE COORDINA TANKS SHALL BE PURGED OF RESIDUAL THE CONTRACTOR SHALL PROVIDE DISC PROVIDER, GAS UTILITY PROVIDER, ELEC PRIOR TO BEGINNING DEMOLITION.
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R CONNECTION PERMITS AND ROADWAY CONSTRUCTION PERMITS THE CONTRACTOR SHALL POST ALL RONDS PAY ALL FEES PROVIDE PROOF	65.	EQUIPMENT FOR DUST CONTROL. THE CONTRACTOR SHALL REPAIR PAVEN LOCAL GOVERNING AUTHORITIES AND TH
ISURANCE AND PROVIDE TRAFFIC CONTROL NECESSARY FOR THIS WORK EXCEPT CTDOT ENCROACHMENT PERMIT BOND.	66.	THE CONTRACTOR SHALL CUT AND REM CONTRACTOR SHALL REMOVE ALL CABL
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CONTRACTOR SHALL REFERENCE ARCHITECTURAL PLANS FOR EXACT DIMENSIONS AND CONSTRUCTION DETAILS OF BUILDING, AND THE RAISED RETE SIDEWALKS, LANDINGS, RAMPS, AND STAIRS.	68.	THE CONTRACTOR SHALL HAVE CALL B
LD ANY UNCHARTED OR INCORRECTLY CHARTED, EXISTING PIPING OR OTHER UTILITY BE UNCOVERED DURING EXCAVATION, CONSULT THE CIVIL IEER IMMEDIATELY FOR DIRECTIONS BEFORE PROCEEDING FURTHER WITH WORK IN THIS AREA.		REQUIRED. MAINTAIN UTILITY SERVICES INSTALLATION AND PAY UTILITY PROVID THE CONTRACTOR SHALL NOT COMMENC
SITE DIMENSIONS ARE REFERENCED TO THE FACE OF CURBS OR EDGE OF PAVING AS APPLICABLE UNLESS OTHERWISE NOTED. ALL BUILDING ISIONS ARE REFERENCED TO THE OUTSIDE FACE OF THE STRUCTURE. CONTRACTOR SHALL PROVIDE AND MAINTAIN TRAFFIC DEVICES FOR PROTECTION OF VEHICLES AND PEDESTRIANS CONSISTING OF DRUMS,		THE CONTRACTOR OR DEMOLITION CONT BUILDINGS, STRUCTURES AND UTILITIES
IERS, SIGNS, LIGHTS, FENCES, TEMPORARY WALKWAYS, TRAFFIC CONTROLLERS AND UNIFORMED TRAFFIC OFFICERS AS REQUIRED OR AS ORDERED HE ENGINEER OR AS REQUIRED BY THE LOCAL GOVERNING AUTHORITIES OR AS REQUIRED BY PERMIT STIPULATIONS OR AS REQUIRED BY THE IR. CONTRACTOR SHALL MAINTAIN ALL TRAFFIC LANES AND PEDESTRIAN WALKWAYS FOR USE AT ALL TIMES UNLESS WRITTEN APPROVAL FROM		ENGINEER, LICENSED IN THIS STATE AN NO SALVAGE SHALL BE PERMITTED UNL ANY EXISTING POTABLE WELL AND ANY
ES. SIGNS SHALL BE INSTALLED PLUMB WITH THE EDGE OF THE SIGN 2' OFF THE FACE OF THE CURB, AND WITH 7' VERTICAL CLEARANCE		REQUIREMENTS. NOTE HAS BEEN PURPOSELY OMITTED.
R TO DETAIL SHEETS FOR PAVEMENT, CURBING, AND SIDEWALK INFORMATION.	74.	THE CONTRACTOR SHALL PRESERVE EXI CONTROL PLAN FOR LIMIT OF DISTURBA
		TOPSOIL SHALL BE STRIPPED AND STOC FILL WITHIN FORMER BUILDING FOUNDAT
	70.	GEOTECHNICAL ENGINEER. SUBGRADE SI UNSUITABLE AND SOFT SUBGRADE MAT SPECIFICATIONS FOR FURTHER DESCRIP
/L 4" SOLID YELLOW LINE VL 4" SOLID WHITE LINE	77.	THE CONTRACTOR SHALL COMPACT FILL AREAS TO 95% OF THE MAXIMUM DRY
VL 4" BROKEN WHITE LINE 10' STRIPE 30' SPACE ING SPACES SHALL BE STRIPED WITH 4" SWL; HATCHED AREA SHALL BE STRIPED WITH 4" SWL AT A 45° ANGLE, 2' ON CENTER. HATCHING,	78.	ENGINEER. UNDERDRAINS SHALL BE ADDED, IF DET GRADED.
	79.	VERTICAL DATUM IS CITY OF BRIDGEPOR
		CLEARING LIMITS SHALL BE PHYSICALLY THE SITE.
S, SWALE, PAVEMENT MARKINGS, OR SIGNAGE DISTURBED DURING DEMOLITION AND/OR CONSTRUCTION TO THEIR ORIGINAL CONDITION OR BETTER, PPROVED BY THE CIVIL ENGINEER, AND TO THE SATISFACTION OF THE OWNER AND REVIEWING AGENCY.	81.	PROPER CONSTRUCTION PROCEDURES S WATERCOURSE OR WETLANDS IN ACCOR AND SEDIMENT CONTROL, LATEST EDITIO CONTAINED HEREIN. THE CONTRACTOR S CONSERVATION DISTRICT WHICH WOULD
E 1" = 20', DATED 10/2/2014, BY ROSE TISO & CO. LLC.	82.	ALL SITE WORK, MATERIALS OF CONSTR THE SPECIFICATIONS AND DETAILS AND THE STATE DEPARTMENT OF TRANSPOR
APPROPRIATE REGULATORY AGENCY PRIOR TO INSTALLATION DURING THE BIDDING PROCESS.		MANUAL. ALL FILL MATERIAL UNDER ST PROJECT GEOTECHNICAL REPORT, AND QUALIFIED PROFESSIONAL ENGINEER. MA
rosion control bond is required to be posted by the contractor before the start of any activity on or off site. The	83.	THE MAXIMUM DRY DENSITY AS DETERM ALL DISTURBANCE INCURRED TO MUNICI
	84.	CONDITION OR BETTER, TO THE SATISFA
	85.	THE UTILITY PLAN DETAILS SITE INSTAL CONNECTIONS. SITE CONTRACTOR SHAI UTILITY OR PIPE CONNECTION POINT.
	86.	THE CONTRACTOR SHALL VISIT THE SITI EXCAVATION. TEST PITS SHALL BE DUG
		Existing utilities, and the horizonta Civil Engineer in the event of any and utilities so that an appropria
RDING TO CONNDOT SPECIFICATIONS.		UTILITY CONNECTION DESIGN AS REFLECT THE CONTRACTOR SHALL ENSURE THAT
CONTRACTOR SHALL REMOVE CONFLICTING PAVEMENT MARKINGS IN THE ROADWAY BY METHOD APPROVED BY THE AUTHORITY HAVING		METHODS ARE MET. THE CONTRACTOR STHE CONTRACTOR SHALL ARRANGE FOR
ada designated parking stalls, access aisles and pedestrian walkways shall conform to the current version of the americans	90	THE CONTRACTOR SHALL COORDINATE N DISCONNECTIONS, RELOCATIONS, INSPEC GENERAL CONDITIONS OF THE CONTRAC ALL EXISTING PAVEMENT WHERE UTILITY
ATIONS, AND CHAPTER 16 OF NFPA 1 UNIFORM FIRE CODE.	50.	CONTRACTOR SHALL INSTALL TEMPORAF HAVING JURISDICTION.
THE SITE. CONTRACTOR SHALL SECURE ANY PERMITS, PAY ALL FEES AND PERFORM CLEARING AND GRUBBING AND DEBRIS REMOVAL PRIOR TO	91. 92.	
	<ul> <li>BES, STASS, LOHES, FENCES, TEARORARY WILLING AND STREPPE CONTROLLERS AND UNFORMED TRAFFIC OFFICES AS REQUERED BY THE ENCORE OF A SERVICE DE YEAR CONTROL ESS ARCHIVED BY THE CONTROL SEANCES WALL MARKAT ALL TRAFFIC LARGE AND HEAD CONTROL AND ARCHIVE AND AND PEDESTINAL WALKWAYS FOR USE AT ALL THES UNLESS WRITEN APPROVAL FROM APPROPRIATE CONTROL. SEANCES SERVICES AND THE MANUAL OF AN EXCURRED BY THE PERFORME. DEVICES ARCHIVES AND TO THE STATE DOT STANDARD DETAIL SKETS AND THE MANUAL OF UNFORM THATPE CONTROL. SESS OTHERWISE DETAILED OR NOTED.</li> <li>RT DO DETAIL SHETS FOR PANEMENT, CURBING, AND SIGEMALL INFORMATION.</li> <li>CONTRACT UNIT DE INFLAUED DEVICES OTHERWISE SPECIFED OR SHOWN ON THE CONTRACT DRAWINGS.</li> <li>CONTRACT UNIT DE PROPERTY UNE UNLESS OTHERWISE SPECIFED OR SHOWN ON THE CONTRACT DRAWINGS.</li> <li>CONTRACT UNIT DE PROPERTY UNE UNLESS OTHERWISE SPECIFED OR SHOWN ON THE CONTRACT DRAWINGS.</li> <li>CONTRACT UNIT DE PROPERTY UNE UNLESS OTHERWISE SPECIFED OR SHOWN ON THE CONTRACT DRAWINGS.</li> <li>CONTRACT UNIT DE STREP SO SPACE</li> <li>KL * SOUD VILLOW UNER</li> <li>KL * SOU</li></ul>	<ul> <li>BERS, BIARS, LIGHTS, FENES, LIGHTGRAFY WALKWAYS, TRAFE CONTROLLERS AND UNFORMED TRAFFC OFFICES AS REQUIRED DY PARE THE ENDANCE OF A REQUIRED DY THE ENDANCE ON AS REQUIRED DY PARE THE ADDRESS AND REPORT OF AS REQUIRED DY PARE THE ADDRESS AND PEDESTRIAL MARKING STARU VERY AND REAL MARKS TO UNFORM TRAFFC CONTROL.</li> <li>TE CORTROL SIGNAGE SHALL CONFERM TO THE STATE OT STARADOD DETAIL SHETS AND THE ANNULL OF UNFORM TRAFFC CONTROL.</li> <li>SS ONGERED BY THE BY THE DE DOES OF THE STAR 20° OF THE STAR 20° OF THE CARE, AND THE ANNULL OF UNFORM TRAFFC CONTROL.</li> <li>SS ONGERED BY THE WITCH DE UNESS OTHERNASE SPECIFIED OR SHOWN ON THE CONTROL OLDARONCE.</li> <li>CONTRACT LINT IS THE PROPERTY LINE UNLESS OTHERNASE SPECIFIED OR SHOWN ON THE CONTROL OLDARONCE.</li> <li>CONTRACT LINT IS THE PROPERTY LINE UNLESS OTHERNASE SPECIFIED OR SHOWN ON THE CONTROL TO STRIPING.</li> <li>CONTRACT LINT IS THE PROPERTY LINE UNLESS OTHERNASE SPECIFIED OR SHOWN ON THE CONTROL TO STRIPING.</li> <li>CONTRACT LINT IS THE PROPERTY LINE UNLESS OTHERNASE SPECIFIED OR SHOWN ON THE CONTROL TO STRIPING.</li> <li>CONTRACT LINT IS THE PROPERTY LINE UNLESS OTHERNASE SPECIFIED OR SHOWN ON THE CONTROL TO STRIPING.</li> <li>CONTRACT LINT IS THE PROPERTY LINE UNLESS OTHER AND STALL BE STRIPING TO STRIPING.</li> <li>CONTRACT DE SHALL SUBMIT A SHOP DRAWING OF THE PAXEMENT MARKING PAINT MIXTURE PRIOR TO STRIPING.</li> <li>CONTRACT SHALL SUBMIT A SHOP DRAWING AND THE DATE DOT SPECIFICATIONS, UNLESS WHERE MATCHING AND THE OR AS NOTED.</li> <li>CONTRACT SHALL DE STRIPING ON THADOLOGIES SHALL BE PARTED WITH ON THE OR AS NOTED.</li> <li>CONTRACT SHALL DE STRIPING ON THADOLOGIES SHALL BE PARTED WITH ON AND/OR CONSTRUCTION TO THERE OR AND THE AND/ON OR ADDRESS SHALL BE ANTED WITH ON AND/OR CONSTRUCTION.</li> <li>CONTRACT SHALL SUBMIT AS SHALL BE AND THE DATE DOT SPECIFICATIONS, UNLESS WHERE EPOXY RESIN PAXEMENT AND/OR OR ADDRESS AND HATCHE ANTED WITH ON AND/OR CONSTRUCTION.</li> <li>CONTRACTOR SHALL RESTRE ANY UTILTY STRUCTURE, DRAMAGE STRUCTURE, PREV</li></ul>

021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

- TROLS AS SHOWN ON THE SEDIMENT AND EROSION CONTROL PLAN AND/OR DEMOLITION PLAN SHALL BE INSTALLED BY PRIOR TO START OF DEMOLITION AND CLEARING AND GRUBBING OPERATIONS.
- SIDEWALKS, FENCES, STAIRS, WALLS, DEBRIS AND RUBBISH REQUIRING REMOVAL FROM THE WORK AREA IN AN BY AN APPROVED HAULER. HAULER SHALL COMPLY WITH ALL REGULATORY REQUIREMENTS.
- URE ALL PERMITS FOR HIS DEMOLITION AND DISPOSAL OF HIS DEMOLITION MATERIAL TO BE REMOVED FROM THE SITE.
- BONDS AND PAY PERMIT FEES AS REQUIRED. BUILDING DEMOLITION CONTRACTOR SHALL BE RESPONSIBLE FOR LL BUILDING DEMOLITION DEBRIS IN AN APPROVED OFF-SITE LANDFILL.
- IATERIAL, IF FOUND ON SITE, SHALL BE REMOVED BY A LICENSED HAZARDOUS MATERIAL ABATEMENT CONTRACTOR. PARE ALL MANIFEST DOCUMENTS AS REQUIRED PRIOR TO COMMENCEMENT OF DEMOLITION.
- AND PLUG. OR ARRANGE FOR THE APPROPRIATE UTILITY PROVIDER TO CUT AND PLUG ALL SERVICE PIPING AT THE , AS REQUIRED BY THE UTILITY PROVIDER, OR AS OTHERWISE NOTED OR SHOWN ON THE CONTRACT DRAWINGS. ALL ON THIS PLAN. THE CONTRACTOR SHALL INVESTIGATE THE SITE PRIOR TO BIDDING TO DETERMINE THE EXTENT OF /ED, CUT OR PLUGGED. THE CONTRACTOR SHALL PAY ALL UTILITY PROVIDER FEES FOR ABANDONMENTS AND REMOVALS.
- TECT ALL IRON PINS, MONUMENTS AND PROPERTY CORNERS DURING DEMOLITION AND CONSTRUCTION ACTIVITIES. ANY MONUMENTS, AND OR PROPERTY CORNERS, ETC. SHALL BE RESET BY A LICENSED LAND SURVEYOR AT THE EXPENSE
- SHALL STABILIZE THE SITE AND KEEP EROSION CONTROL MEASURES IN PLACE UNTIL THE COMPLETION OF HIS WORK T OF WORK BY THE SITE CONTRACTOR, WHICHEVER OCCURS FIRST, AS REQUIRED OR DEEMED NECESSARY BY THE SENTATIVE. THE SITE CONTRACTOR SHALL ASSUME RESPONSIBILITY FOR THE MAINTENANCE OF EXISTING EROSION AND ID FOR INSTALLATION OF ANY NEW SEDIMENT AND EROSION CONTROLS AS PER THE SEDIMENT AND EROSION CONTROL
- IP OUT BUILDING FUEL AND WASTE OIL TANKS (IF ANY ARE ENCOUNTERED) AND REMOVE FUEL TO AN APPROVED SED WASTE OIL HANDLING CONTRACTOR IN STRICT ACCORDANCE WITH STATE REQUIREMENTS.
- ED SOIL IS ENCOUNTERED BY THE CONTRACTOR, THE CONTRACTOR SHALL SUSPEND EXCAVATION WORK OF IMPACTED AND/OR OWNER'S ENVIRONMENTAL CONSULTANT PRIOR TO PROCEEDING WITH FURTHER WORK IN THE IMPACTED SOIL TRUCTED BY THE OWNER AND/OR OWNER'S ENVIRONMENTAL CONSULTANT.
- HALL BE DISCONNECTED AND CAPPED AT MAIN IN ACCORDANCE WITH THE REQUIREMENTS OF THE WATER UTILITY ONSITE WATER PIPING TO BE ABANDONED TO RIGHT OF WAY LINE UNLESS OTHERWISE SHOWN ON DEMOLITION PLANS TER UTILITY PROVIDER TO BE REMOVED TO MAIN.
- SHALL BE PLUGGED WITH NON-SHRINK GROUT AT CURB LINE OR AT MAIN CONNECTION IN ACCORDANCE WITH THE REQUIREMENTS. REMOVE EXISTING LATERAL PIPING FROM SITE UNLESS OTHERWISE SHOWN ON DEMOLITION PLANS OR AS ITILITY PROVIDER.
- LL BE CAPPED AND SERVICE LINES PURGED OF RESIDUAL GAS IN ACCORDANCE WITH THE GAS UTILITY PROVIDER COORDINATED BY AND PAID FOR BY THE CONTRACTOR. REMOVE EXISTING SERVICE PIPING ON SITE. ANY PROPANE RESIDUAL GAS BY PROPANE SUPPLIER. CONTRACTOR SHALL COORDINATE THIS WORK AND PAY NECESSARY FEES.
- DVIDE DISCONNECT NOTIFICATION TO THE MUNICIPALITY ENGINEERING DEPARTMENT. TELECOMMUNICATIONS UTILITY IDER, ELECTRIC UTILITY PROVIDER, SANITARY UTILITY PROVIDER, AND WATER UTILITY PROVIDER AT LEAST THREE WEEKS
- ISIBLE FOR SECURING A DEMOLITION PERMIT FROM THE CITY OF BRIDGEPORT BUILDING DEPARTMENT AND MUST FURNISH MATERIAL AND PAY ALL FEES.
- INDATION HOLES AND REMOVED DRIVEWAY AREAS IN LOCATIONS NOT SUBJECT TO FURTHER EXCAVATION WITH SOIL OWNER'S GEOTECHNICAL ENGINEER AND COMPACT, FERTILIZE, SEED AND MULCH DISTURBED AREAS NOT SUBJECT TO DEMOLISHED BUILDING FOUNDATION AREA AND BASEMENT IF PRESENT TO BE BACKFILLED WITH GRAVEL FILL OR PROJECT GEOTECHNICAL REPORT IN LIFT THICKNESS SPECIFIED IN THE GEOTECHNICAL REPORT. COMPACT TO 95% MAX. 7 AT MOISTURE CONTENT SPECIFIED IN GEOTECHNICAL REPORT AND EARTHWORK SPECIFICATION. EMPLOY WATERING
- AIR PAVEMENTS BY INSTALLING TEMPORARY AND PERMANENT PAVEMENTS IN PUBLIC RIGHTS OF WAYS AS REQUIRED BY ES AND THE CTDOT AND PER PERMIT REQUIREMENTS DUE TO DEMOLITION AND PIPE REMOVAL ACTIVITIES.
- AND REMOVE AT LUMINARE AND SIGN LOCATIONS ANY PROTRUDING CONDUITS TO 24" BELOW GRADE. THE ALL CABLE AND CONDUCTORS FROM REMAINING LIGHTING AND SIGNING CONDUITS TO BE ABANDONED. ANY REMAINING SHALL BE RECIRCUITED OR REWIRED AS NECESSARY TO REMAIN IN OPERATION.
- BE INITIATED BY THE CONTRACTOR UNTIL A PRE-CONSTRUCTION MEETING WITH OWNER AND THE CIVIL ENGINEER IS SHOULD BE AWARE OF ANY SITE INFORMATION AVAILABLE SUCH AS GEOTECHNICAL AND ENVIRONMENTAL REPORTS. CALL BEFORE YOU DIG MARK OUTS OF EXISTING UTILITIES COMPLETED PRIOR TO MEETING.
- ANGE FOR AND INSTALL TEMPORARY OR PERMANENT UTILITY CONNECTIONS WHERE INDICATED ON PLAN OR AS SERVICES TO BUILDINGS OR TO SERVICES TO REMAIN. CONTRACTOR TO COORDINATE WITH UTILITY PROVIDERS FOR TY PROVIDER FEES.
- COMMENCE DEMOLITION OR UTILITY DISCONNECTIONS UNTIL AUTHORIZED TO DO SO BY THE OWNER.
- TION CONTRACTOR SHALL INSTALL TEMPORARY SHEETING OR SHORING AS NECESSARY TO PROTECT EXISTING AND NEW UTILITIES DURING CONSTRUCTION AND DEMOLITION. SHEETING OR SHORING SHALL BE DESIGNED BY A PROFESSIONAL TATE AND EVIDENCE OF SUCH SUBMITTED TO THE OWNER PRIOR TO INSTALLATION.
- AITTED UNLESS PAID TO THE OWNER AS A CREDIT.
- AND ANY EXISTING SEPTIC TANKS/ABSORPTION AREAS SHALL BE ABANDONED AND REMOVED PER THE HEALTH CODE
- ESERVE EXISTING VEGETATION WHERE POSSIBLE AND/OR AS NOTED ON DRAWINGS. REFER TO SEDIMENT AND EROSION DISTURBANCE AND EROSION CONTROL NOTES.
- AND STOCKPILED ON SITE FOR USE IN FINAL LANDSCAPING.
- FOUNDATION SHALL BE CHECKED BY TEST PIT AND PROOF-ROLLING AND SHALL BE OBSERVED BY THE OWNER'S 3GRADE SHALL BE FORMED WITH REMOVAL AND REPLACEMENT OF FILL AND REMOVAL AND REPLACEMENT OF RADE MATERIAL AS REQUIRED BY THE GEOTECHNICAL ENGINEER. SEE GEOTECHNICAL REPORT AND EARTHWORK DESCRIPTION.
- IPACT FILL IN LIFT THICKNESS PER THE GEOTECHNICAL REPORT UNDER ALL PARKING, BUILDING, DRIVE, AND STRUCTURE MUM DRY DENSITY AS DETERMINED BY ASTM D1557 (MODIFIED PROCTOR TEST), OR AS REQUIRED BY THE GEOTECHNICAL
- DED, IF DETERMINED NECESSARY IN THE FIELD BY THE OWNER/GEOTECHNICAL ENGINEER, AFTER SUBGRADE IS ROUGH
- BRIDGEPORT VERTICAL DATUM.
- HYSICALLY MARKED IN THE FIELD AND APPROVED BY THE REVIEWING AGENCY AGENT PRIOR TO THE START OF WORK ON
- CEDURES SHALL BE FOLLOWED ON ALL IMPROVEMENTS WITHIN THIS PARCEL SO AS TO PREVENT THE SILTING OF ANY IN ACCORDANCE WITH THE REGULATIONS OF THE CT DEEP AND THE 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION EST EDITION. IN ADDITION. THE CONTRACTOR SHALL STRICTLY ADHERE TO THE SEDIMENT AND EROSION CONTROL PLAN RACTOR SHALL BE RESPONSIBLE TO POST ALL BONDS AS REQUIRED BY THE LOCAL MUNICIPALITIES, OR SOIL H WOULD GUARANTEE THE PROPER IMPLEMENTATION OF THE PLAN.
- CONSTRUCTION, AND CONSTRUCTION METHODS FOR EARTHWORK AND STORM DRAINAGE WORK SHALL CONFORM TO TAILS AND APPLICABLE SECTIONS OF THE PROJECT SPECIFICATIONS MANUAL. OTHERWISE THIS WORK SHALL CONFORM TO TRANSPORTATION SPECIFICATIONS AND PROJECT GEOTECHNICAL REPORT IF THERE IS NO PROJECT SPECIFICATIONS UNDER STRUCTURES AND PAVED AREAS SHALL BE PER THE ABOVE STATED APPLICABLE SPECIFICATIONS, AND/OR DRT. AND SHALL BE PLACED IN ACCORDANCE WITH THE APPLICABLE SPECIFICATIONS UNDER THE SUPERVISION OF A INEER. MATERIAL SHALL BE COMPACTED IN LIFT THICKNESSES PER THE PROJECT GEOTECHNICAL REPORT TO 95% OF AS DETERMINED BY ASTM D 1557 AT MOISTURE CONTENT INDICATED IN PROJECT GEOTECHNICAL REPORT.
- TO MUNICIPAL, COUNTY, AND STATE PROPERTY DUE TO CONSTRUCTION SHALL BE RESTORED TO ITS PREVIOUS SATISFACTION OF THE MUNICIPALITY, COUNTY AND STATE AS APPLICABLE FOR THE LOCATION OF THE WORK. DOT RIGHT OF WAY SHALL COMPLY WITH ALL DEPARTMENT OF TRANSPORTATION STANDARDS AND SPECIFICATIONS.
- TE INSTALLED PIPES UP TO 5' FROM THE BUILDING FACE. REFER TO DRAWINGS BY OTHERS FOR BUILDING CTOR SHALL SUPPLY AND INSTALL PIPE ADAPTERS AS NECESSARY AT BUILDING CONNECTION POINT OR AT EXISTING
- THE SITE AND VERIFY THE ELEVATION AND LOCATION OF ALL UTILITIES BY VARIOUS MEANS PRIOR TO BEGINNING ANY BE DUG AT ALL LOCATIONS WHERE PROPOSED SANITARY SEWERS AND WHERE PROPOSED STORM PIPING WILL CROSS. HORIZONTAL AND VERTICAL LOCATIONS OF THE UTILITIES SHALL BE DETERMINED. THE CONTRACTOR SHALL CONTACT THE OF ANY DISCOVERED OR UNFORESEEN CONFLICTS BETWEEN EXISTING AND PROPOSED SANITARY SEWERS, STORM PIPING PPROPRIATE MODIFICATION MAY BE MADE.
- AS REFLECTED ON THE PLAN MAY CHANGE SUBJECT TO UTILITY PROVIDER AND GOVERNING AUTHORITY STAFF REVIEW. URE THAT ALL UTILITY PROVIDERS AND GOVERNING AUTHORITY STANDARDS FOR MATERIALS AND CONSTRUCTION TRACTOR SHALL PERFORM PROPER COORDINATION WITH THE RESPECTIVE UTILITY PROVIDER.
- ANGE FOR AND COORDINATE WITH THE RESPECTIVE UTILITY PROVIDERS FOR SERVICE INSTALLATIONS AND CONNECTIONS. ORDINATE WORK TO BE PERFORMED BY THE VARIOUS UTILITY PROVIDERS AND SHALL PAY ALL FEES FOR CONNECTIONS, S, INSPECTIONS, AND DEMOLITION UNLESS OTHERWISE STATED IN THE PROJECT SPECIFICATIONS MANUAL AND/OR CONTRACT.
- TRE UTILITY PIPING IS TO BE INSTALLED SHALL BE SAW CUT. AFTER UTILITY INSTALLATION IS COMPLETED, THE TEMPORARY AND/OR PERMANENT PAVEMENT REPAIR AS DETAILED ON THE DRAWINGS OR AS REQUIRED BY THE OWNER
- STRAIGHT ALIGNMENTS AND EVEN GRADES USING A PIPE LASER OR OTHER ACCURATE METHOD.
- AINTAIN (10' MIN. HORIZONTAL 1.5' VERTICAL MIN.) SEPARATION DISTANCE FROM WATER LINES, OR ADDITIONAL be required where permitted, which shall include concrete encasement of piping unless otherwise

- DIRECTED BY THE UTILITY PROVIDERS AND CIVIL ENGINEER. 93. RELOCATION OF UTILITY PROVIDER FACILITIES SHALL BE DONE IN ACCORDANCE WITH THE REQUIREMENTS OF THE UTILITY
- 94. THE CONTRACTOR SHALL COMPACT THE PIPE BACKFILL IN 8" LIFTS ACCORDING TO THE PIPE BEDDING DETAILS. TRENCH IN HIGH GROUNDWATER AREAS. A PIPE FOUNDATION SHALL BE USED PER THE TRENCH DETAILS AND IN AREAS OF ROCK
- 95. CONTRACTOR TO PROVIDE STEEL SLEEVES AND ANNULAR SPACE SAND FILL FOR UTILITY PIPE AND CONDUIT CONNECTION
- 96. BUILDING UTILITY PENETRATIONS AND LOCATIONS ARE SHOWN FOR THE CONTRACTOR'S INFORMATION AND SHALL BE VERI MEP, STRUCTURAL, AND ARCHITECTURAL DRAWINGS AND WITH THE OWNER'S CONSTRUCTION MANAGER.
- 97. ALL UTILITY CONSTRUCTION IS SUBJECT TO INSPECTION FOR APPROVAL PRIOR TO BACKFILLING, IN ACCORDANCE WITH THI PROVIDER REQUIREMENTS.
- 98. A ONE-FOOT MINIMUM VERTICAL CLEARANCE BETWEEN WATER, GAS, ELECTRICAL, AND TELEPHONE LINES AND STORM PIP SIX-INCH MINIMUM CLEARANCE SHALL BE MAINTAINED BETWEEN STORM PIPING AND SANITARY SEWER WITH A CONCRETE TO 6-INCH VERTICAL CLEARANCE BETWEEN SANITARY SEWER PIPING AND STORM PIPING SHALL REQUIRE CONCRETE ENC/
- 99. GRAVITY SANITARY SEWER PIPING AND PRESSURIZED WATERLINES SHALL BE LOCATED IN SEPARATE TRENCHES AT LEAST POSSIBLE. WHEN INSTALLED IN THE SAME TRENCH, THE WATER PIPE SHALL BE LAID ON A TRENCH BENCH AT LEAST 18 THE SANITARY SEWER PIPE AND AT LEAST 12 INCHES (PREFERABLY 18 INCHES) FROM THE SIDE OF THE SANITARY SEWEI
- 100. SITE CONTRACTOR SHALL PROVIDE ALL BENDS, FITTINGS, ADAPTERS, ETC., AS REQUIRED FOR PIPE CONNECTIONS TO BUIL ROOF/FOOTING DRAIN CONNECTIONS TO ROOF LEADERS AND TO STORM DRAINAGE SYSTEM.
- 101. MANHOLE RIMS AND CATCH BASIN GRATES SHALL BE SET TO ELEVATIONS SHOWN. SET ALL EXISTING MANHOLE RIMS ANI RAISED OR LOWERED FLUSH WITH FINAL GRADE AS NECESSARY. 102. SITE CONTRACTOR SHALL COORDINATE INSTALLATION OF CONDUIT AND CABLES FOR SITE LIGHTING WITH THE BUILDING EL
- 103. CONTRACTOR SHALL COORDINATE INSTALLATION FOR ELECTRICAL SERVICES TO PYLON SIGNS AND SITE LIGHTING WITH THE CONTRACTOR.
- 104. THE CONTRACTOR SHALL ARRANGE AND COORDINATE WITH UTILITY PROVIDERS FOR WORK TO BE PERFORMED BY UTILITY CONTRACTOR SHALL PAY ALL UTILITY FEES UNLESS OTHERWISE STATED IN THE PROJECT SPECIFICATION MANUAL AND GEN REPAIR PAVEMENTS AS NECESSARY.
- 105. ELECTRIC, AND TELECOMMUNICATIONS SERVICES SHALL BE INSTALLED UNDERGROUND FROM SERVICE POLE #2251. THE CON AND INSTALL AND BACKFILL PVC CONDUITS FOR TELECOMMUNICATIONS SERVICE, PVC CONDUITS FOR ELECTRIC SERVICE F ELECTRICAL SECONDARY PER BUILDING ELECTRICAL PLANS, (SCHEDULE 80 UNDER PAVEMENT, SCHEDULE 40 IN NON PAVE MAY BE INSTALLED IN A COMMON TRENCH WITH 12" CLEAR SPACE BETWEEN, MINIMUM COVER IS 36" ON ELECTRIC COND TELECOMMUNICATIONS CONDUITS. SERVICES SHALL BE MARKED WITH MAGNETIC LOCATOR TAPE AND SHALL BE BEDDED, II IN ACCORDANCE WITH ELECTRIC UTILITY PROVIDER. AND TELECOMMUNICATIONS COMPANY STANDARDS. GALVANIZED STEEL BE USED AT POLE AND TRANSFORMER LOCATIONS. INSTALL HANDHOLES AS REQUIRED TO FACILITATE INSTALLATION AND PROVIDER. INSTALL TRAFFIC LOAD QUALIFIED HANDHOLES IN VEHICULAR AREAS. INSTALL CONCRETE ENCASEMENT ON PR REQUIRED BY ELECTRIC UTILITY PROVIDER.
- 106. ALL WATER LINES TO HAVE A MINIMUM COVER OF 4-1/2 FEET. ALL LINES SHALL BE BEDDED IN 6" SAND AND INITIALLY BACKFILLED WITH 12" SAND.
- 107. ALL WATER MAINS, WATER SERVICES AND SANITARY SEWER LATERALS SHALL CONFORM TO THE APPLICABLE WATER UTILITY PROVIDER SPECIFICATIONS, AND TO THE APPLICABLE SANITARY SEWER PROVIDER SPECIFICATIONS, AS WELL AS TO OTHER APPLICABLE INDUSTRY CODES (AWWA) AND PROJECT SPECIFICATIONS FOR POTABLE WATER SYSTEMS, AND FOR SANITARY SEWER SYSTEMS.
- 108. THE CONTRACTOR SHALL MAINTAIN ALL FLOWS AND UTILITY CONNECTIONS TO EXISTING BUILDINGS WITHOUT INTERRUPTION UNLESS/UNTIL AUTHORIZED TO DISCONNECT BY THE OWNERS, THE CIVIL ENGINEER, UTILITY PROVIDERS AND GOVERNING AUTHORITIES.
- 109. THE CONTRACTOR MAY SUBSTITUTE MASONRY STRUCTURES FOR PRECAST STRUCTURES IF APPROVED BY THE CIVIL ENGINEER AND ALLOWED BY THE GOVERNING AUTHORITY ENGINEER OR OTHER GOVERNING AUTHORITY.
- 110. PIPING SHALL BE LAID FROM DOWNGRADIENT END OF PIPE RUN IN AN UPGRADIENT DIRECTION WITH BELL END FACING UPGRADE IN THE DIRECTION OF PIPE LAYING.
- 111. MANHOLE SECTIONS AND CONSTRUCTION SHALL CONFORM TO ASTM C-478.
- 112. HIGH DENSITY POLYETHYLENE (HDPE) STORM SEWER 12" OR GREATER IN DIAMETER SHALL BE HI-Q SURE-LOK 10.8 PIPE AS MANUFACTURED BY HANCOR INC. OR APPROVED EQUAL. HDPE PIPE SHALL HAVE SMOOTH INTERIOR AND CORRUGATED EXTERIOR AND SHALL MEET THE REQUIREMENTS OF AASHTO M294, TYPE S. PIPE SECTIONS SHALL BE JOINED WITH BELL-AND-SPIGOT JOINT MEETING THE REQUIREMENTS OF AASHTO M294. THE BELL SHALL BE AN INTEGRAL PART OF THE PIPE AND PROVIDE A MINIMUM PULL-APART STRENGTH OF 400 POUNDS. THE JOINT SHALL BE WATERTIGHT ACCORDING TO THE REQUIREMENTS OF ASTM D3212. GASKETS SHALL BE MADE OF POLYISOPRENE MEETING THE REQUIREMENTS OF ASTM F477. ALTERNATIVE HDPE PIPE MAY BE USED IF APPROVED BY THE ENGINEER AND OWNER'S CONSTRUCTION MANAGER PRIOR TO ORDERING.
- 113. GAS PIPE MATERIAL SHALL BE PER GAS COMPANY REQUIREMENTS.
- 114. POLYVINYL CHLORIDE PIPE (PVCP) FOR SANITARY PIPING SHALL HAVE BUILT-IN RUBBER GASKET JOINTS. PVCP SHALL CONFORM TO ASTM D3034 (SDR35) WITH COMPRESSION JOINTS AND MOLDED FITTINGS. PVCP SHALL BE INSTALLED IN ACCORDANCE WITH THE DETAILS, ASTM D2321 AND MANUFACTURER'S RECOMMENDED PROCEDURE.
- 115. DUCTILE IRON PIPE SHALL CONFORM TO AWWA C151 FOR CLASS 52 WITH CEMENT LINING IN ACCORDANCE WITH ANSI A21.4 FOR WATER MAINS AND SERVICES 3" ID AND LARGER. JOINTS SHALL BE MADE WITH CONCRETE THRUST BLOCKS OR WITH MEGAULUG RETAINER GLANDS OR WITH RODDING IN ACCORDANCE WITH PROJECT MANUAL SPECIFICATIONS AND IN ACCORDANCE WITH WATER UTILITY PROVIDER REQUIREMENTS TO EXTEND A MINIMUM OF 2 PIPE LENGTHS IN EITHER DIRECTION FROM FITTINGS AND ELBOWS (40 FT MINIMUM). ALL OTHER JOINTS SHALL BE PUSH-ON WITH RUBBER GASKETS (TYTON). USE OF OTHER TYPES OF RETAINER GLANDS SHALL REQUIRE USE WITH CLASS 53 OR GREATER DUCTILE IRON PIPE.
- 116. PVC WATER MAIN PIPING SHALL CONFORM TO AWWA C900.

TY PROVIDER.	<u>DEFINITIONS</u> MUNICIPALITY SHALL MEAN CITY OF BRIDGEPORT	Architecture Engineering Environmental Land Surveying
	COUNTY SHALL MEAN FAIRFIELD COUNTY	line iror d S
CH BOTTOM SHALL BE STABLE CK EXCAVATION.	STATE SHALL MEAN CONNECTICUT	Arc
ons under footings.	WATER UTILITY PROVIDER SHALL MEAN AQUARION WATER COMPANY	
ERIFIED WITH THE BUILDING	SANITARY UTILITY PROVIDER SHALL MEAN CITY OF BRIDGEPORT WATER POLLUTION CONTROL AUTHORITY	
	GAS UTILITY PROVIDER SHALL MEAN SOUTHERN CONNECTICUT GAS	
THE APPROPRIATE UTILITY		
PIPING SHALL BE PROVIDED. A TE ENCASEMENT. AN 18-INCH ICASEMENT OF THE PROPOSED	TELECOMMUNICATIONS UTILITY PROVIDER SHALL MEAN OPTIMUM ELECTRIC UTILITY PROVIDER SHALL MEAN UNITED ILLUMINATING COMPANY	
ST 10 FEET APART WHENEVER 18 INCHES ABOVE THE TOP OF WER PIPE TRENCH.		355 Research Parkway Meriden, CT 06450 (203) 630-1406 (203) 630-2615 Fax
uilding stub outs, including		
and valve covers to be		SUMMUTE OF CONVECTION
ELECTRICAL CONTRACTOR.		A Star Star
THE BUILDING ELECTRICAL		hallow Contra
TY PROVIDERS. THE GENERAL CONDITIONS, AND		SS/ONAL ENGINE
CONTRACTOR SHALL PROVIDE E PRIMARY, PVC CONDUITS FOR AVEMENT AREAS). SERVICES NDUITS, AND 24" ON , INSTALLED, AND BACKFILLED EL ELECTRICAL CONDUIT SHALL ID AS REQUIRED BY UTILITY PRIMARY ELECTRIC CONDUITS IF		

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Scale Project No. 12/23/2021 Date CAD File: GN210235701 itle

Designed

Reviewed

Drawn

T.R.J.

T.R.J.

S.M.K.

NONE

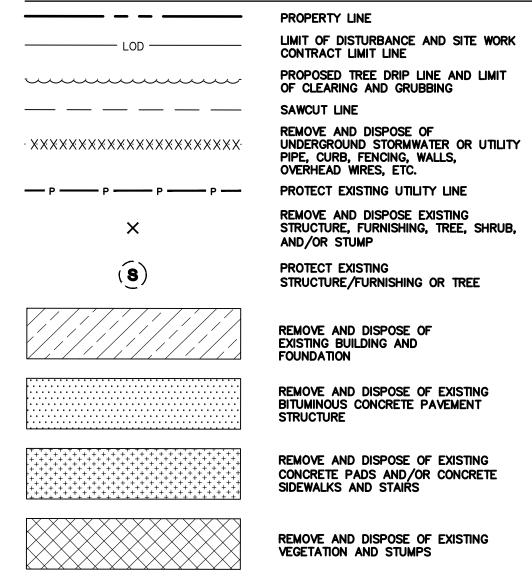
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GENERAL NOTES

Sheet No.







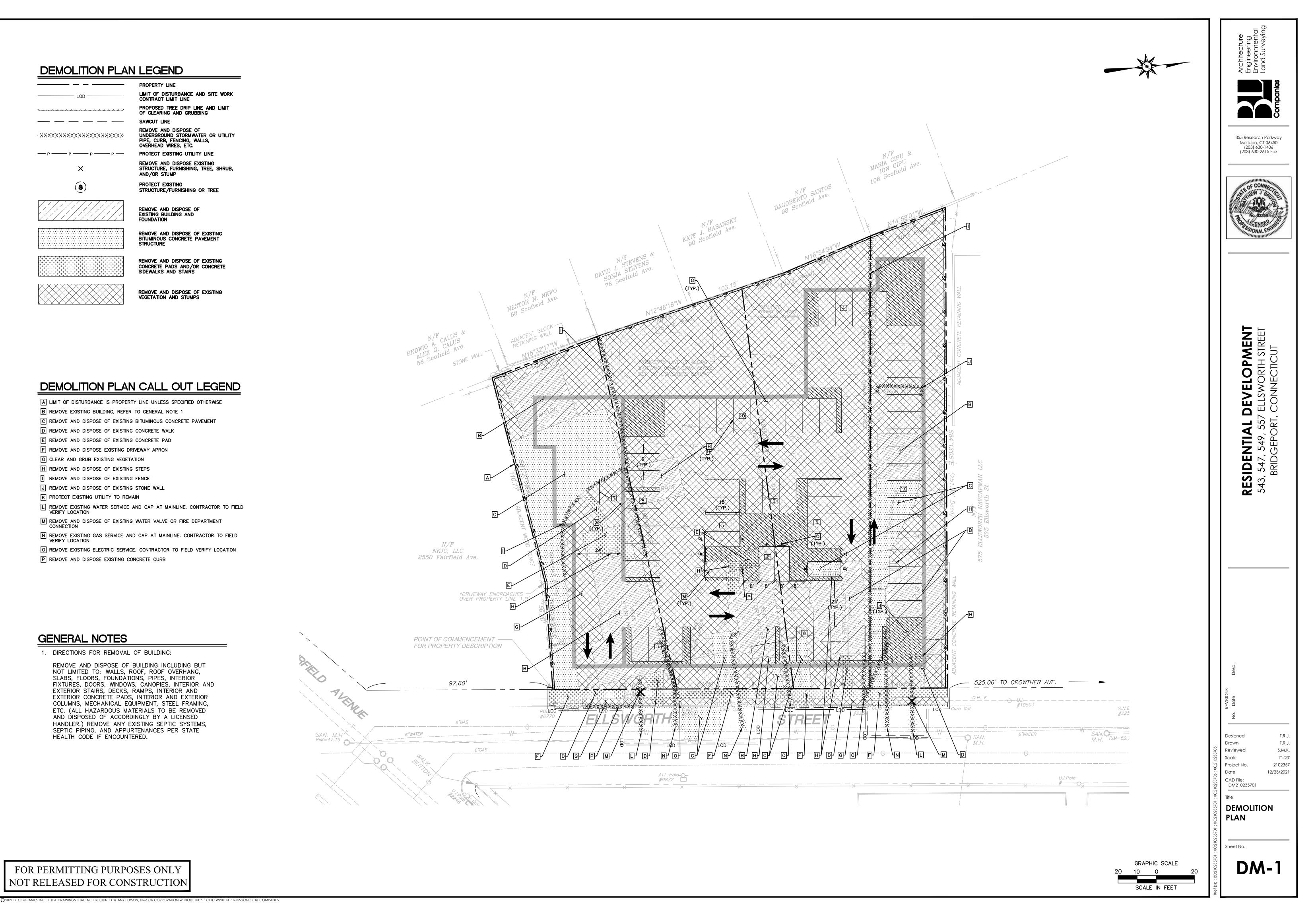
DEMOLITION PLAN CALL OUT LEGEND

- A LIMIT OF DISTURBANCE IS PROPERTY LINE UNLESS SPECIFIED OTHERWISE
- B REMOVE EXISTING BUILDING, REFER TO GENERAL NOTE 1
- C REMOVE AND DISPOSE OF EXISTING BITUMINOUS CONCRETE PAVEMENT
- D REMOVE AND DISPOSE OF EXISTING CONCRETE WALK
- E REMOVE AND DISPOSE OF EXISTING CONCRETE PAD F REMOVE AND DISPOSE EXISTING DRIVEWAY APRON
- G CLEAR AND GRUB EXISTING VEGETATION
- H REMOVE AND DISPOSE OF EXISTING STEPS
- I REMOVE AND DISPOSE OF EXISTING FENCE
- J REMOVE AND DISPOSE OF EXISTING STONE WALL
- K PROTECT EXISTING UTILITY TO REMAIN
- REMOVE EXISTING WATER SERVICE AND CAP AT MAINLINE. CONTRACTOR TO FIELD VERIFY LOCATION
- M REMOVE AND DISPOSE OF EXISTING WATER VALVE OR FIRE DEPARTMENT
- N REMOVE EXISTING GAS SERVICE AND CAP AT MAINLINE. CONTRACTOR TO FIELD VERIFY LOCATION
- O REMOVE EXISTING ELECTRIC SERVICE. CONTRACTOR TO FIELD VERIFY LOCATION
- P REMOVE AND DISPOSE EXISTING CONCRETE CURB

## GENERAL NOTES

1. DIRECTIONS FOR REMOVAL OF BUILDING:

REMOVE AND DISPOSE OF BUILDING INCLUDING BUT NOT LIMITED TO: WALLS, ROOF, ROOF OVERHANG, SLABS, FLOORS, FOUNDATIONS, PIPES, INTERIOR FIXTURES, DOORS, WINDOWS, CANOPIES, INTERIOR AND EXTERIOR STAIRS, DECKS, RAMPS, INTERIOR AND EXTERIOR CONCRETE PADS, INTERIOR AND EXTERIOR COLUMNS, MECHANICAL EQUIPMENT, STEEL FRAMING, ETC. (ALL HAZARDOUS MATERIALS TO BE REMOVED AND DISPOSED OF ACCORDINGLY BY A LICENSED HANDLER.) REMOVE ANY EXISTING SEPTIC SYSTEMS, SEPTIC PIPING, AND APPURTENANCES PER STATE HEALTH CODE IF ENCOUNTERED.



FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

## ZONING INFORMATION

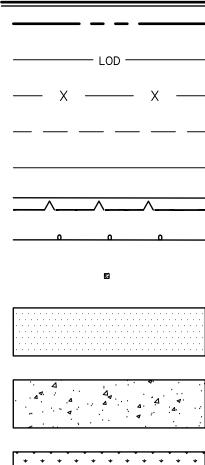
## DADVING INFORMATION

SAN. M.H. RIM=47.19

LOCATION:	OCATION: BRIDGEPORT, FAIRFIELD COUNTY, CONNECTICUT						
ZONE: R-	ONE: R-CC (RESIDENTIAL HIGH DENSITY)						
use: Mul	JSE: MULTI-FAMILY RESIDENTIAL (PERMITTED USE)						
ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE			
1	MINIMUM LOT AREA	10,000 S.F.	46,195 S.F. (1.06 AC.)	NO			
2	MINIMUM LOT DEPTH	100 FEET	209 FEET	NO			
3	MINIMUM LOT FRONTAGE	75 FEET	207 FEET	NO			
4	MINIMUM FRONT SETBACK	10 FEET	10 FEET	NO			
5	MINIMUM SIDE SETBACK	10 FEET	10 FEET	NO			
6	MINIMUM REAR SETBACK	10 FEET	10 FEET	NO			
7	MAXIMUM BUILDING HEIGHT	65 FEET/ 6 STORIES	6 STORIES	NO			
8	MAXIMUM BUILDING COVERAGE	75 PERCENT	70 PERCENT	NO			
9	MAXIMUM IMPERVIOUS COVERAGE	80 PERCENT	79 PERCENT	NO			

ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE
1	BUILDING SIZE	NONE REQUIRED	32,301 S.F.	NO
2 PARKING REQUIRED		<u>MULTI-FAMILY:</u> 1 SPACE PER DWELLING UNIT PLUS 10% FOR VISITORS (123 UNITS) TOTAL REQUIRED = 135	135 STALLS (SEE PARKING BREAKDOWN TABLE)	NO
3	MINIMUM HANDICAPPED PARKING SPACES REQUIRED	5 SPACES	5 SPACES	NO
4	MINIMUM PARKING DIMENSIONS	9 FEET X 18 FEET	9 FEET X 18 FEET	NO
5	MINIMUM AISLE WIDTH	24 FEET – 2–WAY 11 FEET – 1–WAY	24 FEET - 2-WAY	NO
6	MINIMUM FRONT SETBACK	10 FEET	10 FEET	NO
7	MINIMUM SIDE SETBACK	10 FEET	10 FEET	NO
8	MINIMUM REAR SETBACK	5 FEET	5 FEET	NO
9	MINIMUM INTERIOR LANDSCAPING	SURFACE PARKING AREAS GREATER THAN 2,500 S.F. OR WITH MORE THAN 10 SPACES MUST CONTAIN INTERIOR LANDSCAPING	SURFACE PARKING AREA < 2,500 S.F. < 10 SPACES	NO

## SITE PLAN LEGEND



\* \* \* \* \* \* \* \* \* \*

### LIMIT OF DISTURBANCE CHAIN LINK FENCE

SAWCUT

PROPERTY LINE

BITUMINOUS CONCRETE, CONCRETE, OR GRANITE CURB

MODULAR BLOCK RETAINING WALL

RB-350 METAL BEAM GUIDERAIL

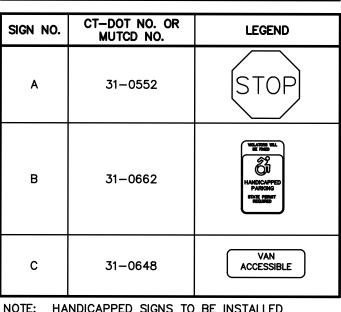
BUILDING COLUMN

LANDSCAPED AREA

STANDARD DUTY PAVEMENT STRUCTURE

REINFORCED CONCRETE SIDEWALK

## SIGN LEGEND



NOTE: HANDICAPPED SIGNS TO BE INSTALLED IN PIPE BOLLARDS (SEE DETAIL). ALL HANDICAP SIGNAGE TO CONFORM TO LATEST BUILDING CODE.

## SITE PLAN CALL OUT LEGEND

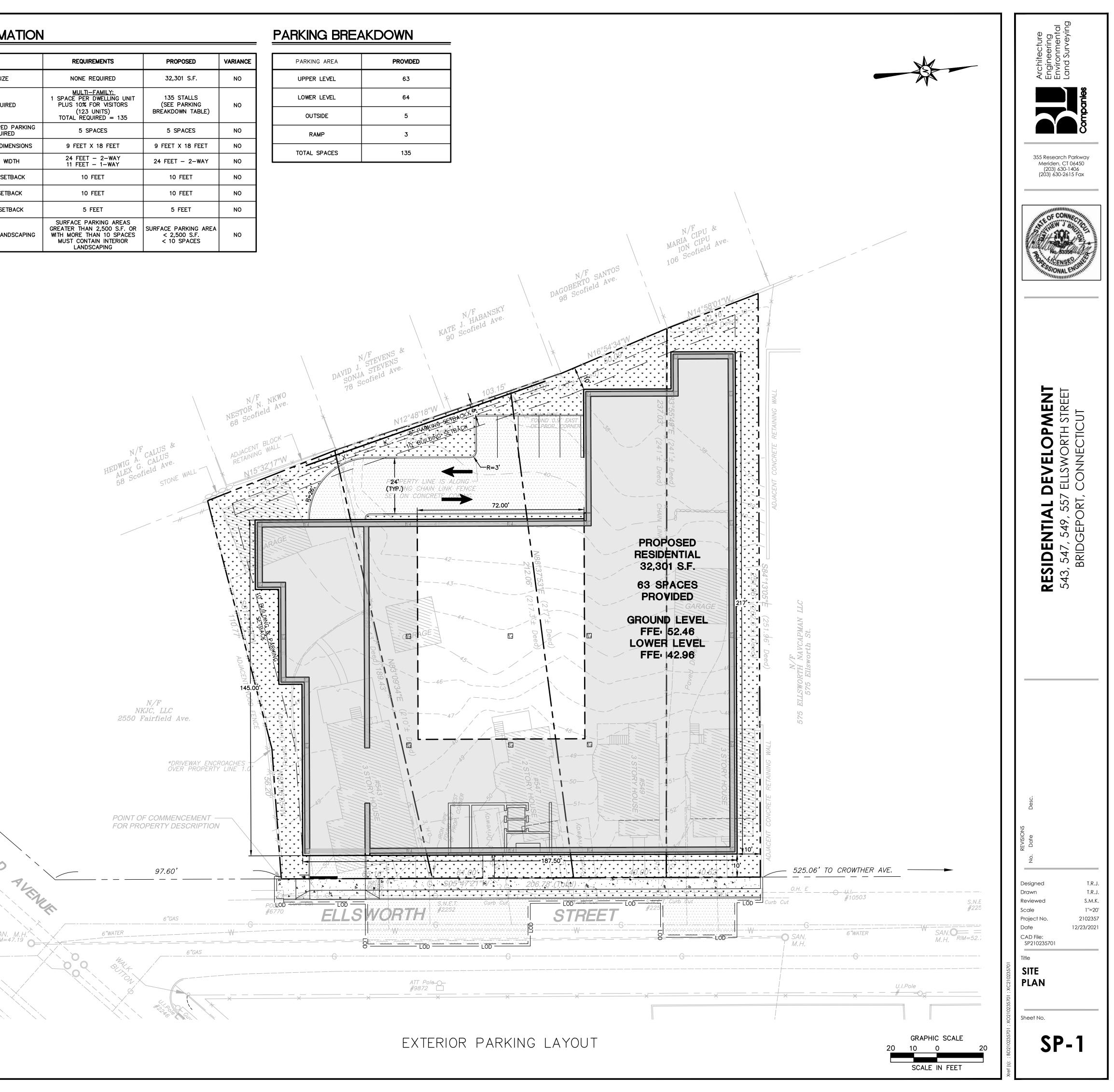
A LIMIT OF DISTURBANCE IS PROPERTY LINE UNLESS SPECIFIED OTHERWISE B PROVIDE AND INSTALL BITUMINOUS CONCRETE CURB C PROVIDE AND INSTALL CONCRETE CURB D PROVIDE AND INSTALL BUILDING, SEE PLANS BY OTHERS E PROVIDE AND INSTALL STANDARD DUTY PAVEMENT STRUCTURE F PROVIDE AND INSTALL CONCRETE SIDEWALK G PROVIDE AND INSTALL CONCRETE DRIVEWAY APRON H PROVIDE AND INSTALL 4" SWL PROVIDE AND INSTALL 4" SWL J PROVIDE AND INSTALL 4" SOLID WHITE LINE AT 45" AND SPACED 2' O.C. K PROVIDE AND INSTALL 12" SOLID WHITE STOP BAR L PROVIDE AND INSTALL DIRECTIONAL ARROWS M PROVIDE AND INSTALL CONNECTICUT ACCESSIBLE SYMBOL PAVEMENT MARKING N PROVIDE AND INSTALL SIGN A O PROVIDE AND INSTALL SIGN B P PROVIDE AND INSTALL SIGNS B AND C Q PROVIDE AND INSTALL CONCRETE TRANSITION CURB R PROPERTY LINE DIVIDING PARCELS TO BE EXTINGUISHED S PROVIDE AND INSTALL CONCRETE TRANSFORMER PAD PER UTILITY PROVIDER'S REQUIREMENTS T RAMP TO LOWER LEVEL U RAMP TO STREET LEVEL V PROVIDE AND INSTALL RETAINING WALL W COLUMN (SEE ARCHITECTURAL PLAN) X SOLID WALL Y GARAGE ENTRANCE Z PROVIDE AND INSTALL CHAIN LINK FENCE



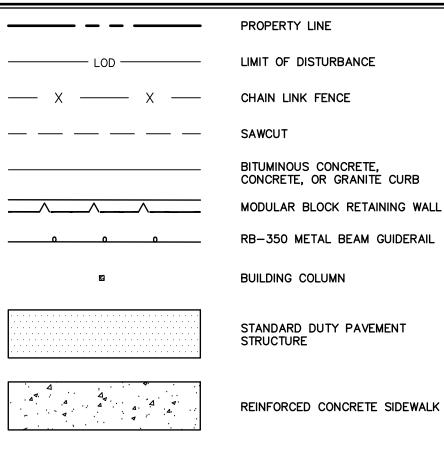
## FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

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PARKING AREA	PROVIDED
UPPER LEVEL	63
LOWER LEVEL	64
OUTSIDE	5
RAMP	3
TOTAL SPACES	135



## SITE PLAN LEGEND



\* \* \* \* \* \* \* \* \*

BITUMINOUS CONCRETE, CONCRETE, OR GRANITE CURB MODULAR BLOCK RETAINING WALL

BUILDING COLUMN

LANDSCAPED AREA

STANDARD DUTY PAVEMENT STRUCTURE

REINFORCED CONCRETE SIDEWALK

## SIGN LEGEND

SIGN NO.	CT-DOT NO. OR MUTCD NO.	LEGEND
A	31–0552	STOP
В	31–0662	HANDLOAPPED HANDLOAPPED PARKING FILE FILE FILE FILE FILE FILE FILE FILE
с	31–0648	VAN ACCESSIBLE

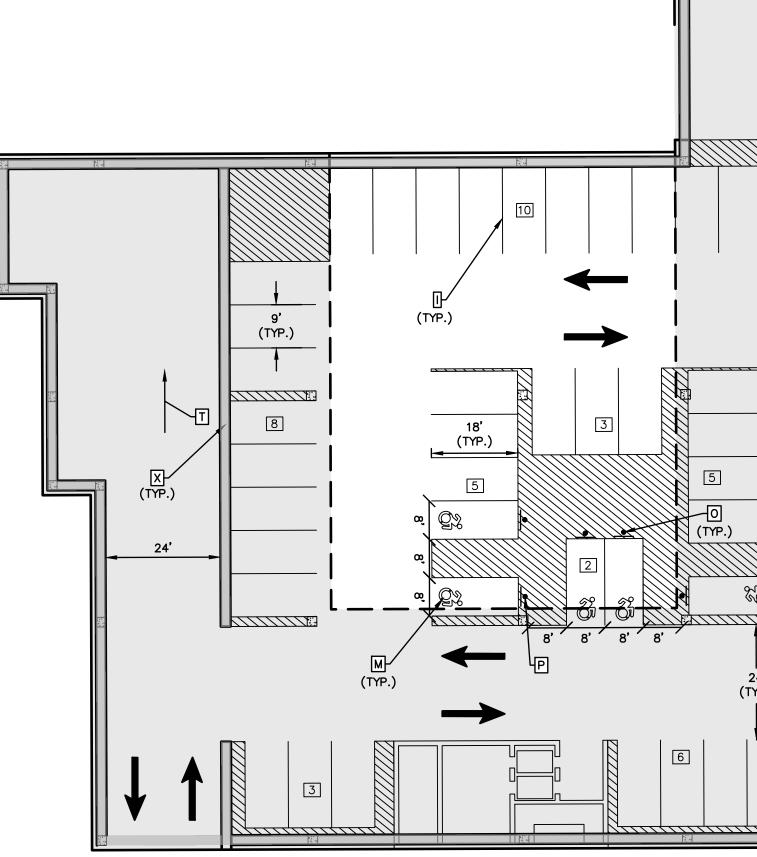
NOTE: HANDICAPPED SIGNS TO BE INSTALLED IN PIPE BOLLARDS (SEE DETAIL). ALL HANDICAP SIGNAGE TO CONFORM TO LATEST BUILDING CODE.

## SITE PLAN CALL OUT LEGEND

A LIMIT OF DISTURBANCE IS PROPERTY LINE UNLESS SPECIFIED OTHERWISE B PROVIDE AND INSTALL BITUMINOUS CONCRETE CURB C PROVIDE AND INSTALL CONCRETE CURB D PROVIDE AND INSTALL BUILDING, SEE PLANS BY OTHERS E PROVIDE AND INSTALL STANDARD DUTY PAVEMENT STRUCTURE F PROVIDE AND INSTALL CONCRETE SIDEWALK G PROVIDE AND INSTALL CONCRETE DRIVEWAY APRON H PROVIDE AND INSTALL 4" SWL PROVIDE AND INSTALL 4" SWL J PROVIDE AND INSTALL 4" SOLID WHITE LINE AT 45" AND SPACED 2' O.C. K PROVIDE AND INSTALL 12" SOLID WHITE STOP BAR L PROVIDE AND INSTALL DIRECTIONAL ARROWS M PROVIDE AND INSTALL CONNECTICUT ACCESSIBLE SYMBOL PAVEMENT MARKING N PROVIDE AND INSTALL SIGN A O PROVIDE AND INSTALL SIGN B P PROVIDE AND INSTALL SIGNS B AND C Q PROVIDE AND INSTALL CONCRETE TRANSITION CURB R PROPERTY LINE DIVIDING PARCELS TO BE EXTINGUISHED S PROVIDE AND INSTALL CONCRETE TRANSFORMER PAD PER UTILITY PROVIDER'S REQUIREMENTS T RAMP TO LOWER LEVEL U RAMP TO STREET LEVEL V PROVIDE AND INSTALL RETAINING WALL W COLUMN (SEE ARCHITECTURAL PLAN) X SOLID WALL Y GARAGE ENTRANCE Z PROVIDE AND INSTALL CHAIN LINK FENCE

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UPPER LEVEL GARAGE PARKING LAYO

		Architecture         Braineering         Braineering
PROPOSED   RESIDENTIAL   32,301 S.F.   63 SPACES   PROVIDED   GROUND LEVEL   FFE: 52.46   DED   GROUND LEVEL   FFE: 42.96		<b>RESIDENTIAL DEVELOPMENT</b> 543, 547, 549, 557 ELLSWORTH STREET BRIDGEPORT, CONNECTICUT
OUT	GRAPHIC SCALE 20 10 0 20	IVECTOR TO THE SITE PLAN Sheet No. Sheet No.
	20 10 0 20	Xref (s): ; BD2

## SITE PLAN LEGEND

	PROPERTY LINE
LOD	LIMIT OF DISTURBAN
x x	CHAIN LINK FENCE
	SAWCUT
	BITUMINOUS CONCRE CONCRETE, OR GRAM
	MODULAR BLOCK RE
0 0 0	RB-350 METAL BEA
83	BUILDING COLUMN
	STANDARD DUTY PA STRUCTURE
	REINFORCED CONCRE
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	LANDSCAPED AREA

DISTURBANCE

DUS CONCRETE, TE, OR GRANITE CURB

BLOCK RETAINING WALL METAL BEAM GUIDERAIL

RD DUTY PAVEMENT

CED CONCRETE SIDEWALK

## SITE PLAN CALL OUT LEGEND

A LIMIT OF DISTURBANCE IS PROPERTY LINE UNLESS SPECIFIED OTHERWISE
B PROVIDE AND INSTALL BITUMINOUS CONCRETE CURB
C PROVIDE AND INSTALL CONCRETE CURB
D PROVIDE AND INSTALL BUILDING, SEE PLANS BY OTHERS
E PROVIDE AND INSTALL STANDARD DUTY PAVEMENT STRUCTURE
F PROVIDE AND INSTALL CONCRETE SIDEWALK
G PROVIDE AND INSTALL CONCRETE DRIVEWAY APRON
H PROVIDE AND INSTALL 4" SWL
PROVIDE AND INSTALL 4" SWL
PROVIDE AND INSTALL 4" SOLID WHITE LINE AT 45" AND SPACED 2' O.C.
K PROVIDE AND INSTALL 12" SOLID WHITE STOP BAR
L PROVIDE AND INSTALL DIRECTIONAL ARROWS
M PROVIDE AND INSTALL CONNECTICUT ACCESSIBLE SYMBOL PAVEMENT MARKING
N PROVIDE AND INSTALL SIGN A
O PROVIDE AND INSTALL SIGN B
P PROVIDE AND INSTALL SIGNS B AND C
Q PROVIDE AND INSTALL CONCRETE TRANSITION CURB
R PROPERTY LINE DIVIDING PARCELS TO BE EXTINGUISHED
S PROVIDE AND INSTALL CONCRETE TRANSFORMER PAD PER UTILITY PROVIDER'S REQUIREMENTS
T RAMP TO LOWER LEVEL
U RAMP TO STREET LEVEL
V PROVIDE AND INSTALL RETAINING WALL

W COLUMN (SEE ARCHITECTURAL PLAN)

X SOLID WALL

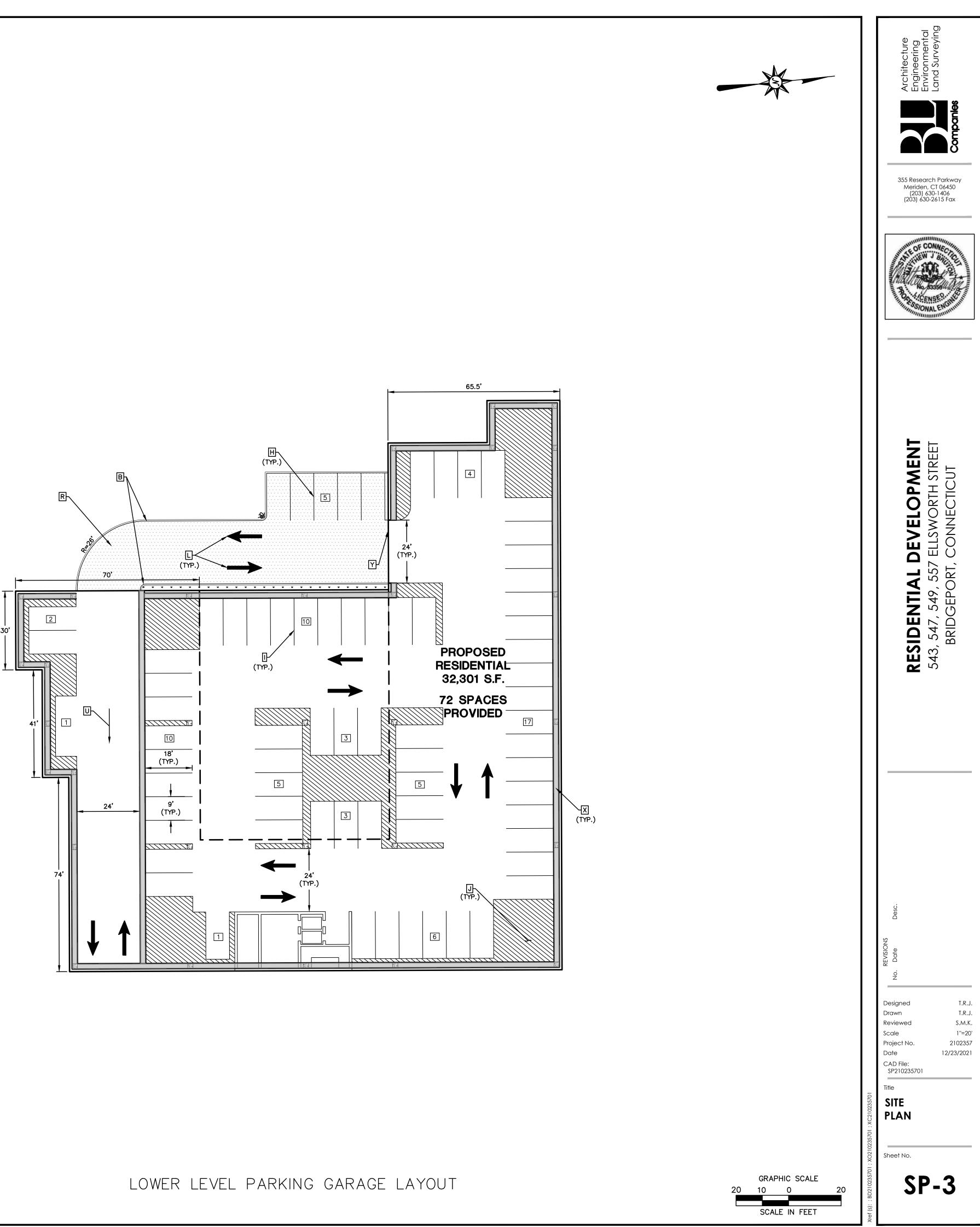
Y GARAGE ENTRANCE Z PROVIDE AND INSTALL CHAIN LINK FENCE

## SIGN LEGEND

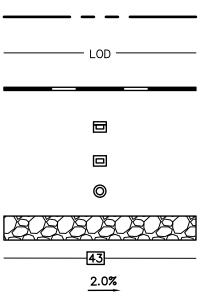
SIGN NO.	CT-DOT NO. OR MUTCD NO.	LEGEND
A	31–0552	STOP
В	31–0662	HANDGLAPPED STATE OF THE
с	31–0648	VAN ACCESSIBLE

NOTE: HANDICAPPED SIGNS TO BE INSTALLED IN PIPE BOLLARDS (SEE DETAIL). ALL HANDICAP SIGNAGE TO CONFORM TO LATEST BUILDING CODE.

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## GRADING AND DRAINAGE LEGEND



×43.00

PROPOSED SPOT GRADE

 TC=TOP OF CURB - BC=BOTTOM OF CURB

- TW=TOP OF WALL - BW=BOTTOM OF WALL

LIMIT OF DISTURBANCE

STORM LINE

RIP RAP SWALE

PROPERTY LINE

TYPE "C" CATCH BASIN

TYPE "CL" CATCH BASIN

OUTLET CONTROL STRUCTURE/WATER QUALITY UNIT

PROPOSED CONTOUR LINE

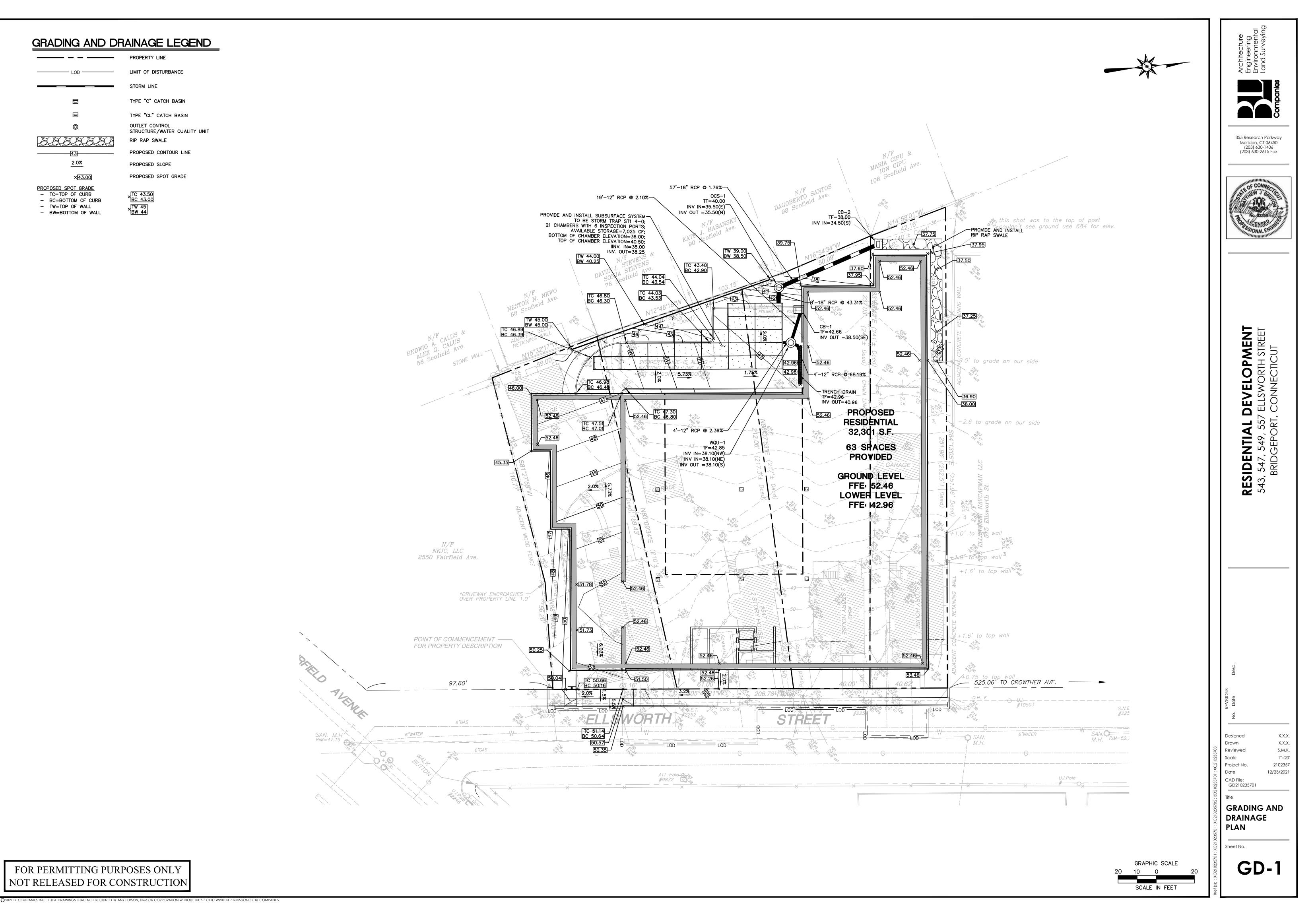
PROPOSED SLOPE

PROPOSED SPOT GRADE





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## SITE UTILITIES LEGEND

PROPERTY LINE	
LIMIT OF DISTURBANCE LINE AND CONTRACT LIMIT LINE	LOD
ELECTRIC LINE	— Е — Е — —
	ETC
TELECOMMUNICATIONS LINES GAS LINE	G G
WATER LINE	W W
SANITARY SEWER LINE	S
SANITARY SEWER FORCE MAIN	SFM
OVERHEAD LINE	—— ОН ——— ОН ———
TRANSFORMER	T
HYDRANT	×
UTILITY POLE	-@-
SANITARY MANHOLE	•
SANITARY CLEANOUT	°00
WATER VALVE	$\otimes^{WV}$
GATE VALVE	⊗ <sup>GV</sup>
THRUST BLOCK	4
GREASE TRAP	••
OUTLET CONTROL STRUCTURE	$\bigcirc$
HYDRODYNAMIC SEPARATOR	
STORM LINE	
CATCH BASIN	
STORM MANHOLE	
FLARED END	
END WALL OR HEADWALL	
PROPOSED CONTOUR LINE	228
PROPOSED SPOT GRADE	X [100.00]
ABBREVIATIONS	TC=100.00
<ul> <li>TC=TOP OF CURB</li> <li>BC=BOTTOM OF CURB</li> </ul>	X BC=99.50
- TW=TOP OF WALL - BW=BOTTOM OF WALL	TW=108.00
	x BW=100.00
PROPOSED SURFACE SLOPE	2%

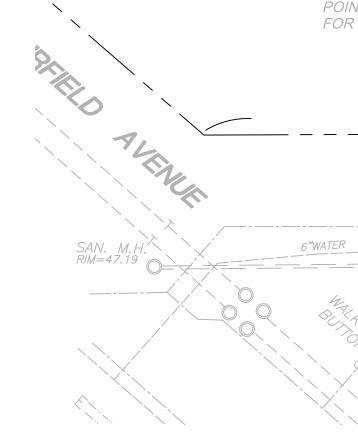
## SITE UTILITIES CALL OUT LEGEND

- A PROVIDE AND INSTALL TAPPING SLEEVE AND VALVE FOR DOMESTIC WATER SERVICE. COORDINATE TAPPING SLEEVE AND VALVE INSTALLATION WITH AQUARION WATER COMPANY.
   B PROVIDE AND INSTALL 4" DI CLASS 52 WATER SERVICE TO BUILDING
- C PROVIDE AND INSTALL WATER METER PER AQUARION WATER COMPANY STANDARDS
- D PROVIDE AND INSTALL TAPPING SLEEVE AND VALVE FOR FIRE WATER SERVICE. COORDINATE TAPPING SLEEVE AND VALVE INSTALLATION WITH AQUARION WATER COMPANY.
- E PROVIDE AND INSTALL 6" CLDI FIRE SERVICE TO BUILDING
- F CONNECT TO EXISTING GAS MAIN. COORDINATE WITH UTILITY PROVIDER
- G PROVIDE AND INSTALL GAS METER AND BOLLARDS PER GAS UTILITY PROVIDER REQUIREMENTS
- H PROVIDE AND INSTALL 6" PVC SDR 35 SANITARY PIPE
- PROVIDE AND INSTALL CLEANOUT
- J PROVIDE AND INSTALL SADDLE CONNECTION AT SANITARY PIPE. CONTRACTOR TO FIELD VERIFY INVERT AND NOTIFY ENGINEER PRIOR TO CONSTRUCTION.
- K CONNECT TO EXISTING UTILITY POLE FOR ELECTRIC SERVICE. COORDINATE WITH ELECTRIC SERVICE PROVIDER
- PROVIDE AND INSTALL (6) 4" SCH 80 PVC CONDUITS FOR TELECOMMUNICATIONS AND ELECTRICAL SERVICE
- M CONNECT TO EXISTING UTILITY POLE FOR TELEPHONE SERVICE. COORDINATE WITH TELEPHONE SERVICE PROVIDER

## NOTES

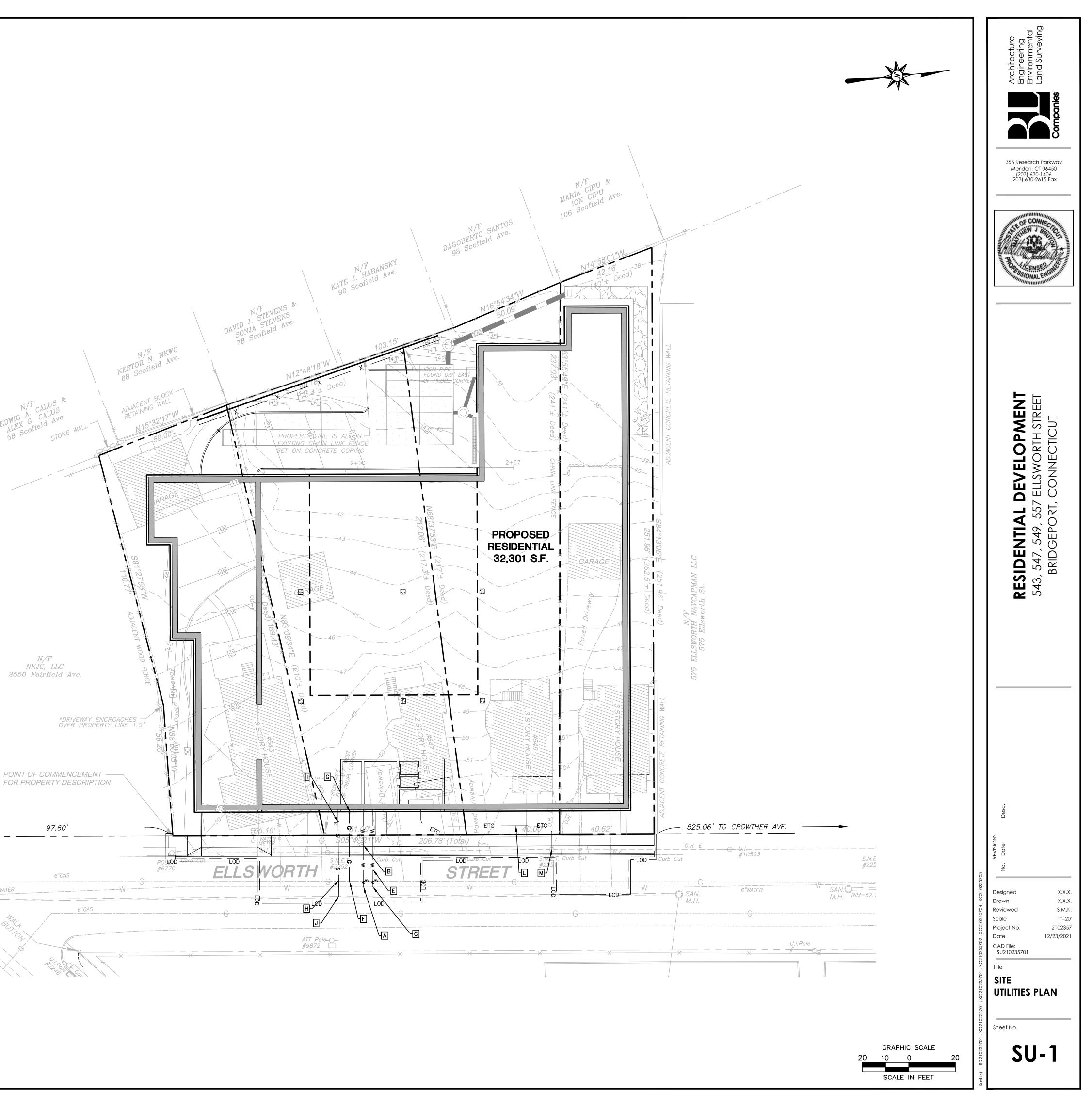
CONTRACTOR SHALL CONFIRM LOCATION, SIZE, CONDITION AND ELEVATION OF ALL UTILITY LATERAL STUBS, WATER MAINS, GAS MAINS AND ELECTRICAL SERVICES PRIOR TO CONSTRUCTION.

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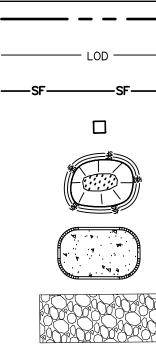


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## EROSION CONTROL PLAN LEGEND



PROPERTY LINE LIMIT OF DISTURBANCE SILT FENCE SILT SACKS AT CATCH BASINS

MATERIAL STOCKPILE AREA WITH DOUBLE ROW OF SILT FENCE

CONCRETE WASHPIT

CONSTRUCTION ENTRANCE

## SOIL TYPE

307 URBAN LAND

NOTE: ENTIRE SITE IS DESIGNATED AS SOIL TYPE 307 PER WEB SOIL SURVEY

## EROSION CONTROL CALL OUT LEGEND

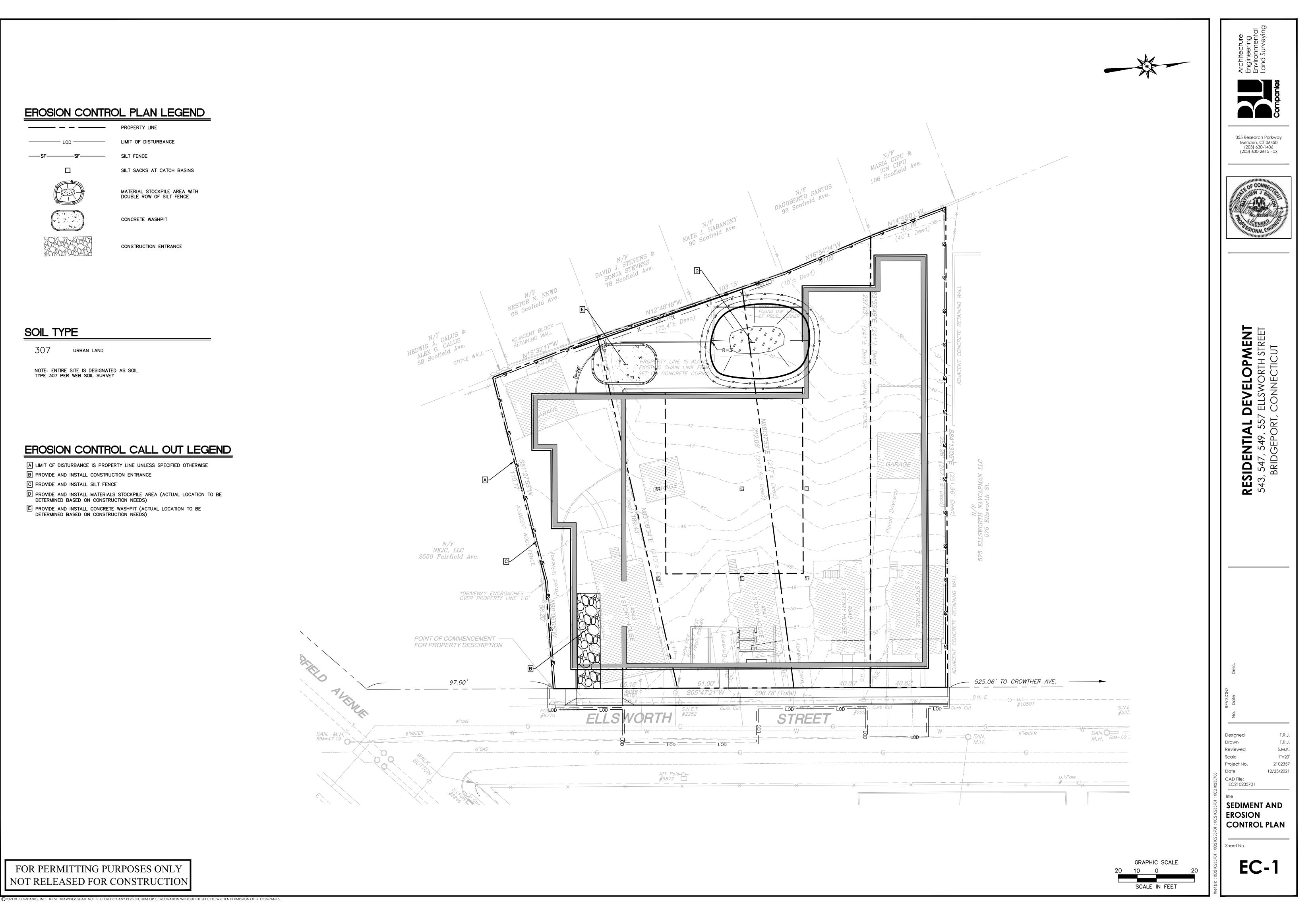
A LIMIT OF DISTURBANCE IS PROPERTY LINE UNLESS SPECIFIED OTHERWISE

B PROVIDE AND INSTALL CONSTRUCTION ENTRANCE

C PROVIDE AND INSTALL SILT FENCE

PROVIDE AND INSTALL MATERIALS STOCKPILE AREA (ACTUAL LOCATION TO BE DETERMINED BASED ON CONSTRUCTION NEEDS)

E PROVIDE AND INSTALL CONCRETE WASHPIT (ACTUAL LOCATION TO BE DETERMINED BASED ON CONSTRUCTION NEEDS)



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## SEDIMENT AND EROSION CONTROL NOTES

SEDIMENT AND EROSION CONTROL NOTES - CONNECTICUT

SEDIMENT & EROSION CONTROL NARRATIVE

THE SEDIMENT AND EROSION CONTROL PLAN WAS DEVELOPED TO PROTECT THE EXISTING ROADWAY AND STU SYSTEMS, ADJACENT PROPERTIES, AND ANY ADJACENT WETLAND AREA AND ANY ADJACENT WATER COURSE LADEN SURFACE RUNOFF AND EROSION. A CONSTRUCTION SEQUENCE IS PROVIDED TO PROVIDE SURFACE R CONTROLS PRIOR TO THE BEGINNING OF PROJECT DEMOLITION AND/OR CONSTRUCTION.

CONSTRUCTION SCHEDULE THE ANTICIPATED STARTING DATE FOR CONSTRUCTION IS SPRING 2022 WITH COMPLETION ANTICIPATED WINTE APPROPRIATE SEDIMENT AND EROSION CONTROL MEASURES AS DESCRIBED HEREIN SHALL BE INSTALLED BY PRIOR TO THE COMMENCEMENT OF ALL DEMOLITION OR CONSTRUCTION ACTIVITY. SCHEDULE WORK TO MINIMIZ TIME THAT BARE SOIL WILL BE EXPOSED.

CONTINGENCY EROSION PLAN THE CONTRACTOR SHALL INSTALL ALL SPECIFIED SEDIMENT AND EROSION CONTROL MEASURES AND WILL BE MAINTAIN THEM IN THEIR INTENDED FUNCTIONING CONDITION. THE AGENTS OF THE MUNICIPALITY AND/OR CO CONSERVATION DISTRICT OR INLAND WETLANDS COMMISSION CITY OF BRIDGEPORT AND/OR CIVIL ENGINEER S AUTHORITY TO REQUIRE SUPPLEMENTAL MAINTENANCE OR ADDITIONAL MEASURES IF FIELD CONDITIONS ARE BEYOND WHAT WOULD NORMALLY BE ANTICIPATED.

CONSTRUCTION SEQUENCE THE FOLLOWING CONSTRUCTION SEQUENCE IS RECOMMENDED:

- 1. CONTACT MUNICIPALITY AND/OR COUNTY SOILS CONSERVATION DISTRICT OR INLAND WETLANDS COMMISSI BRIDGEPORT AGENT AT LEAST FORTY-EIGHT (48) HOURS PRIOR TO COMMENCEMENT OF ANY DEMOLITION, OR REGULATED ACTIVITY ON THIS PROJECT.
- 2. CLEARING LIMITS SHALL BE PHYSICALLY MARKED IN THE FIELD AND APPROVED BY THE MUNICIPALITY AN SOILS CONSERVATION DISTRICT OR INLAND WETLANDS COMMISSION CITY OF BRIDGEPORT AGENT PRIOR TO WORK ON THE SITE. INSTALL TREE PROTECTION AND PERIMETER SILT FENCE.
- 3. CONSTRUCT STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS AT CONSTRUCTION ENTRANCES/EXIT FILTER FABRIC AROUND GRATES OF CATCH BASINS OR INSTALL SILT SACKS ON CATCH BASIN INLETS ON INSTALL SILT FENCE AND OTHER EROSION CONTROL DEVICES INDICATED ON THESE PLANS AT PERIMETER SITE DISTURBANCE AND INSTALL ALL EROSION CONTROL MEASURES AND TREE PROTECTION INDICATED ON INSTALL SEDIMENT BASINS AND SEDIMENT TRAPS IF REQUIRED AT LOW AREAS OF SITE OR AS ORDERED OR AS SHOWN ON THESE PLANS.
- 4. CLEAR AND GRUB SITE. STOCKPILE CHIPS. STOCKPILE TOPSOIL. INSTALL SEDIMENT AND EROSION CONTR STOCKPILES.
- 5. BUILDING AND SITE DEMOLITION AND REMOVAL. PAVEMENT REMOVAL.
- 6. INSTALL SILT FENCE, CONSTRUCT DIVERSION SWALES AND SEDIMENT TRAPS. COMMENCE INSTALLATION O SYSTEM.
- 7. COMMENCE EARTHWORK. CONSTRUCT FILL SLOPE, ROADWAY, RETAINING WALLS. INSTALL ADDITIONAL SED CONTROLS AS WORK PROGRESSES AND CONTINUE STORM DRAINAGE SYSTEM CONSTRUCTION, TOPSOIL AN WHICH HAVE ACHIEVED FINAL SITE GRADING.
- 8. CONSTRUCTION STAKING OF ALL BUILDING CORNERS, UTILITIES, ACCESS DRIVES, AND PARKING AREAS.9. ROUGH GRADING AND FILLING OF SUBGRADES AND SLOPES.
- 10. IMMEDIATELY UPON DISCOVERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATE SEDIMENT POLLUTION, THE OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO POTENTIAL FOR ACCELERATED EROSION AND/OR SEDIMENT POLLUTION.
- 11. BEFORE DISPOSING OF SOIL OR RECEIVING BORROW FOR THE SITE, THE CONTRACTOR MUST PROVIDE EVIL SPOIL OR BORROW AREA HAS A SEDIMENT AND EROSION CONTROL PLAN APPROVED BY THE MUNICIPALIT SOILS CONSERVATION DISTRICT AND/OR INLAND WETLANDS COMMISSION CITY OF BRIDGEPORT AND WHICH IMPLEMENTED AND MAINTAINED. THE CONTRACTOR SHALL ALSO NOTIFY THE MUNICIPALITY AND/OR COUL CONSERVATION DISTRICT AND/OR INLAND WETLANDS COMMISSION CITY OF BRIDGEPORT IN WRITING OF AL AND BORROW AREAS WHEN THEY HAVE BEEN IDENTIFIED.
- 12. CONTINUE INSTALLATION OF STORM DRAINAGE AS SUBGRADE ELEVATIONS ARE ACHIEVED.
- 13. BUILDING FOUNDATION SUBGRADE AND PAD SUBGRADE PREPARATION.
- 14. BUILDING FOUNDATION CONSTRUCTION. BEGIN BUILDING SUPERSTRUCTURE
- 15. THROUGHOUT CONSTRUCTION SEQUENCE, REMOVE SEDIMENT FROM BEHIND SILT FENCES, STRAW BALES A CONTROL DEVICES, AND FROM SEDIMENT BASINS AND SEDIMENT TRAPS AS REQUIRED. REMOVAL SHALL B BASIS (EVERY SIGNIFICANT RAINFALL OF 0.25 INCH OR GREATER). INSPECTION OF SEDIMENT AND EROSIO MEASURES SHALL BE ON A WEEKLY BASIS AND AFTER EACH RAINFALL OF 0.25 INCHES OR GREATER. SE SHALL BE DEPOSITED AND SPREAD EVENLY UPLAND ON SLOPES DURING CONSTRUCTION.
- 16. INSTALL SANITARY LATERAL AND UTILITIES. COMPLETE STORM DRAINAGE SYSTEM.
- 17. INSTALL SITE LIGHTING AND TRASH ENCLOSURE.
- 18. COMPLETE GRADING TO SUBGRADES AND CONSTRUCT PARKING AREA SUBGRADE.
- 19. CONSTRUCT CURBS, PAVEMENT STRUCTURE AND SIDEWALKS.
- 20. CONDUCT FINE GRADING.
- 21. PAVING OF PARKING AREAS AND DRIVEWAYS
- 22. FINAL FINE GRADING OF SLOPE AND NON-PAVED AREAS.
- 23. PLACE 4" TOPSOIL ON SLOPES AFTER FINAL GRADING IS COMPLETED. FERTILIZE SEED AND MULCH. SEED INSTALLED AUGUST 15, 2022 - OCTOBER 1, 2022. USE EROSION CONTROL BLANKETS AS REQUIRED OR SLOPES GREATER THAN 3:1 AND AS SHOWN ON LANDSCAPE PLANS OR EROSION CONTROL PLANS. FOR STABILIZATION BEYOND SEEDING DATES USE ANNUAL RYE AT 4.0 LBS/1,000 S.F. FERTILIZE WITH 10-10-NITROGEN PER 1,000 S.F. AND LIME AT 100 LBS/1,000 S.F. (MAX.).
- 24. LANDSCAPE ISLANDS, INTERIOR NON-PAVED AREAS, AND PERIMETER AREAS.
- 25. INSTALL SIGNING AND PAVEMENT MARKINGS
- 26. CLEAN STORM DRAINAGE PIPE STRUCTURES, DETENTION SYSTEMS AND WATER QUALITY DEVICES OF DEBI
- 27. UPON DIRECTION OF THE MUNICIPALITY AND/OR COUNTY SOILS CONSERVATION DISTRICT AND/OR INLAND COMMISSION CITY OF BRIDGEPORT AGENT, SEDIMENT AND EROSION CONTROL MEASURES SHALL BE REMO STABILIZATION OF THE SITE.

OPERATION REQUIREMENTS

ROUGH GRADING OPERATIONS

- CLEARING AND GRUBBING OPERATIONS 1. ALL SEDIMENT AND EROSION CONTROL MEASURES, INCLUDING THE CONSTRUCTION OF TEMPORARY SEDIMI AND STONE CONSTRUCTION ENTRANCE ANTI-TRACKING PADS, WILL BE INSTALLED PRIOR TO THE START GRUBBING AND DEMOLITION OPERATIONS.
- FOLLOWING INSTALLATION OF ALL SEDIMENT AND EROSION CONTROL MEASURES, THE CONTRACTOR SHALL WITH GRADING, FILLING OR OTHER CONSTRUCTION OPERATIONS UNTIL THE ENGINEER HAS INSPECTED AND INSTALLATIONS.
- 3. THE CONTRACTOR SHALL TAKE EXTREME CARE DURING CLEARING AND GRUBBING OPERATIONS SO AS NO UNPROTECTED WETLAND AREAS OR SEDIMENT AND EROSION CONTROL DEVICES.
- 4. FOLLOWING THE COMPLETION OF CLEARING AND GRUBBING OPERATIONS, ALL AREAS SHALL BE STABILIZE AND SEEDING OR CRUSHED STONE AS SOON AS PRACTICAL.

1. DURING THE REMOVAL AND/OR PLACEMENT OF EARTH AS INDICATED ON THE GRADING PLAN, TOPSOIL

- AND APPROPRIATELY STOCKPILED FOR REUSE. 2. ALL STOCKPILED TOPSOIL SHALL BE SEEDED, MULCHED WITH HAY, AND ENCLOSED BY A SILTATION FENC
- FILLING OPERATIONS
- 1. PRIOR TO FILLING, ALL SEDIMENT AND EROSION CONTROL DEVICES SHALL BE PROPERLY IMPLEMENTED, FULLY INSTALLED, AS DIRECTED BY THE ENGINEER AND AS SHOWN ON THIS PLAN.
- 2. ALL FILL MATERIAL ADJACENT TO ANY WETLAND AREAS, IF APPLICABLE TO THIS PROJECT, SHALL BE GO LESS THAN 5% FINES PASSING THROUGH A #200 SIEVE (BANK RUN), SHALL BE PLACED IN LIFT THICKNE GREATER THAN THAT SPECIFIED IN PROJECT SPECIFICATIONS AND/OR THE PROJECT GEOTECHNICAL REPO BE COMPACTED TO 95% MAX. DRY DENSITY MODIFIED PROCTOR OR AS SPECIFIED IN THE CONTRACT SPE THE GEOTECHNICAL REPORT.
- 3. AS GENERAL GRADING OPERATIONS PROGRESS, ANY TEMPORARY DIVERSION DITCHES SHALL BE RAISED NECESSARY, TO DIVERT SURFACE RUNOFF TO THE SEDIMENT BASINS OR SEDIMENT TRAPS.

PLACEMENT OF DRAINAGE STRUCTURES, UTILITIES, AND BUILDING CONSTRUCTION OPERATIONS.

1. SILT FENCES SHALL BE INSTALLED AT THE DOWNHILL SIDES OF BUILDING EXCAVATIONS, MUD PUMP DISC UTILITY TRENCH MATERIAL STOCKPILES. STRAW BALES MAY BE USED IF SHOWN ON THE SEDIMENT AND

FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

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	PLANS OR IF DIRECTED BY THE CIVIL ENGINEER. FINAL GRADING AND PAVING OPERATIONS	8. THE CONTRACTOR SHALL KEE JUTE MESH, RIP RAP, ETC.)
STORM DRAINAGE SE FROM SEDIMENT E RUNOFF EROSION	1. ALL INLET AND OUTLET PROTECTION SHALL BE PLACED AND MAINTAINED AS SHOWN ON SEDIMENT AND EROSION CONTROL PLANS AND DETAILS, AND AS DESCRIBED IN SPECIFICATIONS AND AS DESCRIBED HEREIN.	9. PROTECT EXISTING TREES TH ORANGE SAFETY FENCE, OR IN THAT AREA; FENCING SHA
NTER 2022. BY THE CONTRACTOR IIMIZE THE LENGTH OF	<ol> <li>NO CUT OR FILL SLOPES SHALL EXCEED 2:1 EXCEPT WHERE STABILIZED BY ROCK FACED EMBANKMENTS OR EROSION CONTROL BLANKETS, OR JUTE MESH AND VEGETATION. ALL SLOPES SHALL BE SEEDED, AND ANY ROAD OR DRIVEWAY SHOULDER AND BANKS SHALL BE STABILIZED IMMEDIATELY UPON COMPLETION OF FINAL GRADING UNTIL TURF IS ESTABLISHED.</li> </ol>	10. INSTALL PERIMETER SEDIMENT BE CONTAINED WITHIN THE LI BALES, RIBBONS, OR OTHER THE SILT FENCE UNLESS WOF
IMIZE THE LENGTH OF	3. PAVEMENT SUB-BASE AND BASE COURSES SHALL BE INSTALLED OVER AREAS TO BE PAVED AS SOON AS FINAL SUB-GRADES ARE ESTABLISHED AND UNDERGROUND UTILITIES AND STORM DRAINAGE SYSTEMS HAVE BEEN INSTALLED.	11. STONE CONSTRUCTION ENTRA MAINTAINED THROUGHOUT TH
BE REQUIRED TO COUNTY SOILS R SHALL HAVE THE RE ENCOUNTERED	4. AFTER CONSTRUCTION OF PAVEMENT, TOPSOIL, FINAL SEED, MULCH AND LANDSCAPING, REMOVE ALL TEMPORARY SEDIMENT AND EROSION CONTROL DEVICES ONLY AFTER ALL AREAS HAVE BEEN PAVED AND/OR GRASS HAS BEEN WELL ESTABLISHED AND THE SITE IS STABLE AND HAS BEEN INSPECTED AND APPROVED BY THE MUNICIPALITY AND/OR COUNTY SOILS CONSERVATION DISTRICT AND/OR INLAND WETLANDS COMMISSION REVIEWING AGENCY.	VARIOUS PHASES OF CONSTR 12. TOPSOIL SHALL BE STRIPPED HAY BALES OR SILT FENCE A
	INSTALLATION OF SEDIMENTATION AND EROSION CONTROL MEASURES	PLACE FOR MORE THAN ONE 13. SEDIMENT BASINS AND SEDIM CONTRIBUTING TO THE BASIN
ISSION CITY OF ION, CONSTRUCTION	<ul><li>A. DIG A SIX INCH TRENCH ON THE UPHILL SIDE OF THE DESIGNATED FENCE LINE LOCATION.</li><li>B. POSITION THE POST AT THE BACK OF THE TRENCH (DOWNHILL SIDE), AND HAMMER THE</li></ul>	14. COMPLY WITH REQUIREMENTS AND WITH CT DEEP RECORD
AND/OR COUNTY	POST AT LEAST 1.5 FEET INTO THE GROUND. C. LAY THE BOTTOM SIX INCHES OF THE FABRIC INTO THE TRENCH TO PREVENT UNDERMINING BY STORM WATER RUN-OFF.	15. STONE CONSTRUCTION ENTRA SHALL BE MAINTAINED DURIN
TO THE START OF	D. BACKFILL THE TRENCH AND COMPACT.	16. MINIMIZE LAND DISTURBANCES (ONE WEEK MAXIMUM UNSTAE SLOPES AND SWALES WITH L
XITS AND INSTALL ON OFF SITE ROADS. TER OF PROPOSED ON THESE PLANS.	A. BALES SHALL BE PLACED IN A SINGLE ROW, LENGTHWISE, ORIENTED PARALLEL TO THE CONTOUR, WITH ENDS OF ADJACENT BALES TIGHTLY ABUTTING ONE ANOTHER.	SLOPES WITH EROSION CONTR CONSTRUCTION STAGING ARE
ed by the engineer	B. BALES SHALL BE ENTRENCHED AND BACKFILLED. A TRENCH SHALL BE EXCAVATED THE WIDTH OF A BALE AND THE LENGTH OF THE PROPOSED BARRIER TO A MINIMUM DEPTH OF FOUR INCHES. AFTER THE BALES ARE STAKED, THE EXCAVATED SOIL SHALL BE BACKFILLED AGAINST THE BARRIER.	17. MAINTAIN EXISTING PAVED AF 18. SILT FENCE AND OTHER SEDI CONTRACT DRAWINGS AND M
NTROLS AT	C. EACH BALE SHALL BE SECURELY ANCHORED BY AT LEAST TWO (2) STAKES. D. THE GAPS BETWEEN BALES SHALL BE WEDGED WITH STRAW TO PREVENT WATER LEAKAGE.	19. EXCAVATED MATERIAL FROM
OF STORM DRAINAGE	E. THE BARRIER SHALL BE EXTENDED TO SUCH A LENGTH THAT THE BOTTOMS OF THE END BALES ARE HIGHER IN ELEVATION THAN THE TOP OF THE LOWEST MIDDLE BALE, TO ENSURE THAT RUN—OFF WILL FLOW EITHER THROUGH OR OVER THE BARRIER, BUT NOT AROUND IT.	20. INSTALL SILT FENCE ACCORD GROUND. SILT FENCE SHALL ENGINEER. FILTER FABRIC U FURTHER INFORMATION.
EDIMENT AND EROSION AND SEED SLOPES	OPERATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES	21. WHERE INDICATED ON SEDIME THEIR CONDITION DETERIORAT
	I. SILTATION FENCE A. ALL SILTATION FENCES SHALL BE INSPECTED AS A MINIMUM WEEKLY OR AFTER EACH RAINFALL. ALL DETERIORATED FABRIC AND DAMAGED POSTS SHALL BE REPLACED AND PROPERLY REPOSITIONED IN ACCORDANCE WITH THIS PLAN. B. SEDIMENT DEPOSITS SHALL BE REMOVED FROM BEHIND THE FENCE WHEN THEY REACH A MAXIMUM HEIGHT OF ONE FOOT.	TIGHTLY TOGETHER TO PREVE 22. INSTALL TEMPORARY DIVERSION DEWATERING PITS AS SHOWN UPHILL AREAS ARE DETERMIN
ATED EROSION AND/OR TO ELIMINATE THE	II. STRAW BALES A. ALL STRAW BALE RINGS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OR REPLACEMENT SHALL BE PROMPTLY MADE AS NEEDED.	SEDIMENT BASINS WILL REQU 23. DIRECT ALL DEWATERING PUN SEDIMENT BASINS OR GRASS
EVIDENCE THAT EACH ALITY AND/OR COUNTY IICH IS BEING	B. DEPOSITS SHALL BE REMOVED AND CLEANED-OUT IF ONE HALF OF THE ORIGINAL HEIGHT OF THE BALES BECOMES FILLED WITH SEDIMENT.	SYSTEM OR SURFACE WATERS 24. BLOCK END OF STORM SEWER WHEN RAIN IS EXPECTED.
OUNTY SOILS ALL RECEIVING SPOIL	III. SEDIMENT BASINS/SEDIMENT TRAPS A. CONTRACTOR TO KEEP WEEKLY CHECKLIST LOGS FOR INSPECTIONS OF ALL SEDIMENT AND EROSION CONTROL DEVICES AND HAVE THEM READILY AVAILABLE ON-SITE AT ALL TIMES FOR INSPECTION BY CT DEEP, LOCAL AUTHORITIES OR ENGINEER.	25. SWEEP AFFECTED PORTIONS A PROBLEM) DURING CONSTR DOWN DISTURBED AREAS, US
	B. ALL SEDIMENT BASINS AND/OR SEDIMENT TRAPS SHALL BE INSPECTED FOLLOWING EACH RAINFALL. REPAIR OF SLOPES SHALL BE PROMPTLY MADE AS NEEDED.	26. PERIODICALLY CHECK ACCUM CONSTRUCTION AND CLEAN A
S AND OTHER EROSION L BE ON A PERIODIC	C. SEDIMENT DEPOSITS SHALL BE REMOVED FROM SEDIMENT BASINS AND/OR SEDIMENT TRAPS WHEN THEY REACH A MAXIMUM HEIGHT OF ONE FOOT UNLESS OTHERWISE INDICATED ON THE EROSION CONTROL PLANS AND DETAILS TO BE AT A SPECIFIC ELEVATION PER CLEAN OUT MARKERS.	OR PER SPECIFIC CLEANOUT NECESSARY AND AS DIRECTE ACCUMULATED SEDIMENT FRC BALE OR ONE FOOT AT SILT
SION CONTROL SEDIMENT COLLECTED	D. SEDIMENT SHALL BE DISPOSED OF ON-SITE OR AS DIRECTED BY THE ENGINEER AND LOCAL GOVERNING OFFICIALS. SEE SEDIMENT AND EROSION CONTROL NOTES HEREIN REGARDING DISPOSAL REQUIREMENTS FOR OFF SITE SPOIL DISPOSAL. SEDIMENT AND EROSION CONTROL PLAN	27. IMMEDIATELY UPON DISCOVER SEDIMENT POLLUTION, THE O
	1. STRAW BALE FILTERS WILL BE INSTALLED AT ALL CULVERT OUTLETS IF CULVERT OUTLETS ARE APPLICABLE TO THIS PROJECT AND SILTATION FENCE INSTALLED ALONG THE TOE OF ALL CRITICAL CUT AND FILL SLOPES.	POTENTIAL FOR ACCELERATED 28. ALL PUMPING OF SEDIMENT FILTER BAG OR EQUIVALENT
	<ol> <li>CULVERT DISCHARGE AREAS WILL BE PROTECTED WITH RIP RAP CHANNELS. ENERGY DISSIPATORS WILL BE INSTALLED AS SHOWN ON THESE PLANS AND AS NECESSARY.</li> <li>CATCH BASINS WILL BE PROTECTED WITH STRAW BALE FILTERS, SILT SACKS, SILTATION FENCE, OR OTHER INLET</li> </ol>	29. ALL EXCAVATED MATERIAL S ALLOW THE TRENCH TO INTER
	PROTECTION DEVICES PER DETAILS, THROUGHOUT THE CONSTRUCTION PERIOD AND UNTIL ALL DISTURBED AREAS ARE THOROUGHLY STABILIZED.	30. CONTRACTOR SHALL ONLY E BACKFILLED AND STABILIZED
	<ol> <li>ALL SEDIMENT AND EROSION CONTROL MEASURES WILL BE INSTALLED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL LATEST EDITION.</li> <li>SEDIMENT AND EROSION CONTROL MEASURES WILL BE INSTALLED PRIOR TO DEMOLITION AND/OR CONSTRUCTION WHENEVER POSSIBLE.</li> </ol>	31. ANY STOCKPILES OF STRIPP STABILIZE POTENTIALLY WIND NEEDED TO SUPPRESS DUST. AIRBORNE DUST. DURING HI
EED MIXTURE TO BE DR ORDERED FOR DR TEMPORARY	<ol> <li>ALL CONTROL MEASURES WILL BE MAINTAINED IN EFFECTIVE CONDITION THROUGHOUT THE DEMOLITION AND CONSTRUCTION PERIOD UNTIL THE SITE IS DETERMINED TO BE STABILIZED BY THE AUTHORITY HAVING JURISDICTION.</li> </ol>	CEASED IF DUST CANNOT BE 32. AN AREA SHALL BE CONSID PERENNIAL VEGETATIVE COVE
10–10 AT 1.0 LBS. OF	7. ADDITIONAL CONTROL MEASURES WILL BE INSTALLED DURING THE CONSTRUCTION PERIOD, IF NECESSARY OR REQUIRED OR AS DIRECTED BY THE CIVIL ENGINEER OR BY THE AUTHORITY HAVING JURISDICTION.	ACCELERATED SURFACE EROS MOVEMENTS UNLESS OTHERW
	8. SEDIMENT REMOVED FROM EROSION CONTROL STRUCTURES WILL BE DISPOSED IN A MANNER WHICH IS CONSISTENT WITH THE INTENT AND REQUIREMENTS OF THE SEDIMENT AND EROSION CONTROL PLANS, NOTES, AND DETAILS.	33. MAINTAIN ALL PERMANENT AI THROUGHOUT THE CONSTRUC TEMPORARY EROSION AND SE
EBRIS AND SEDIMENT. ND WETLANDS MOVED FOLLOWING	9. OWNER'S CONSTRUCTION REPRESENTATIVE IS ASSIGNED THE RESPONSIBILITY FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE INSTALLATION AND MAINTENANCE OF CONTROL MEASURES, INFORMING ALL PARTIES ENGAGED ON THE CONSTRUCTION SITE OF THE REQUIREMENTS AND OBJECTIVES OF THE PLAN, NOTIFICATION OF THE MUNICIPALITY AND/OR COUNTY SOILS CONSERVATION DISTRICT AND/OR INLAND WETLANDS COMMISSION REVIEWING AGENCY OFFICE OR AUTHORITY HAVING JURISDICTION OF ANY TRANSFER OF THIS RESPONSIBILITY AND FOR CONVEYING A COPY OF THE SEDIMENT AND EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED.	OF TERMINATION) WITH AUTH CONSTRUCTION ACTIVITIES PE
DIMENTATION BASINS RT OF CLEARING AND	SEDIMENT AND EROSION CONTROL NOTES 1. THE SEDIMENT AND EROSION CONTROL PLAN IS ONLY INTENDED TO DESCRIBE THE SEDIMENT AND EROSION CONTROL TREATMENT FOR THIS SITE. SEE SEDIMENT AND EROSION CONTROL DETAILS AND CONSTRUCTION SEQUENCE. REFER TO SITE PLAN FOR GENERAL INFORMATION AND OTHER CONTRACT PLANS FOR APPROPRIATE INFORMATION.	
ALL NOT PROCEED AND APPROVED ALL	2. OWNER'S CONSTRUCTION REPRESENTATIVE IS RESPONSIBLE FOR IMPLEMENTING THIS SEDIMENT AND EROSION CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE PROPER INSTALLATION AND MAINTENANCE OF SEDIMENT AND EROSION CONTROL MEASURES, INFORMING ALL PARTIES ENGAGED WITH CONSTRUCTION ON THE SITE OF THE REQUIREMENTS AND OBJECTIVES	
NOT TO DISTURB	OF THIS PLAN, INFORMING THE AUTHORITY HAVING JURISDICTION OR COUNTY SOILS CONSERVATION DISTRICT OR INLAND WETLANDS AGENCY OF ANY TRANSFER OF THIS RESPONSIBILITY, AND FOR CONVEYING A COPY OF THE SEDIMENT & EROSION CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED.	
IZED WITH TOPSOIL	3. AN EROSION CONTROL BOND MAY BE REQUIRED TO BE POSTED WITH CITY OF BRIDGEPORT TO ENSURE IMPLEMENTATION OF THE SEDIMENT AND EROSION CONTROL MEASURES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE POSTING OF THIS BOND AND FOR INQUIRIES TO THE CITY OF BRIDGEPORT FOR INFORMATION ON THE METHOD, TYPE AND AMOUNT OF THE BOND POSTING UNLESS OTHERWISE DIRECTED BY THE OWNER.	
_ SHALL BE STRIPPED	4. VISUAL SITE INSPECTIONS SHALL BE CONDUCTED WEEKLY, AND AFTER EACH MEASURABLE PRECIPITATION EVENT OF 0.25 INCHES OR GREATER BY QUALIFIED PERSONNEL, TRAINED AND EXPERIENCED IN SEDIMENT AND EROSION CONTROL, TO ASCERTAIN THAT THE SEDIMENT AND EROSION CONTROL (E&S) BMPS ARE OPERATIONAL AND EFFECTIVE IN PREVENTING POLLUTION. A WRITTEN REPORT OF EACH INSPECTION SHALL BE KEPT, AND INCLUDE: A)A SUMMARY OF THE SITE CONDITIONS, E&S BMPS, AND COMPLIANCE; AND B)THE DATE, TIME, AND THE NAME OF THE PERSON CONDUCTING THE INSPECTION	
, MAINTAINED AND	<ul> <li>C) TURBIDITY TESTING AS REQUIRED BY THE GENERAL PERMIT (NPDES).</li> <li>5. THE CONTRACTOR SHALL CONSTRUCT ALL SEDIMENT AND EROSION CONTROLS IN ACCORDANCE WITH 2002 CONNECTICUT GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL LATEST EDITION IN ACCORDANCE WITH THE CONTRACT DOCUMENTS,</li> </ul>	
GOOD QUALITY, WITH KNESSES NOT EPORT. LIFTS SHALL SPECIFICATIONS OR IN	AND AS DIRECTED BY THE MUNICIPALITY AND/OR COUNTY SOILS CONSERVATION DISTRICT AND/OR INLAND WETLANDS COMMISSION REVIEWING AGENCY. THE CONTRACTOR SHALL KEEP A COPY OF THE GUIDELINES ON-SITE FOR REFERENCE DURING CONSTRUCTION.	
D OR LOWERED, AS	6. ADDITIONAL AND/OR ALTERNATIVE SEDIMENT AND EROSION CONTROL MEASURES MAY BE INSTALLED DURING THE CONSTRUCTION PERIOD IF FOUND NECESSARY BY THE CONTRACTOR, OWNER, SITE ENGINEER, MUNICIPALITY AND/OR COUNTY SOILS CONSERVATION DISTRICT AND/OR INLAND WETLANDS COMMISSION REVIEWING AGENCY, OR GOVERNING AGENCIES. THE CONTRACTOR SHALL CONTACT THE OWNER AND APPROPRIATE GOVERNING AGENCIES FOR APPROVAL IF ALTERNATIVE CONTROLS OTHER THAN THOSE SHOWN ON THE PLANS ARE PROPOSED.	
ISCHARGES, AND D EROSION CONTROL	<ol> <li>THE CONTRACTOR SHALL INSPECT ALL SEDIMENT AND EROSION CONTROLS BEFORE AND AFTER EACH STORM (0.25 INCHES OR GREATER RAINFALL), OR AT LEAST WEEKLY, TO VERIFY THAT THE CONTROLS ARE OPERATING PROPERLY AND MAKE REPAIRS WHERE NECESSARY.</li> </ol>	

KEEP A SUPPLY OF SEDIMENT AND EROSION CONTROL MATERIAL (STRAW BALES, SILT FENCE, S.) ON-SITE FOR MAINTENANCE AND EMERGENCY REPAIRS.

THAT ARE TO BE SAVED BY FENCING AT THE DRIP LINE OR AS SHOWN WITH SNOW FENCE, I'R EQUIVALENT FENCING. ANY LIMB TRIMMING SHOULD BE DONE BEFORE CONSTRUCTION BEGINS SHALL BE MAINTAINED AND REPAIRED DURING CONSTRUCTION.

ENT AND EROSION CONTROLS PRIOR TO CLEARING OR CONSTRUCTION. ALL CONSTRUCTION SHALL LIMIT OF DISTURBANCE, WHICH SHALL BE MARKED WITH SILT FENCE, SAFETY FENCE, STRAW R MEANS PRIOR TO CLEARING. CONSTRUCTION ACTIVITY SHALL REMAIN ON THE UPHILL SIDE OF WORK IS SPECIFICALLY CALLED FOR ON THE DOWNHILL SIDE OF THE FENCE.

RANCE ANTI-TRACKING PADS SHALL BE INSTALLED AT START OF CONSTRUCTION AND THE DURATION OF CONSTRUCTION. THE LOCATION OF THE TRACKING PADS MAY CHANGE AS STRUCTION ARE COMPLETED.

ED AND STOCKPILED FOR USE IN FINAL LANDSCAPING. ALL EARTH STOCKPILES SHALL HAVE E AROUND THE LIMIT OF PILE. PILES SHALL BE TEMPORARILY SEEDED IF PILE IS TO REMAIN IN NE (1) MONTH.

DIMENT TRAPS SHALL PROVIDE 134 CUBIC YARDS OF SEDIMENT STORAGE PER ACRE SIN. PROVIDE BASIN VOLUMES FOR ALL DISTURBANCE ON SITE.

TS OF <u>CGS SECTION 22A 430B</u>, FOR STORMWATER DISCHARGE FROM CONSTRUCTION ACTIVITIES D KEEPING AND INSPECTION REQUIREMENTS.

RANCE ANTI-TRACKING PADS SHALL BE INSTALLED PRIOR TO ANY ON SITE EXCAVATION AND RING ALL DEMOLITION, EXCAVATION AND CONSTRUCTION ACTIVITIES.

CES. SEED AND MULCH DISTURBED AREAS WITH TEMPORARY MIX AS SOON AS PRACTICABLE TABILIZED PERIOD) USING PERENNIAL RYEGRASS AT 40 LBS PER ACRE. MULCH ALL CUT AND FILL LOOSE STRAW AT A RATE OF 2 TONS PER ACRE. IF NECESSARY, REPLACE LOOSE HAY ON NTROL BLANKETS OR JUTE CLOTH. MODERATELY GRADED AREAS, ISLANDS, AND TEMPORARY REAS MAY BE HYDROSEEDED WITH TACKIFIER.

AREAS FOR CONSTRUCTION STAGING FOR AS LONG AS POSSIBLE.

EDIMENT AND EROSION CONTROL MEASURES SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS PRIOR TO WORK IN ANY UPLAND AREAS. OM TEMPORARY SILT TRAPS MUST BE STOCKPILED ON UPHILL SIDE OF SILT FENCE.

RDING TO MANUFACTURER'S INSTRUCTION, PARTICULARLY, BURY LOWER EDGE OF FABRIC INTO L BE TENCATE ENVIROFENCE, PROPEX GEOTEX OR EQUIVALENT APPROVED BY THE CIVIL USED SHALL BE TENCATE 140N OR 170N, OR APPROVED EQUIVALENT. SEE SPECIFICATIONS FOR

MENT AND EROSION CONTROL PLANS USE NEW STRAW BALES AND REPLACE THEM WHENEVER ATES BEYOND REASONABLE USABILITY. STAKE BALES SECURELY INTO GROUND AND BUTT EVENT UNDERCUTTING AND BYPASSING.

RSION DITCHES, PLUNGE POOLS, SEDIMENT BASINS, SEDIMENT TRAPS, CONCRETE WASH PITS AND WN AND AS NECESSARY DURING VARIOUS PHASES OF CONSTRUCTION TO CONTROL RUNOFF UNTIL MINED TO BE STABILIZED BY THE AUTHORITY HAVING JURISDICTION. LOCATION OF TEMPORARY QUIRE REVIEW AND APPROVAL BY THE CIVIL ENGINEER AND AUTHORITY HAVING JURISDICTION.

UMP DISCHARGE TO A SEDIMENT CONTROL DEVICE SUCH AS TEMPORARY PITS, SEDIMENT TRAP, SS FILTERS WITHIN THE APPROVED LIMIT OF DISTURBANCE. DISCHARGE TO STORM DRAINAGE ERS FROM SEDIMENT CONTROLS SHALL BE CLEAR.

WERS IN EXPOSED TRENCHES WITH BOARDS AND SANDBAGS AT THE END OF EACH WORKING DAY

S OF OFF SITE ROADS ONE OR MORE TIMES A DAY (OR LESS FREQUENTLY IF TRACKING IS NOT STRUCTION. OTHER DUST CONTROL MEASURES TO BE USED AS NECESSARY INCLUDE WATERING USING CALCIUM CHLORIDE, AND COVERING LOADS ON DUMP TRUCKS.

JMULATED SEDIMENT LEVELS IN THE SEDIMENT BASINS AND SEDIMENT TRAPS DURING A ACCUMULATED SILT WHEN NECESSARY OR WHEN ONE FOOT OF SEDIMENT HAS ACCUMULATED JT MARKER ELEVATION. CLEAN ACCUMULATED SEDIMENT FROM CATCH BASIN SUMPS AS CTED BY THE CIVIL ENGINEER OR OWNER'S CONSTRUCTION REPRESENTATIVE. REMOVE ROM BEHIND STRAW BALES AND SILT FENCE WHEN LEVEL REACHES HALF THE HEIGHT OF THE LT FENCE. DISPOSE OF SEDIMENT LEGALLY EITHER ON OR OFF SITE.

ERING UNFORESEEN CIRCUMSTANCES POSING THE POTENTIAL FOR ACCELERATED EROSION AND/OR OPERATOR SHALL IMPLEMENT APPROPRIATE BEST MANAGEMENT PRACTICES TO ELIMINATE THE TED EROSION AND/OR SEDIMENT POLLUTION.

NT LADEN WATER SHALL BE THROUGH A SEDIMENT CONTROL BMP, SUCH AS A PUMPED WATER IT SEDIMENT REMOVAL FACILITY, OVER UNDISTURBED VEGETATED AREAS.

L SHALL BE PLACED ON THE HIGH SIDE OF UTILITY AND STORM PIPE TRENCHES SO AS TO TERCEPT ALL SILT LADEN RUNOFF.

Y EXCAVATE AS MUCH UTILITY AND STORM PIPE TRENCH WORK AS CAN BE COMPLETED, ED IN ONE DAY SO AS TO LIMIT THE AMOUNT OF OPEN, DISTURBED TRENCHING.

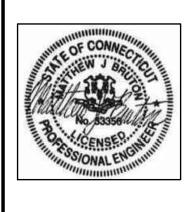
PPED MATERIALS ARE TO BE PERIODICALLY SPRAYED WITH WATER OR A CRUSTING AGENT TO ND-BLOWN MATERIAL. HAUL ROADS BOTH INTO AND AROUND THE SITE ARE TO BE SPRAYED AS ST. TRUCKS HAULING IMPORT FILL MATERIAL ARE TO BE TARPED TO AID IN THE CONTROL OF HIGH WIND EVENTS (20 TO 30 MPH SUSTAINED) CONSTRUCTION ACTIVITY SHALL BE LIMITED OR BE CONTROLLED BY WETTING.

SIDERED TO HAVE ACHIEVED FINAL STABILIZATION WHEN IT HAS A MINIMUM OF 70% UNIFORM IVER OR OTHER PERMANENT NON-VEGETATIVE COVER WITH A DENSITY SUFFICIENT TO RESIST ROSION AND SUBSURFACE CHARACTERISTICS SUFFICIENT TO RESIST SLIDING OR OTHER RWISE DETERMINED BY THE AUTHORITY HAVING JURISDICTION.

AND TEMPORARY EROSION AND SEDIMENT CONTROL DEVICES IN EFFECTIVE CONDITION UCTION PERIOD. UPON COMPLETION OF WORK SWEEP PARKING LOT AND REMOVE ALL SEDIMENT CONTROLS WHEN AUTHORIZED BY AUTHORITY HAVING JURISDICTION. FILE NOT (NOTICE THORITY HAVING JURISDICTION RESPONSIBLE FOR REGULATING STORM WATER DISCHARGES FROM PER NPDES.



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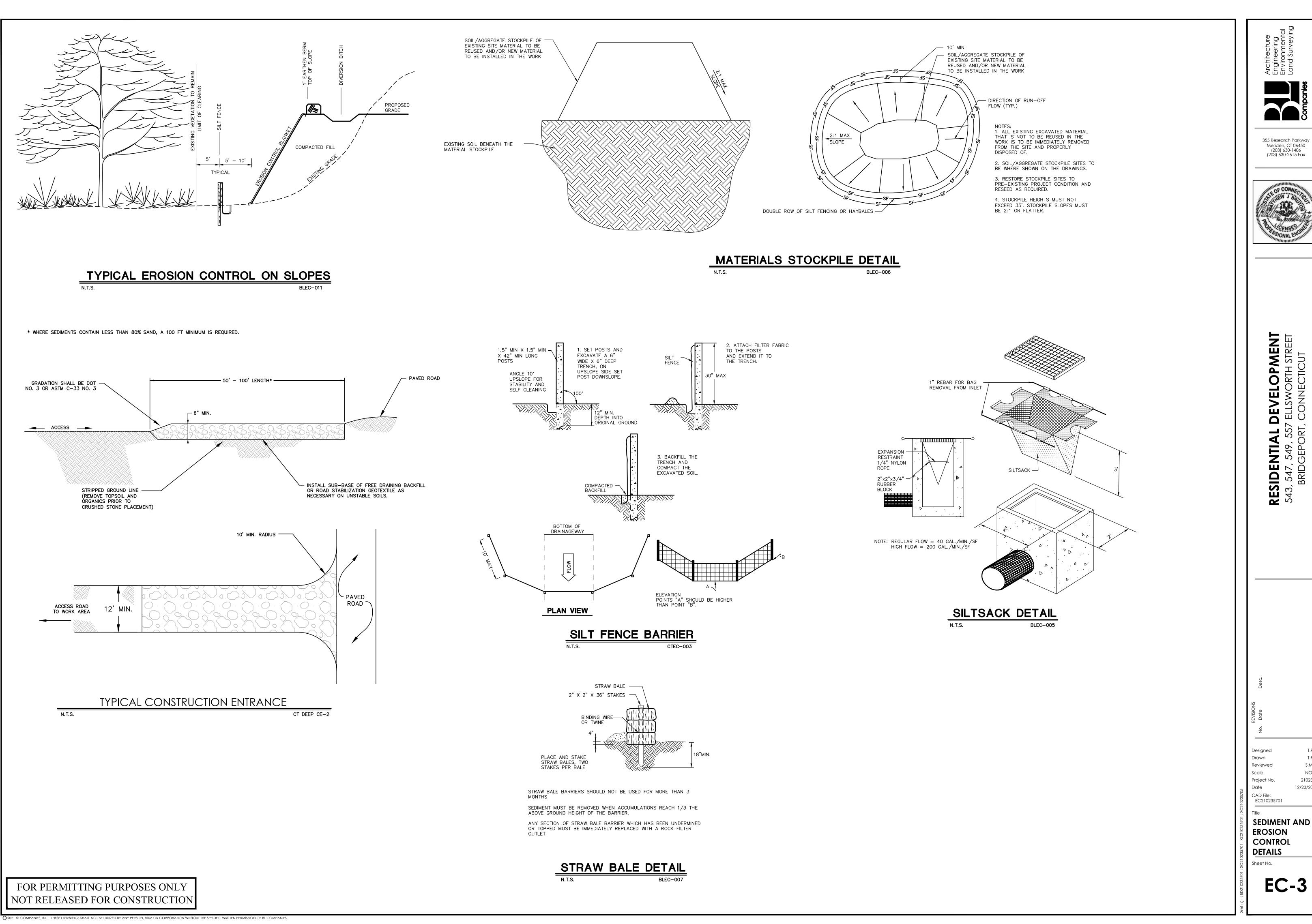
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NOTES

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RESIDENTIAL DEVELOPMENT	543, 547, 549, 557 ELLSWORTH STREET BRIDGEPORT, CONNECTICUT
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TREES					
KEY	QTY	BOTANICAL NAME	COMMON NAME	ROOT	SIZE
СВ	5	CARPINUS BETULUS 'FASTIGIATA'	PYRAMIDAL EUROPEAN HORNBEAM	B&B	2"—2.5" CAL
PA	1	PLATANUS X ACERIFOLIA 'BLOODGOOD'	BLOODGOOD PLANETREE	B&B	2"—2.5" CAL
то	24	THUJA OCCIDENTALIS 'NIGRA'	DARK AMERICAN ARBORVITAE	B&B	6'—7' HT.
тс	0	TILIA CORDATA 'GREENSPIRE'	GREENSPIRE LITTLE-LEAF LINDEN	B&B	2"-2.5" CAI
SHRUB	s and	GRASSES			
CA	24	CALAMAGROSTIS X ACUTIFLORA 'KARL FOERSTER'	FEATHER REED GRASS	CONT.	24"—30" HT
IG	24	ILEX GLABRA	INKBERRY	CONT.	24"-30" H1

NOTES: 1) ALL SUBSTITUTIONS MUST RECEIVE APPROVAL FROM THE LANDSCAPE ARCHITECT PRIOR TO DELIVERY TO SITE.

2) PROVIDE AND INSTALL ALL PLANTS SHOWN ON THE PLANTING PLAN DRAWINGS; THE QUANTITIES IN THE PLANT LIST ARE PROVIDED FOR THE CONTRACTOR'S CONVENIENCE ONLY. IF DISCREPANCIES OCCUR, THE LARGER QUANTITY SHALL APPLY.

3) IF THERE IS A DISCREPANCY BETWEEN BOTANICAL AND COMMON NAME, BOTANICAL NAME PREVAILS.

## LEGEND

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# PROVIDE AND INSTALL CANOPY TREE

PROVIDE AND INSTALL EVERGREEN TREE

PROVIDE AND INSTALL SHRUB

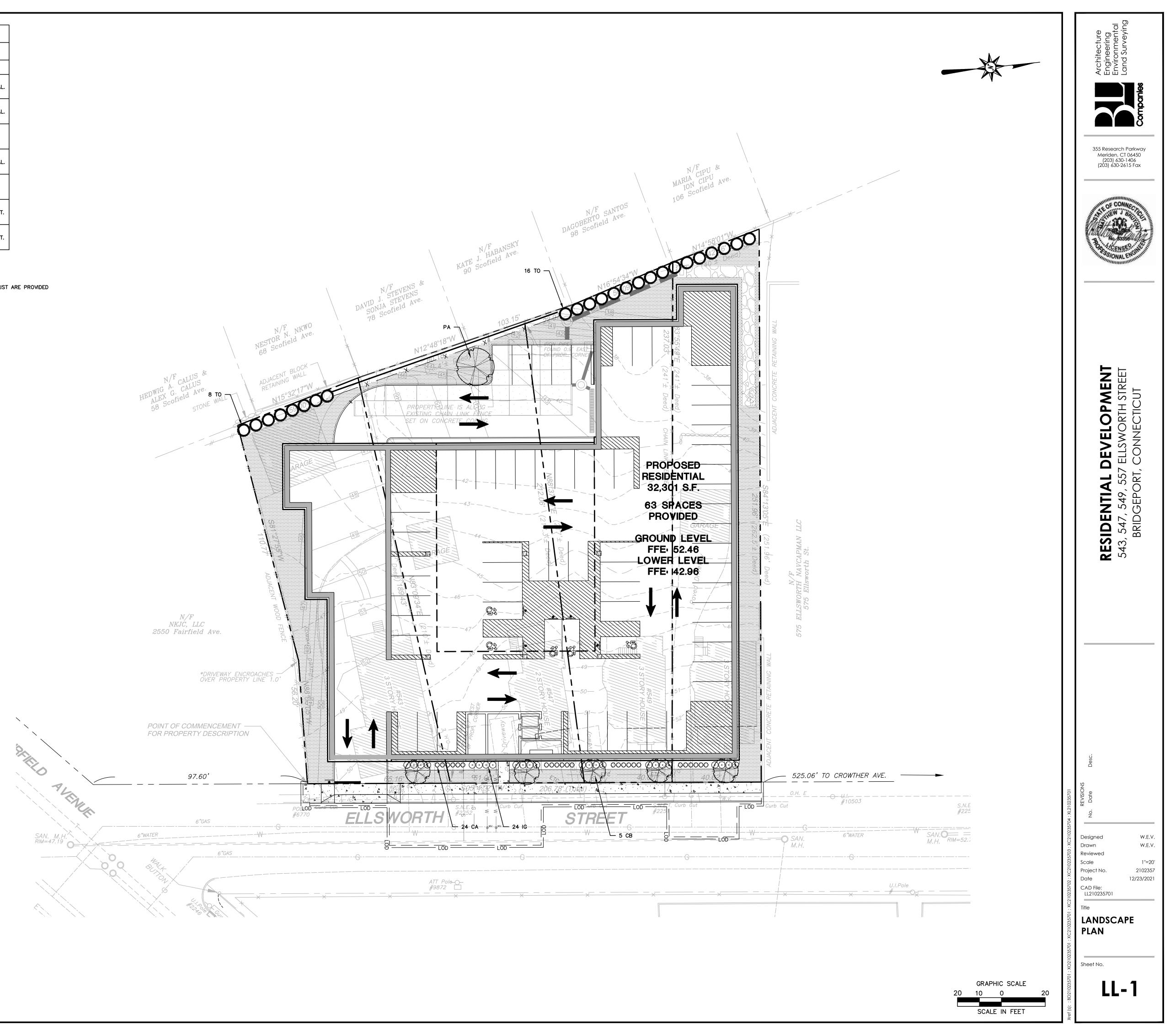
PROVIDE AND INSTALL GRASS

PROVIDE AND INSTALL SOD

## SEE SHEET LL-2 FOR LANDSCAPE NOTES AND DETAILS

## FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

🗘 2021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.



LANDSCAPEN	NOTES
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THE LANDSCAPE PLAN AND DETAIL SHEET ARE FOR LANDSCAPING INFORMATION ONLY. REFER TO THE OTHER PLANS FOR ALL OTHER INFORMATION.

2. COORDINATE PLANT MATERIAL LOCATIONS WITH SITE UTILITIES. UTILITY LOCATIONS SHOWN ON THE DRAWINGS ARE APPROXIMATE. EXERCISE CARE WHEN DIGGING IN AREAS OF POTENTIAL CONFLICT WITH UNDERGROUND OR OVERHEAD UTILITIES. THE CONTRACTOR IS RESPONSIBLE FOR ANY DAMAGE DUE TO CONTRACTOR'S NEGLIGENCE AND SHALL REPLACE OR REPAIR ANY DAMAGE AT CONTRACTOR'S EXPENSE. PRIOR TO DIGGING AND INSTALLATION OF PLANT MATERIAL, THE CONTRACTOR SHALL CONTACT "CALL BEFORE YOU DIG" 72 HOURS BEFORE COMMENCEMENT OF WORK AT "(800)922-4455" AND VERIFY ALL UTILITY SYSTEM LOCATIONS.

3. THE LOCATIONS FOR PLANT MATERIAL ARE APPROXIMATE AND ARE SUBJECT TO FIELD ADJUSTMENT DUE TO UTILITY LOCATIONS AND SITE CONDITIONS. THE CONTRACTOR SHALL ACCURATELY STAKE OUT THE LOCATIONS FOR ALL PLANTS FOR THE REVIEW, ADJUSTMENT, AND APPROVAL BY OWNER OR LANDSCAPE ARCHITECT PRIOR TO PLANTING.

4. THE CONTRACTOR SHALL GUARANTEE THAT ALL PLANTS SHALL BE HEALTHY AND FREE OF DISEASE FOR A PERIOD OF ONE YEAR OR JUNE 1ST OF THE YEAR FOLLOWING INSTALLATION. WHICHEVER IS LONGER, AFTER SUBSTANTIAL COMPLETION AND ACCEPTANCE BY OWNER OR LANDSCAPE ARCHITECT. CONTRACTOR SHALL REPLACE ANY DEAD OR UNHEALTHY PLANTS AT CONTRACTOR'S EXPENSE. PLANT MATERIAL REPLACEMENTS SHALL BE GUARANTEED FOR ONE FULL YEAR FROM DATE OF REPLACEMENT. REPLACEMENT PLANTS SHALL BE THE SAME AS SPECIFIED FOR THE ORIGINAL PLANTING. REPLACEMENTS SHALL BE MADE AS MANY TIMES AS NECESSARY TO ENSURE HEALTHY PLANTS. FINAL ACCEPTANCE SHALL BE MADE IF ALL PLANTS MEET THE GUARANTEE REQUIREMENTS INCLUDING MAINTENANCE. MAINTENANCE RESPONSIBILITIES INCLUDE CULTIVATING, SPRAYING, WEEDING, WATERING, TIGHTENING GUYS, PRUNING, FERTILIZING, MULCHING, AND ANY OTHER OPERATIONS NECESSARY TO MAINTAIN PLANT VIABILITY, MAINTENANCE SHALL BEGIN IMMEDIATELY AFTER PLANTING AND CONTINUE UNTIL THE END OF THE GUARANTEE PERIOD. DURING THE LANDSCAPE MAINTENANCE PERIOD (GUARANTEE) THE LANDSCAPE CONTRACTOR SHALL NOTIFY THE OWNER IN WRITING OF ANY SITE CONSTRAINTS (PHYSICAL, ENVIRONMENT, ETC.) OR MAINTENANCE DEFICIENCIES THAT MAY AFFECT LANDSCAPE VEGETATION ESTABLISHMENT.

THE CONTRACTOR SHALL SUPPLY ALL LABOR, PLANTS, AND MATERIALS IN QUANTITIES SUFFICIENT TO COMPLETE THE WORK SHOWN ON THE DRAWINGS AND LISTED IN THE PLANT SCHEDULE. IN THE EVENT OF A DISCREPANCY BETWEEN QUANTITIES SHOWN IN THE PLANT SCHEDULE AND THOSE REQUIRED BY THE DRAWINGS, THE LARGER SHALL APPLY. ALL PLANTS SHALL BE ACCLIMATED BY THE SUPPLY NURSERY TO THE LOCAL HARDINESS ZONE AND BE CERTIFIED THAT THE PLANTING MATERIAL HAS BEEN GROWN FOR A MINIMUM OF TWO YEARS AT THE SOURCE AND OBTAINED WITHIN 200 MILES OF PROJECT SITE UNLESS OTHERWISE APPROVED BY OWNER OR LANDSCAPE ARCHITECT.

6. PLANTS SHALL HAVE TAGS THAT IDENTIFY PLANT GENUS, SPECIES, CULTIVAR (IF APPLICABLE), PLANT COMMON NAME, NAME OF SOURCE NURSERY, AND SIZE OF PLANT FOR REVIEW OF OWNER OR LANDSCAPE ARCHITECT.

7. NO PLANT SHALL BE PLACED IN THE GROUND BEFORE ROUGH GRADING HAS BEEN COMPLETED AND APPROVED BY THE OWNER OR LANDSCAPE ARCHITECT. STAKING THE LOCATION OF ALL TREES AND SHRUBS SHALL BE COMPLETED PRIOR TO PLANTING FOR APPROVAL BY THE OWNER OR LANDSCAPE ARCHITECT.

8. FINAL GRADES SHALL BLEND SMOOTHLY WITH EXISTING GRADES, AND TOP AND BOTTOM OF SLOPES SHALL BE ROUNDED.

9. ALL TREE AND SHRUB MASSINGS SHALL BE MULCHED TO A DEPTH OF 3". ANNUAL AND PERENNIAL BEDS SHALL BE MULCHED TO A DEPTH OF 2". MULCH SHALL BE UNCOLORED TRIPLE-SHREDDED HARDWOOD BARK MULCH, AGED AT LEAST 6 MONTHS.

10. IF TREE STAKING IS PROPOSED, TREE STAKING MUST BE COMPLETED THE SAME DAY AS THE TREE IS INSTALLED. ALL TREES SHALL BE STAKED OR GUYED PER DETAIL.

11. LANDSCAPE PLANTING AREAS MUST BE FREE DRAINING. PAVEMENT, COMPACTED SUBGRADE, DEAD OR DYING PLANT MATERIAL, BLASTED ROCK, STONES GREATER THAN 1" IN DIAMETER, AND ANY OTHER MATERIAL HARMFUL TO PLANT GROWTH AND DEVELOPMENT SHALL BE REMOVED FROM AREAS TO BE LANDSCAPED AS REQUIRED BY PLANTING DETAILS OR SPECIFICATIONS.

12. PLANTING SOIL:

DEPTH: PLANTING SOIL SHALL BE INSTALLED AT A MINIMUM DEPTH OF 4" OR AS NOTED IN THE LANDSCAPE DETAILS. PLANTING SOIL SHALL BE UTILIZED IN ALL PLANTING AREAS INCLUDING SEEDED AREAS.

TESTING: CONTRACTOR SHALL SUBMIT (2) SOIL SAMPLES PER SOIL STOCKPILE TO A CERTIFIED TESTING LABORATORY TO DETERMINE ACIDITY, ORGANIC CONTENT, MECHANICAL ANALYSIS, AVAILABLE NUTRIENTS (N,P,K,Ca,Mg,S,Fe,Mn,Zn,Cu,B,AI,Pb) AND NECESSARY AMENDMENTS TO SOIL. THE CONTRACTOR SHALL SUBMIT THE TEST RESULTS TO THE OWNER OR LANDSCAPE ARCHITECT FOR REVIEW AND APPROVAL. TEST RESULTS SHALL RECOMMEND AMENDMENTS THAT WILL ALTER THE SOIL CHARACTERISTICS SUCH THAT THE CHARACTERISTICS DESCRIBED BELOW ARE ACHIEVED AND THE SPECIFIED PLANTS (CONTRACTOR TO PROVIDE LIST TO TESTING LABORATORY) WILL ACHIEVE PROPER GROWTH THAT IS NEITHER DEFICIENT NOR EXCESSIVE. THE CONTRACTOR SHALL INCORPORATE THESE AMENDMENTS AT NO INCREASE IN CONTRACT PRICE.

CHARACTERISTICS: PLANTING SOIL MAY CONSIST OF EXISTING ON-SITE SOILS, AMENDED ON-SITE SOILS, OR IMPORTED SOILS MEETING THE FOLLOWING CRITERIA:

- A. NOT TO CONTAIN MATERIALS HARMFUL TO PLANT LIFE, TO BE CLEAN, FERTILE, FRIABLE, AND WELL DRAINING. ALL PLANTING SOIL SHALL BE FREE OF ANY SUBSOIL EARTH CLODS, SODS, STONES OVER 1" IN ANY DIMENSION, STICKS, ROOTS, WEEDS, LITTER AND OTHER DELETERIOUS MATERIAL. PLANTING SOIL SHALL BE UNIFORM IN QUALITY AND TEXTURE.
- B. PLANTING SOIL SHALL HAVE THE FOLLOWING OPTIMUM RANGES UNLESS OTHERWISE APPROVED BY THE OWNER OR LANDSCAPE ARCHITECT 3% - 6% FOR LAWN OR GRASS AREAS. ORGANIC CONTENT

4% - 8% FOR TREE AND SHRUB PLANTERS 8%-16% FOR RETENTION OR DETENTION BASINS.

(BY LOSS OF IGNITION AT 375 C METHOD OF TESTING)

- 6.0 7.3 C. NUTRIENT LEVELS SHALL BE ACHIEVED BY THE CONTRACTOR'S ADDITION OF AMENDMENTS TO THE PLANTING SOIL TO MEET THE OPTIMUM NUTRIENT LEVELS SPECIFIED IN THE TESTING LABORATORY REPORT FOR EACH OF PLANTS TO BE INSTALLED.
- SOIL SHALL BE COMPACTED TO A SURFACE PENETRATION RESISTANCE OF 75-125 LBS/SQ.IN. SOIL MAY BE TREATED FOR WEEDS WITH PRE-EMERGENT OR POST-EMERGENT HERBICIDE, AS NEEDED AND AS APPROPRIATE FOR THE APPLICATION SEASON OR LOCATION, OR ELIMINATE GROWTH OF UNWANTED PLANT MATERIAL. APPLY HERBICIDES IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS. HERBICIDE APPLICATOR MUST BE LICENSED IN THE STATE OF CONNECTICUT, AND PERFORM APPLICATIONS IN ACCORDANCE WITH LOCAL REQUIREMENTS, PERMITTING STIPULATIONS, AND ANY OTHER RESTRICTIONS INCLUDING AND IN EXCESS OF STATE AND FEDERAL REGULATIONS. F. PROPOSED TOPSOIL SHALL MEET THE USDA SOILS TEXTURAL PERCENTAGES OF SAND, SILT, AND CLAY FOR FOLLOWING CLASSIFICATIONS: – LOAM
  - SANDY LOAM WHERE SAND DOES NOT EXCEED 70% AND CLAY IS NOT LESS THAN 5%. - SANDY CLAY LOAM WHERE SAND DOES NOT EXCEED 70% AND CLAY IS LESS THAN 28%.
- G. BIORETENTION SOILS: SOIL TO BE INSTALLED IN RETENTION BASINS, PONDS, OR OTHER STORMWATER MANAGEMENT ENVIRONS SHALL MEET THE ABOVE DESCRIBED CHARACTERISTICS AND AS FOLLOWS: - SOIL SHALL NOT CONTAIN MORE THAN 20% CLAY AND LESS THAN 40% SILT.
- SOIL SHALL HAVE AN INFILTRATION RATE BETWEEN 1/2" AND 3" PER HOUR. H. MODIFICATION TO THE PLANTING SOIL CHARACTERISTICS DESCRIBED ABOVE MAY BE SUBMITTED FOR APPROVAL BY THE LANDSCAPE ARCHITECT. CONTRACTOR MUST DEMONSTRATE PROPOSED CHARACTERISTICS ARE EQUAL TO OR SUPERIOR TO THE SPECIFIED
- CHARACTERISTICS WITH RESPECT TO SUPPORTING PLANT GROWTH, AND STORMWATER MANAGEMENT 12. PLANTING AMENDMENTS:

APPLY FERTILIZER AND OTHER AMENDMENTS AS RECOMMENDED FOR EACH PLANTING AREA BY SOIL ANALYSIS. APPLY AMENDMENTS IN A MANNER CONSISTENT WITH MANUFACTURER'S RECOMMENDATIONS. ANY ORGANIC AMENDMENTS SHALL HAVE A pH BETWEEN 4.5 AND 5.5 UNLESS OTHERWISE RECOMMENDED

13. PLANT REQUIREMENTS: ALL PLANTS SHALL CONFORM IN SIZE AND GRADE TO THE AMERICAN STANDARD FOR NURSERY STOCK, ANSI Z60.1 (LATEST EDITION). ALL PLANTS SHALL MEET THE ADDITIONAL REQUIREMENTS SET FORTH BELOW AND IN WRITTEN SPECIFICATIONS AS APPLICABLE. ALL TREES AND SHRUBS SHALL HAVE BEEN GROWN AT A COMMERCIAL NURSERY WITHIN 200 MILES OF THE PROJECT SITE UNLESS OTHERWISE APPROVED BY OWNER OR LANDSCAPE ARCHITECT. THEY SHALL BE TYPICAL OF THEIR SPECIES OR VARIETY. THEY SHALL BE HEALTHY, SYMMETRICAL, EVENLY AND DENSELY BRANCHED, AND DENSELY FOLIATED WHEN IN LEAF. THEY SHALL BE FREE OF BARK INJURY, DISEASE, AND INSECT PESTS. ALL TREES SHALL HAVE A STRAIGHT TRUNK WITH A SINGLE MAIN LEADER UNLESS OTHERWISE CHARACTERISTIC OF THE SPECIES OR VARIETY. THE OWNER OR LANDSCAPE ARCHITECT WILL ALLOW SUBSTITUTIONS ONLY UPON WRITTEN APPROVAL. SIZES SHALL CONFORM TO THE MEASUREMENT SPECIFIED ON THE DRAWINGS. PLANTS LARGER THAN SPECIFIED MAY BE USED IF APPROVED. BUT THE USE OF SUCH PLANTS SHALL NOT INCREASE THE CONTRACT PRICE. ALL OVERSTORY TREES PLANTED ALONG PARKING AREAS, SIDEWALKS AND PEDESTRIAN ACCESSES SHALL NOT BRANCH BELOW 7' FEET IF THE TREE CALIPER IS 3" INCHES OR GREATER. ALL PLANT MATERIALS ARE SUBJECT TO INSPECTION AND ACCEPTANCE BY THE OWNER OR LANDSCAPE ARCHITECT AT THE NURSERY SOURCE. THE CONTRACTOR SHALL COORDINATE SOURCE VISITS WITH THE LANDSCAPE ARCHITECT AND SHALL ACCOMPANY THE OWNER AND/OR LANDSCAPE ARCHITECT FOR ALL INSPECTIONS. CERTIFICATES OF COMPLIANCE WITH SPECIFICATIONS ARE REQUIRED FOR ALL PLANTS.

14. INSPECTION AND REVIEW:

ALL PLANT MATERIAL SHALL BE SUBJECT TO INSPECTION AND ACCEPTANCE BY THE OWNER OR LANDSCAPE ARCHITECT AT THE NURSERY SOURCE OR PLACE OF GROWTH. THE CONTRACTOR SHALL COORDINATE WITH THE LANDSCAPE ARCHITECT ON A SCHEDULE FOR SOURCE VISITS AND ACCOMPANY THE OWNER OR LANDSCAPE ARCHITECT FOR ALL SOURCE INSPECTIONS. CERTIFICATES OF COMPLIANCE ARE REQUIRED FOR ALL PLANT MATERIALS. PHOTOGRAPHIC REVIEW OF PLANT MATERIAL IS ACCEPTABLE IF APPROVED BY LANDSCAPE ARCHITECT. PHOTOGRAPHS MUST BE PROVIDED IN QUANTITY AND VARIETY TO ALLOW LANDSCAPE ARCHITECT SUFFICIENT INFORMATION TO MAKE A REASONABLE DETERMINATION AS TO THE PLANTS' QUALITY. OWNER AND LANDSCAPE ARCHITECT RESERVES THE RIGHT TO REJECT PLANT MATERIAL DELIVERED TO THE SITE BUT PREVIOUSLY ACCEPTED IF DAMAGED OR NOT PROPERLY MAINTAINED DURING THE DELIVERY PROCESS.

15. PLANTING SEASONS (UNLESS OTHERWISE APPROVED BY THE OWNER OR LANDSCAPE ARCHITECT)

SERING	
APRIL 1 TO JUNE 15	SEPTEMBER 1 TO OCTOBER 15
APRIL 1 TO JUNE 15	SEPTEMBER 15 TO NOVEMBER 15
APRIL 1 TO JUNE 15	SEPTEMBER 1 TO OCTOBER 15
MAY 15 TO JUNE 15	SEPTEMBER 1 TO OCTOBER 15
PER MANUFACTURERS RECOMM	MENDATIONS OR AS LISTED IN SEED MIX NOTES
	APRIL 1 TO JUNE 15 APRIL 1 TO JUNE 15 APRIL 1 TO JUNE 15 MAY 15 TO JUNE 15

16. SEEDING MIXTURES: REFER TO SEED MIX NOTES. SEEDED AREA SHALL BE ACCEPTED WHEN SEED AREA ACHIEVES 90% COVERAGE.

17. ALL SLOPES STEEPER THAN 3:1 RECEIVING A SEED MIX SHALL BE COVERED WITH AN EROSION CONTROL BLANKET OF STRAW FIBER AND BIODEGRADABLE OR PHOTODEGRADABLE NETTING.

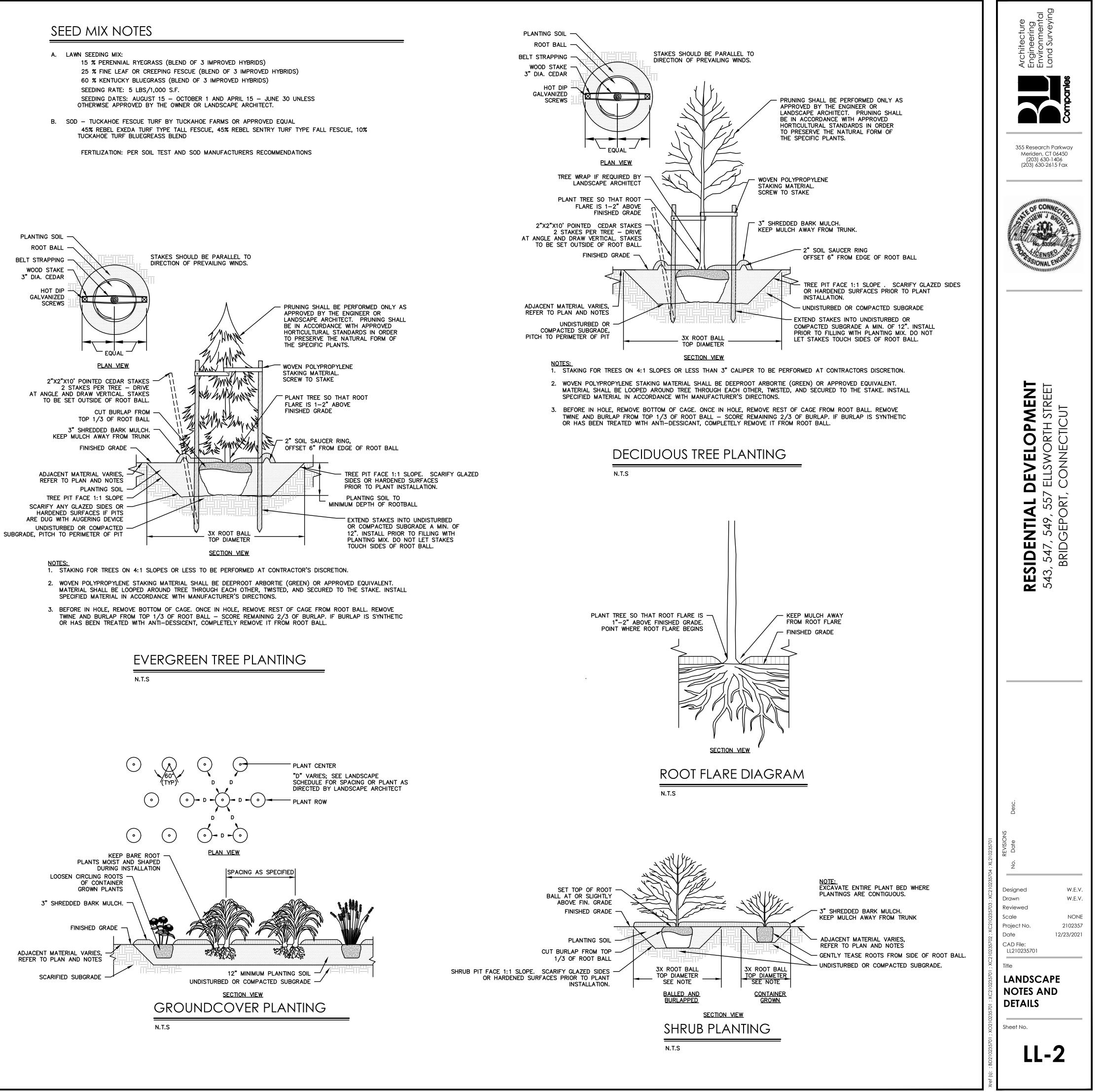
18. UNLESS OTHERWISE NOTED IN DRAWING SET, NEW TREELINES SHALL EQUAL CLEARING AND GRUBBING LIMIT FOR CONSTRUCTION.

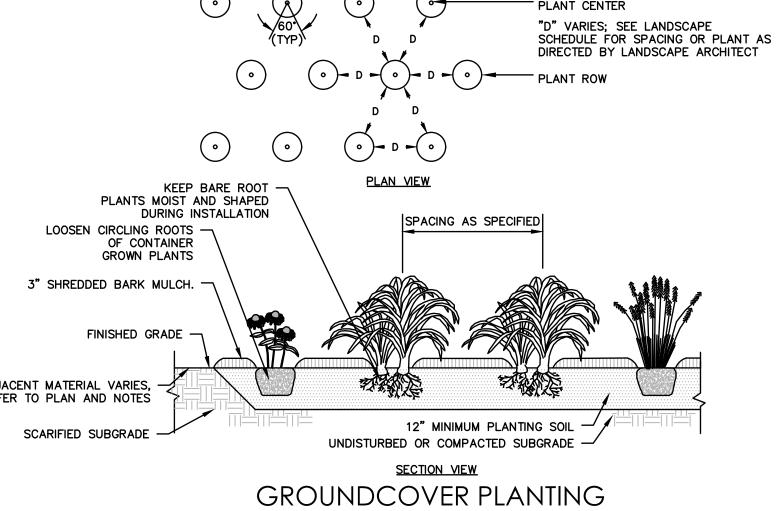
19. ALL DISTURBED AREAS NOT OTHERWISE DEVELOPED SHALL BE SEEDED WITH THE LAWN SEED MIX.

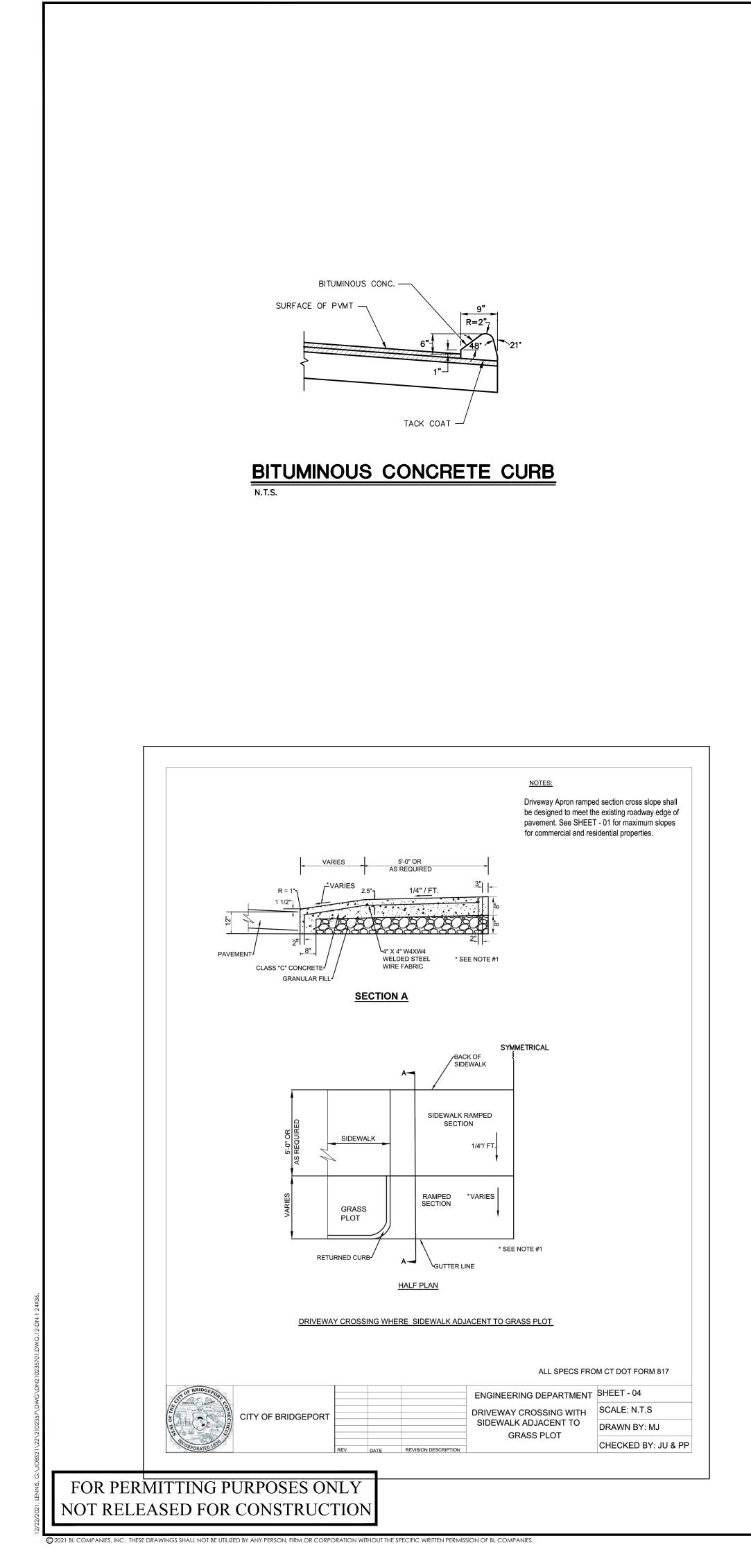
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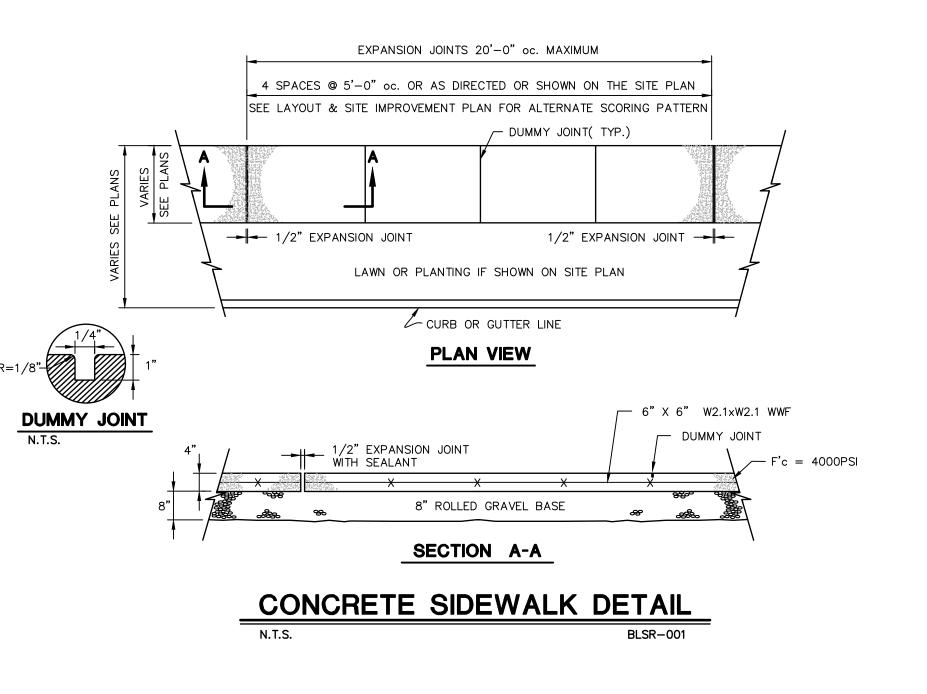
20. ALL SHADE TREE, BUFFER YARD AND OTHER LANDSCAPING REQUIRED BY LOCAL ORDINANCE OR ZONING SHALL BE PERPETUALLY MAINTAINED BY THE PROPERTY OWNER. ANY LANDSCAPING NEEDED TO MEET AN ORDINANCE OR ZONING REQUIREMENT THAT DIES, IS REMOVED, OR IS SEVERELY DAMAGED SHALL BE REPLACED BY THE CURRENT PROPERTY OWNER AS SOON AS IS PRACTICAL CONSIDERING GROWING SEASONS, WITH A MAXIMUM OF 150 DAYS.

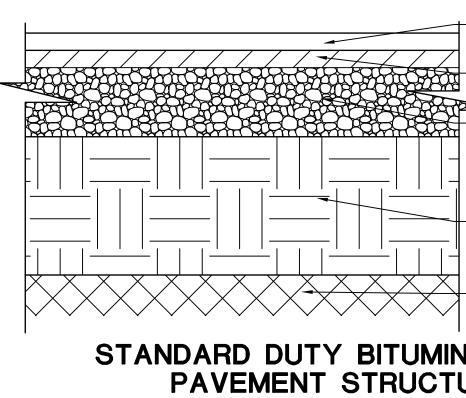
## FOR PERMITTING PURPOSES ONLY NOT RELEASED FOR CONSTRUCTION

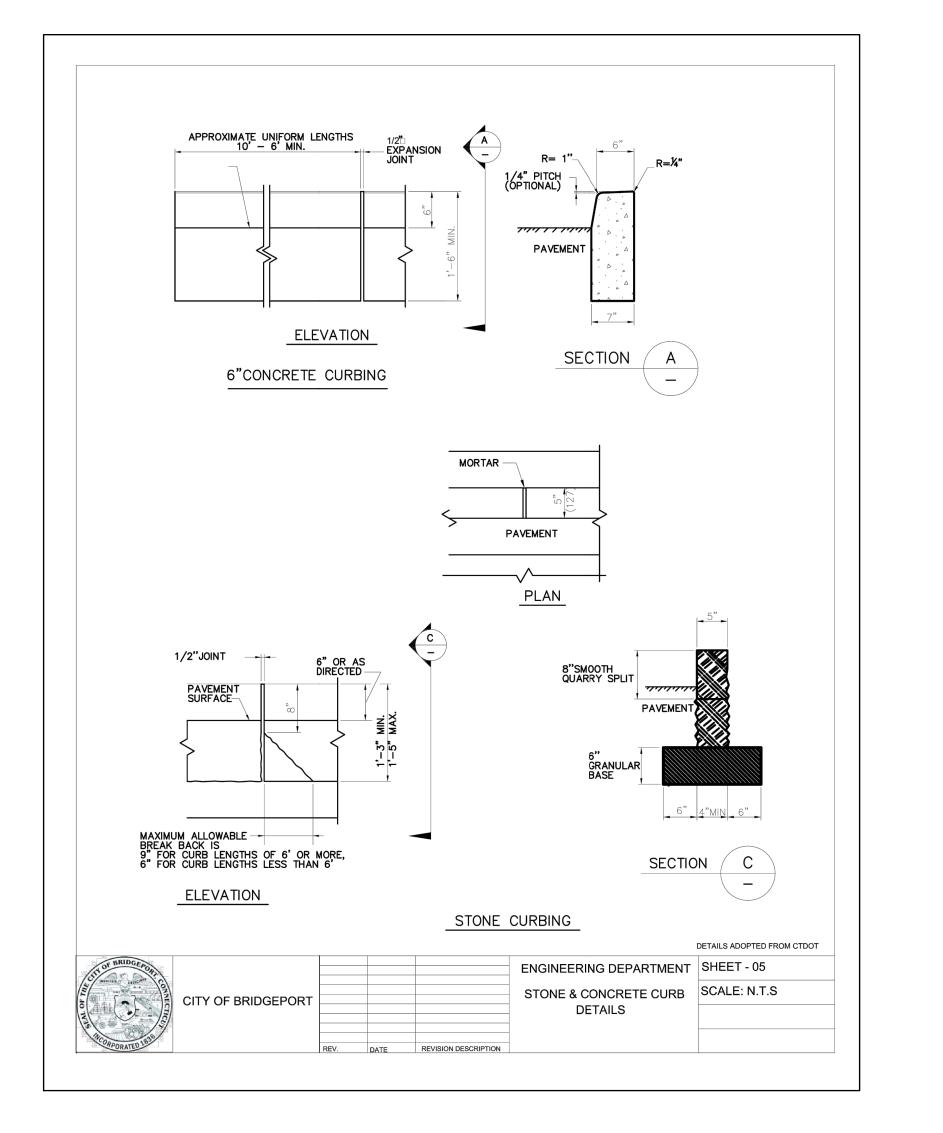


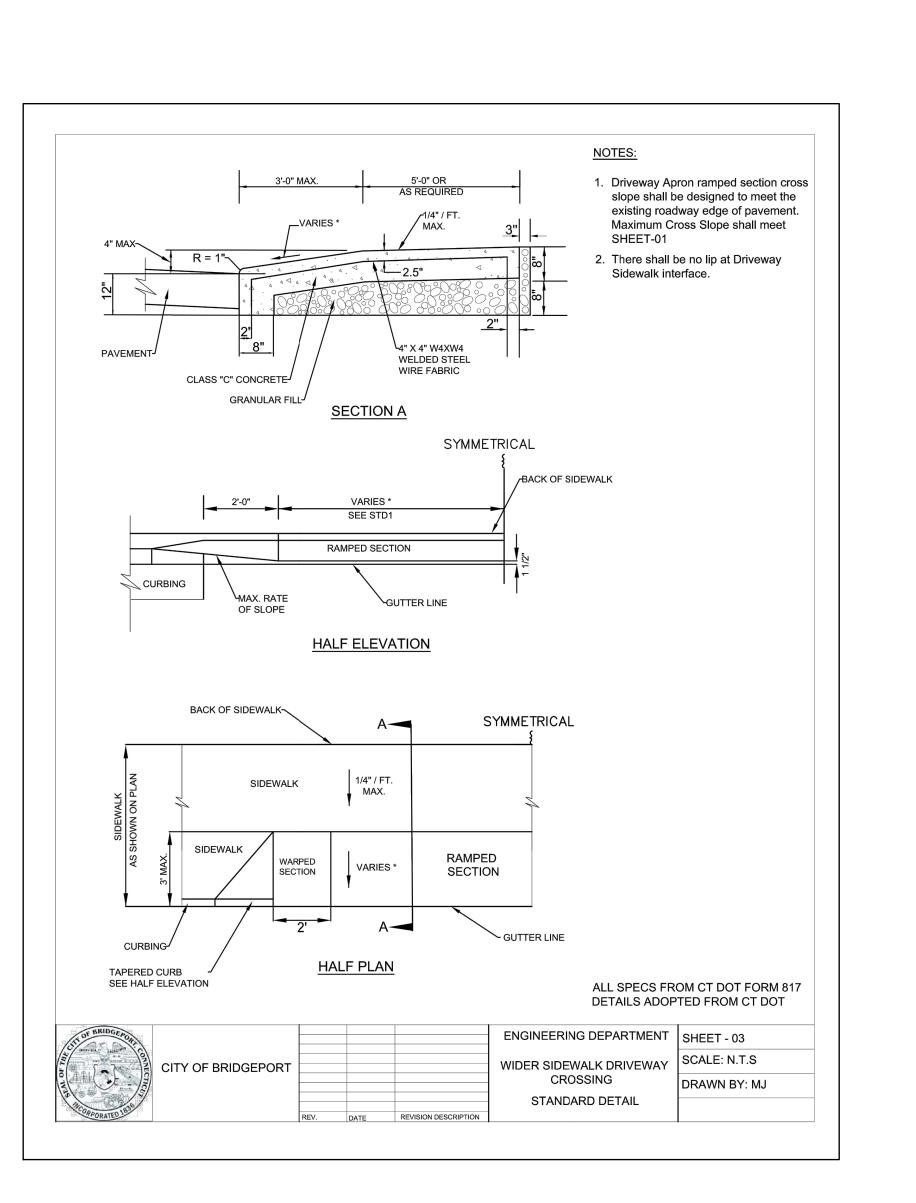












# STANDARD DUTY BITUMINOUS CONCRETE PAVEMENT STRUCTURE DETAIL

– PREPARED SUBGRADE COMPACTED TO 95% MAXDRY DENSITY PER ASTM D1557

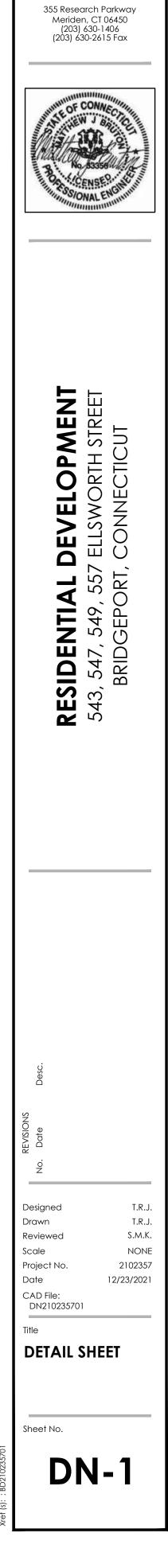
– 6" GRANULAR SUBBASE COURSE, CTDOT M.02.02. AND M.02.06, GRADING B

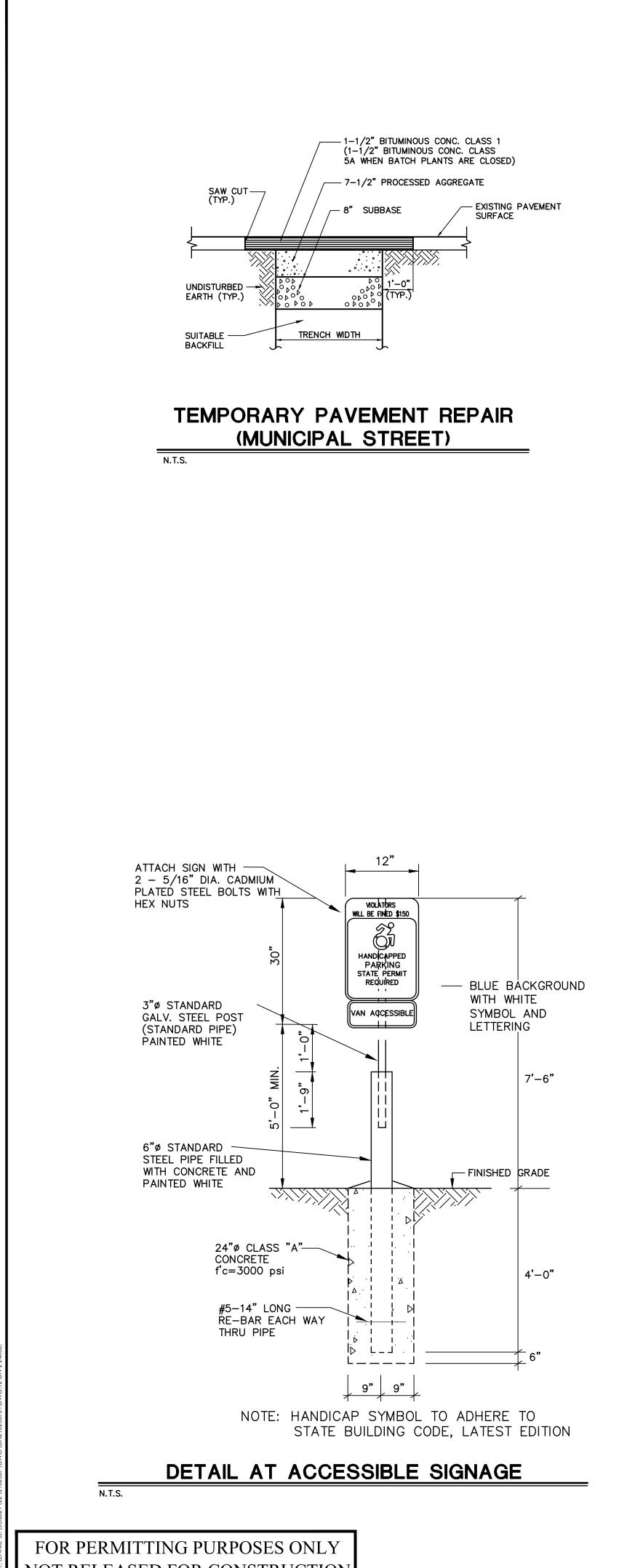
- 6" PROCESSED AGGREGATE BASE COURSE, CTDOT M.05.01

N.T.S.

2 1/2" BITUMINOUS CONCRETE BINDER COURSE, CTDOT M.04.02 CLASS 1

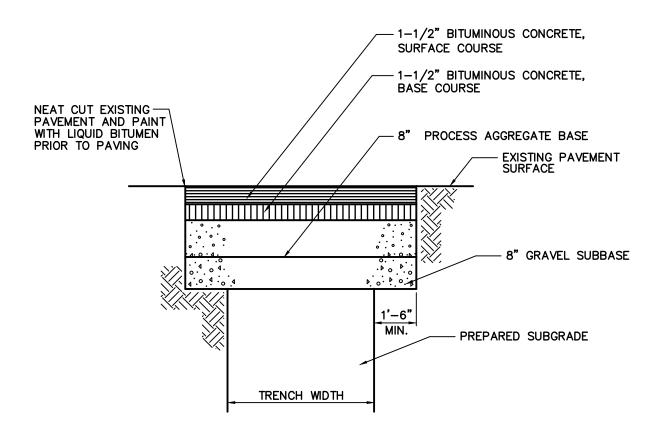
— 1 1/2" BITUMINOUS CONCRETE WEARING COURSE, CTDOT M.04.02 CLASS 2





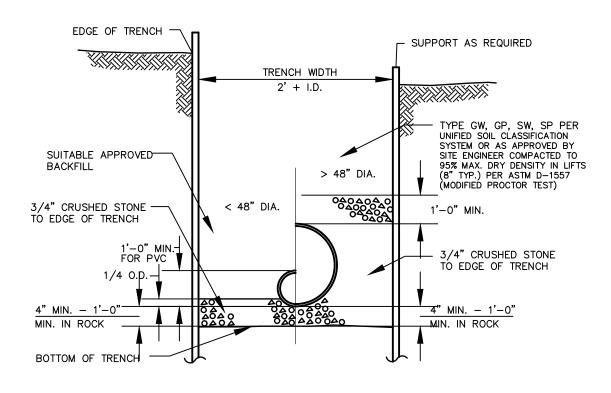
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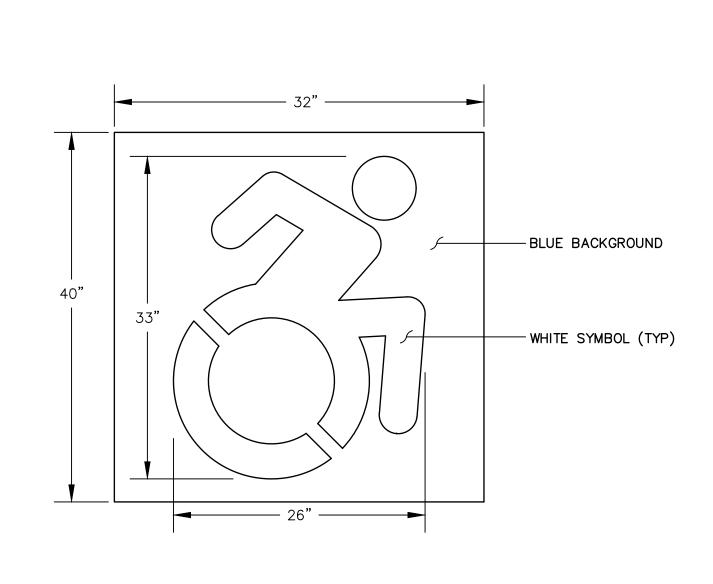


## **PAVEMENT REPAIR OVER TRENCH** (MUNICIPAL STREET)

N.T.S.



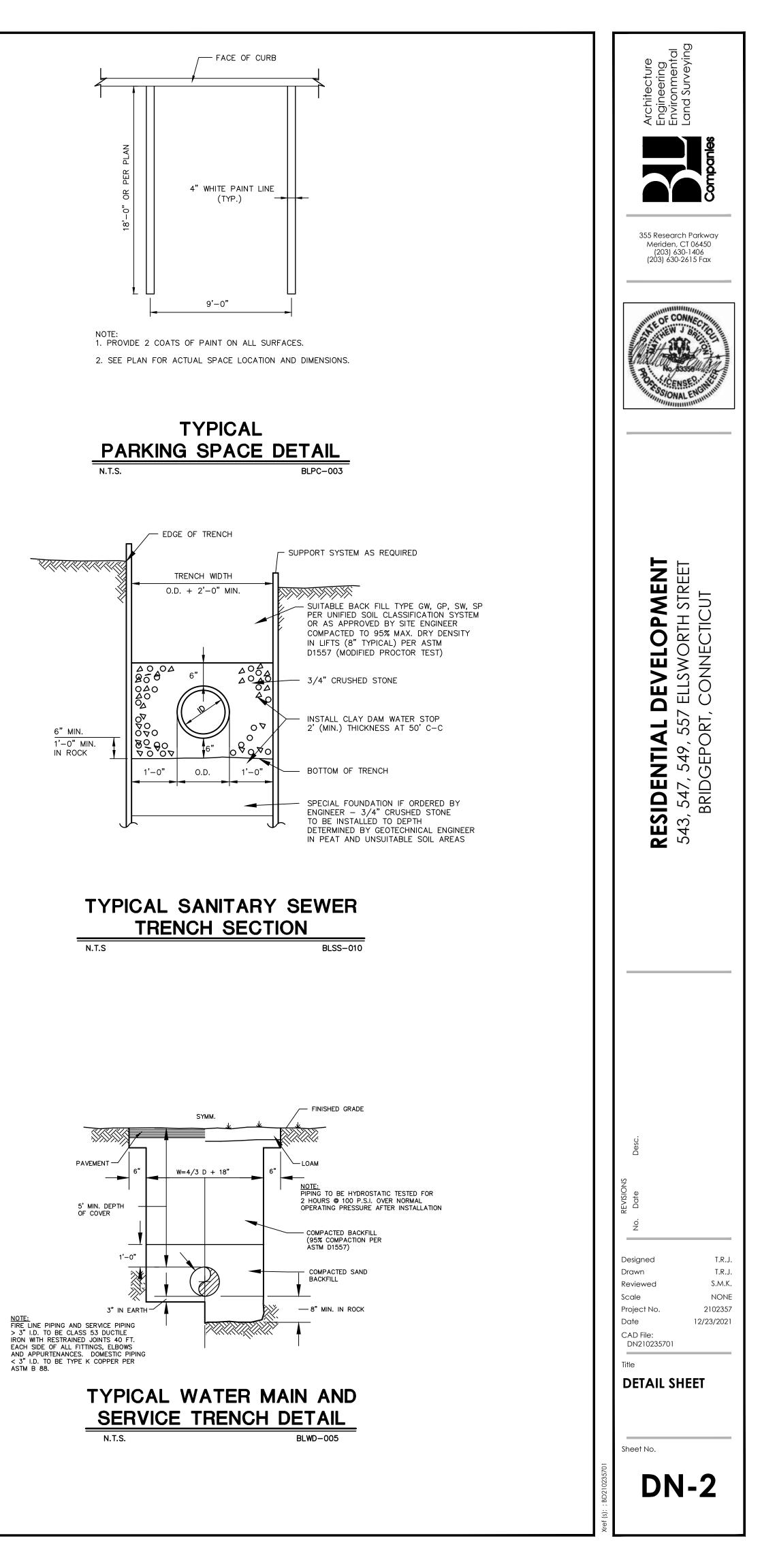
#### **TYPICAL STORM SEWER TRENCH SECTION** N.T.S. BLDD-004

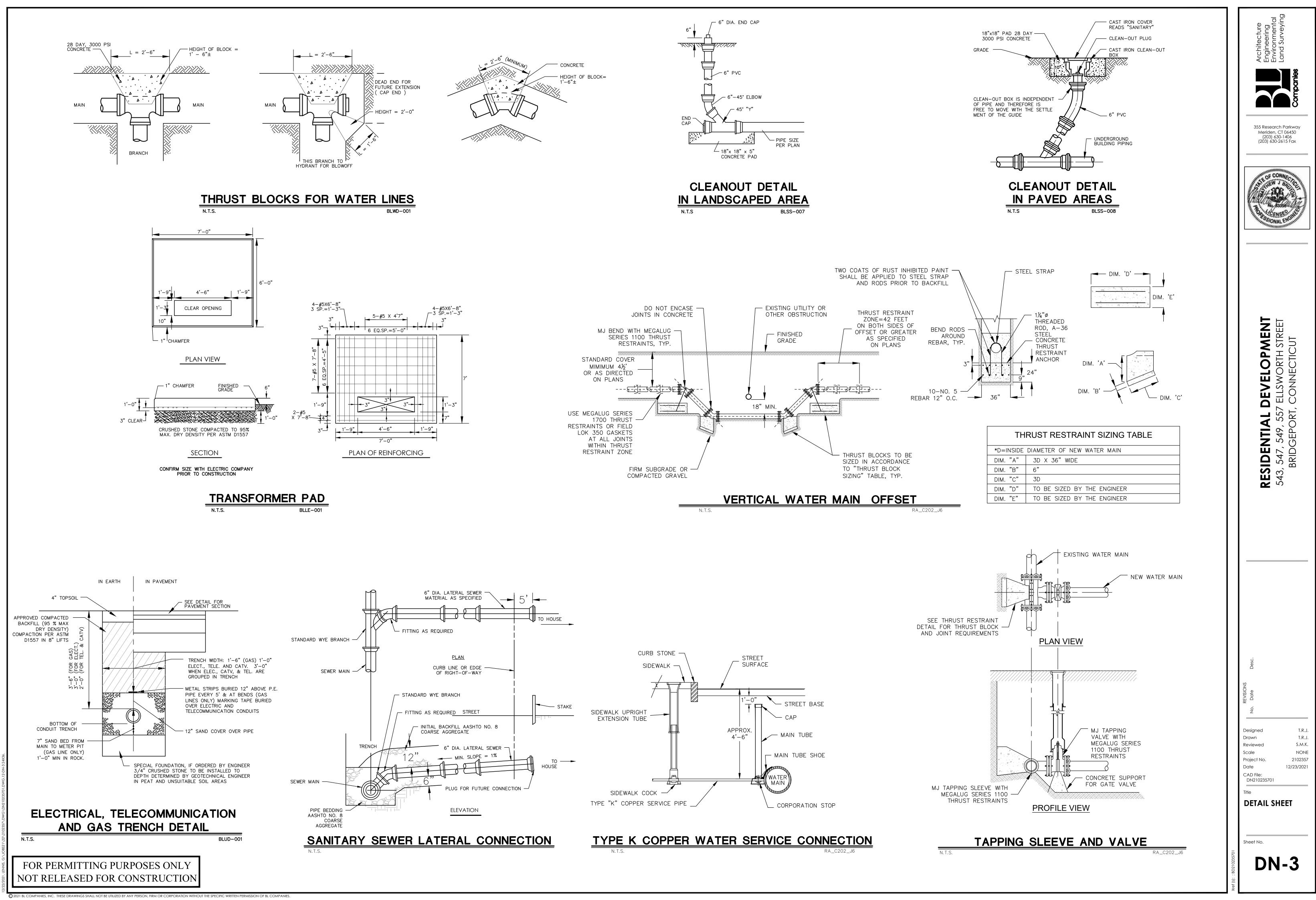


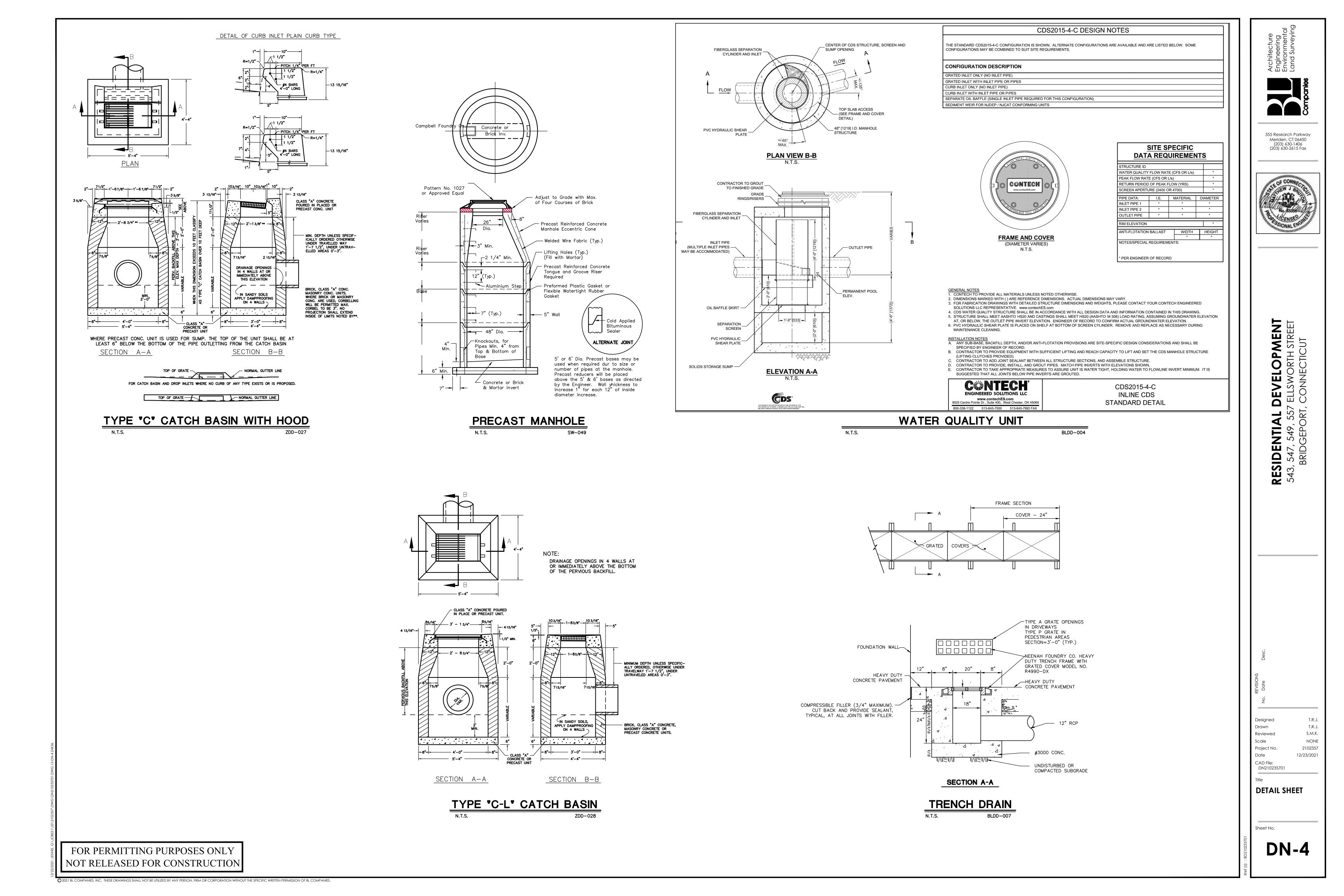
NOTE: HANDICAP SYMBOL TO ADHERE TO STATE BUILDING CODE, LATEST EDITION

## CONNECTICUT SYMBOL OF ACCESSIBILITY

N.T.S.







GENERAL	NOTES

- 1. STRIP ALL VEGETATION AND ORGANIC SOIL FROM THE WALL AND GRID ALIGNMENT.
- 2. BENCH CUT ALL EXCAVATED SLOPES.
- 3. DO NOT OVER EXCAVATE UNLESS DIRECTED BY SITE SOIL ENGINEER TO REMOVE UNSUITABLE SOIL.
- 4. SITE SOIL ENGINEER SHALL VERIFY FOUNDATION SOILS AS BEING COMPETENT PER THE DESIGN STANDARDS AND PARAMETERS.
- 5. LEVELING PAD SHALL CONSIST OF 3/4" CRUSHED STONE, MINIMUM 6" THICK OR MINIMUM 2000 PSI CONCRETE.
- 6. MINIMUM EMBEDMENT OF WALL BELOW FINISH GRADE SHALL BE 24".
- 7. FOLLOW APPLICABLE PROVISIONS OF THE MANUFACTURERS INSTALLATION INSTRUCTIONS AND WRITTEN SPECIFICATIONS.
- 8. WHERE DRAIN PIPE IS USED, PROVIDE OUTLETS AS SHOWN ON WALL ELEVATIONS.
- 9. COMPACTION TESTS SHALL BE TAKEN AS THE WALL IS INSTALLED. THE MINIMUM NUMBER OF TESTS SHALL BE DETERMINED BY THE SITE SOILS ENGINEER, OR AS INDICATED IN THE SPECIFICATION. 10. COMPACTION SHALL BE 95% OF MAXIMUM DRY DENSITY PER AASHTO
- T—99.
- 11. GEOGRID SHALL BE PER BLOCK MANUFACTURER'S DESIGN ON SHOP DRAWINGS.
- 12. PULL GEOGRID TIGHT PRIOR TO BACK FILLING. LENGTH OF GEOGRID SHALL BE MEASURED FROM FRONT OF SEGMENTAL CONCRETE UNITS.
- 13. PROVIDE LATERAL DRAINAGE SWALES TO DIRECT FLOWS AROUND THE
- ENDS OF THE WALL. 14. ESTABLISH TURF AS SOON AS THE WALL IS COMPLETED.
- 15. FINAL WALL ALIGNMENT SHALL BE LOCATED IN THE FIELD.
- 16. REINFORCED BACK FILL REQUIREMENTS FOR THE SEGMENTAL CONCRETE RETAINING WALL SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:

SIEVE SIZE	PERCENT PASSING
	REINFORCED BACK FILL
5 INCH	100
3-1/2 INCH	90-100
1-1/2 INCH	55-95
1/4 INCH	25-60
NO. 10	15-45
NO. 40	5–25
NO. 100	0-10
NO. 200	0-5
	LESS THAN OR EQUAL TO 10 AND A LIQ
CC THAN OD FOUND	

IQUID LIMIT LESS THAN OR EQUAL TO 40. REINFORCED BACK FILL SHALL BE PLACED AND COMPACTED IN LIFTS NOT EXCEEDING 10 INCHES. REINFORCED BACK FILL SHALL BE COMPACTED TO 95 PERCENT OF THE MAXIMUM DENSITY AS DETERMINED BY AASHTO T-99. THE MOISTURE CONTENT OF THE BACK FILL PER SPECIFICATIONS, MATERIAL PRIOR TO AND DURING COMPACTION SHALL BE WITHIN 2 PERCENTAGE POINTS OF DRY OPTIMUM.

IF CONDITIONS ARE DIFFERENT THAN THOSE STATED IN THESE DRAWINGS AND SPECIFICATIONS, THE CONTRACTOR MUST CONTACT THE ENGINEER PRIOR TO PROCEEDING WITH THE CONSTRUCTION OF THE WALL.

••• • • • • LEVELING PAD, 6" MIN. \_\_ GRID LENGTHS TO BE SUPPLIED BY BLOCK MANUFACTURER OR 8" CONCRETE NOTE: 1. CONTRACTOR TO SUBMIT DESIGN PLANS FOR THE PROPOSED

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• •

6" MIN. 🔨

CONCRETE ADHESIVE

TO TOP UNIT W/ANCHOR

SEGMENTAL BLOCK —

FACING UNITS

5' MIN.

RETAINING WALL INCLUDING CALCULATIONS, PREPARED AND STAMPED BY A MASSACHUSETTS LICENSED PROFESSIONAL ENGINEER PRIOR TO ORDERING MATERIALS.

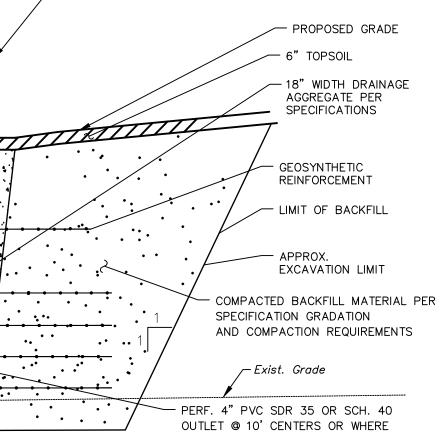
TYPICAL SEGMENTAL RETAINING WALL SECTION

N.T.S.

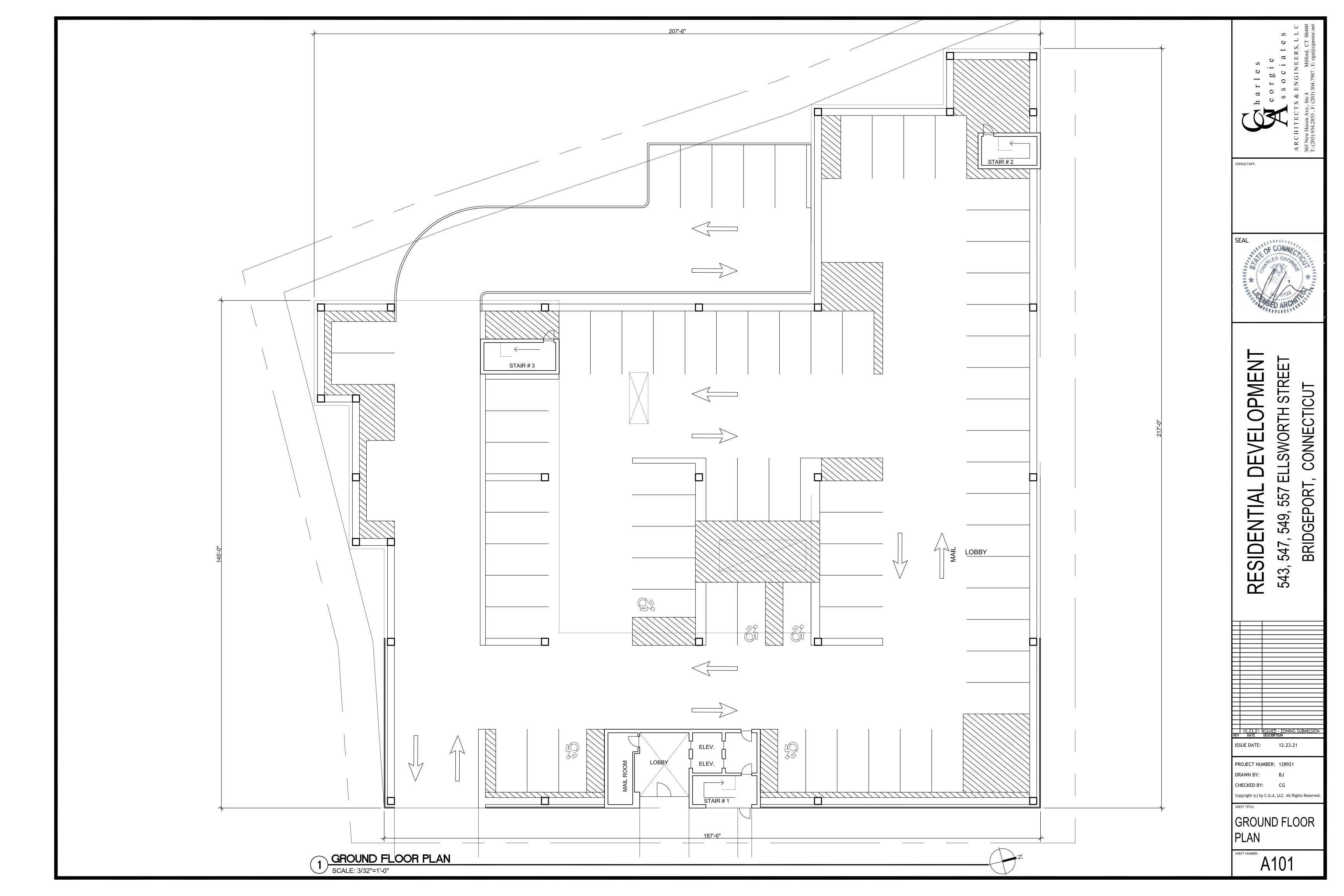


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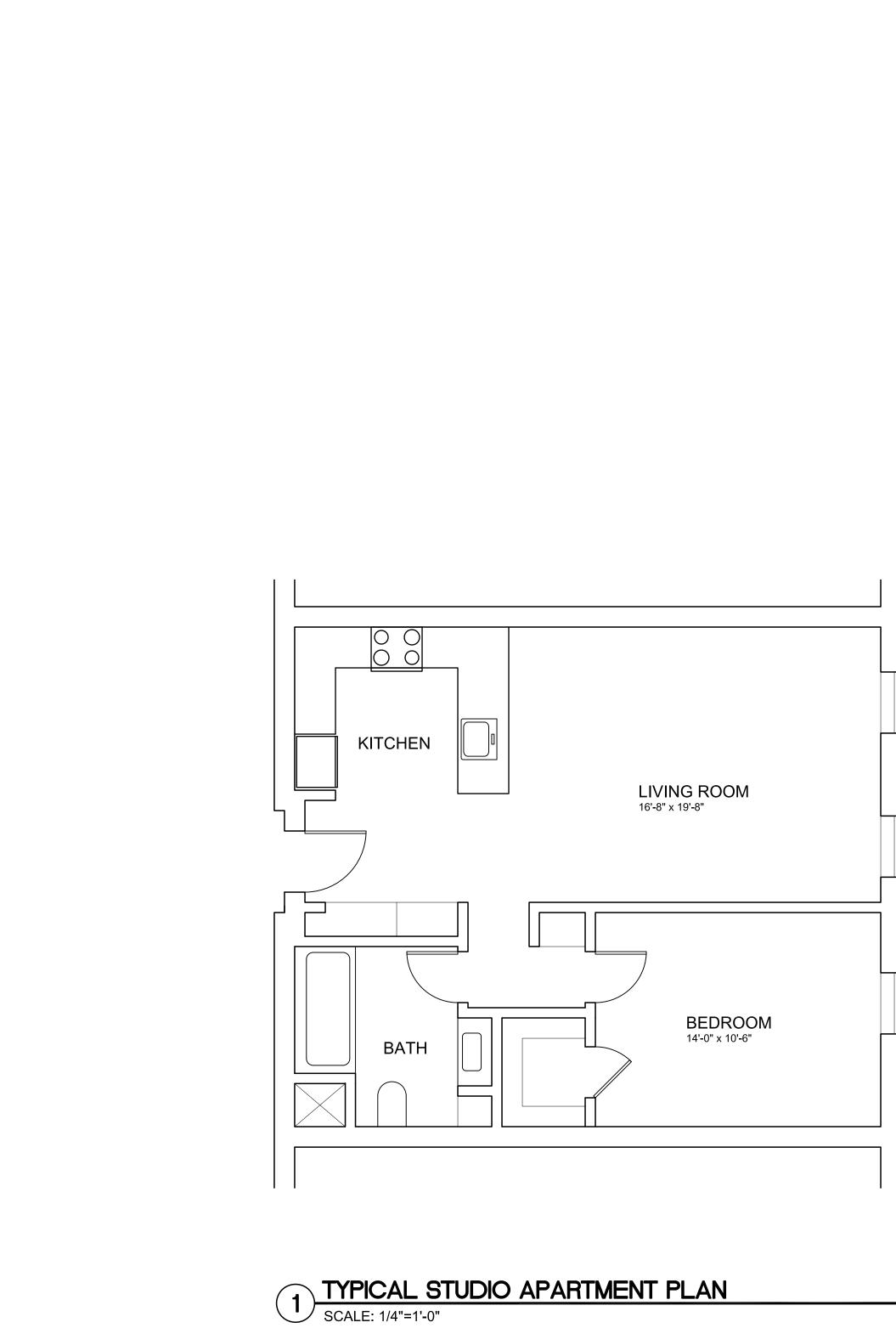
SHOWN ON PLANS. MIN. SLOPE TO DRAIN (1/8" PER FT.) WITH FILTER FABRIC. ENCASE WITH CONCRETE THROUGH WALL UNITS.

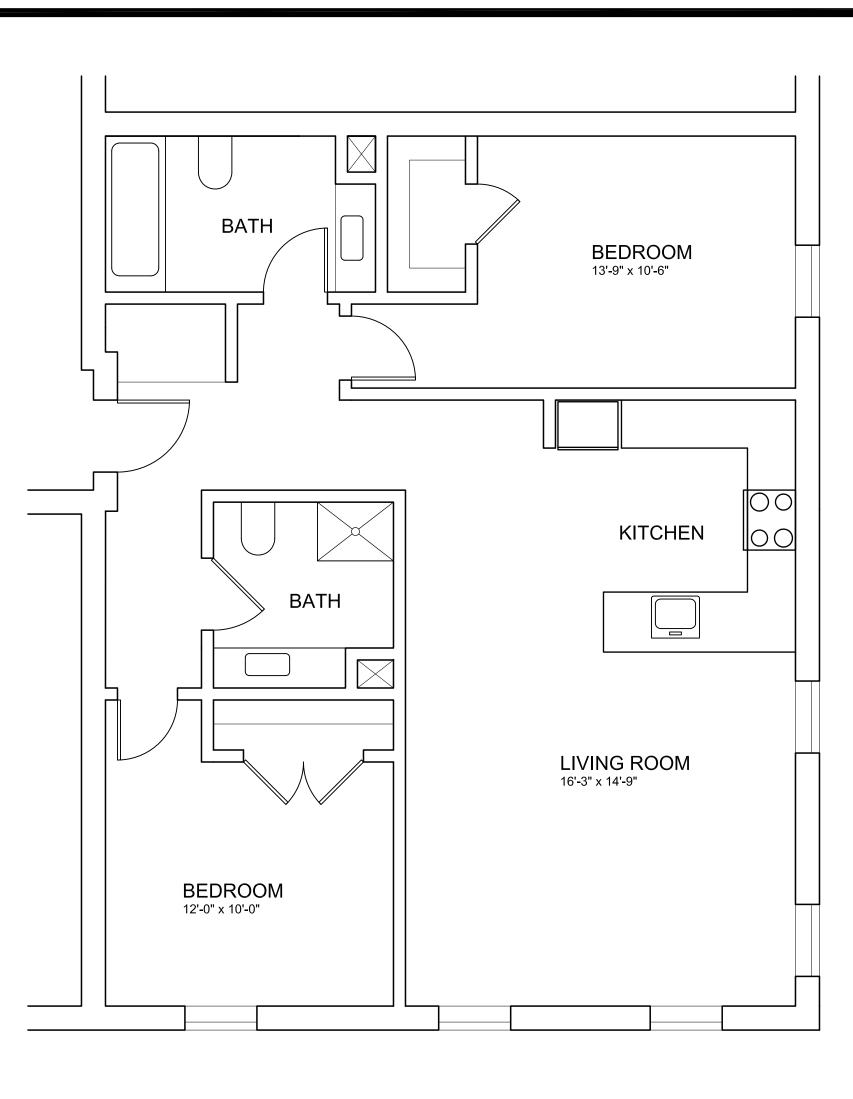




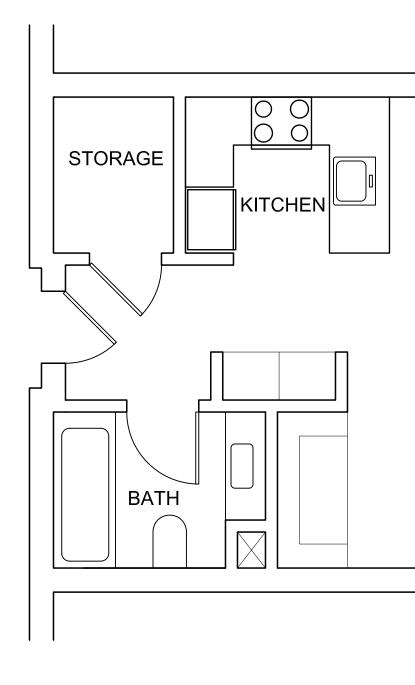


STUDIO	1-BEDROOM	ELEV. ELEV. STAIR # 1	STUDIO	STUDIO



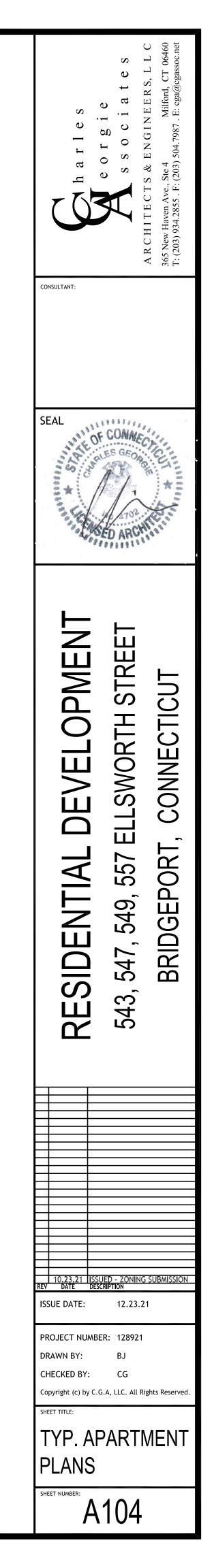


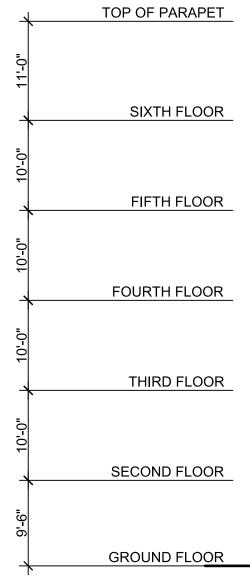


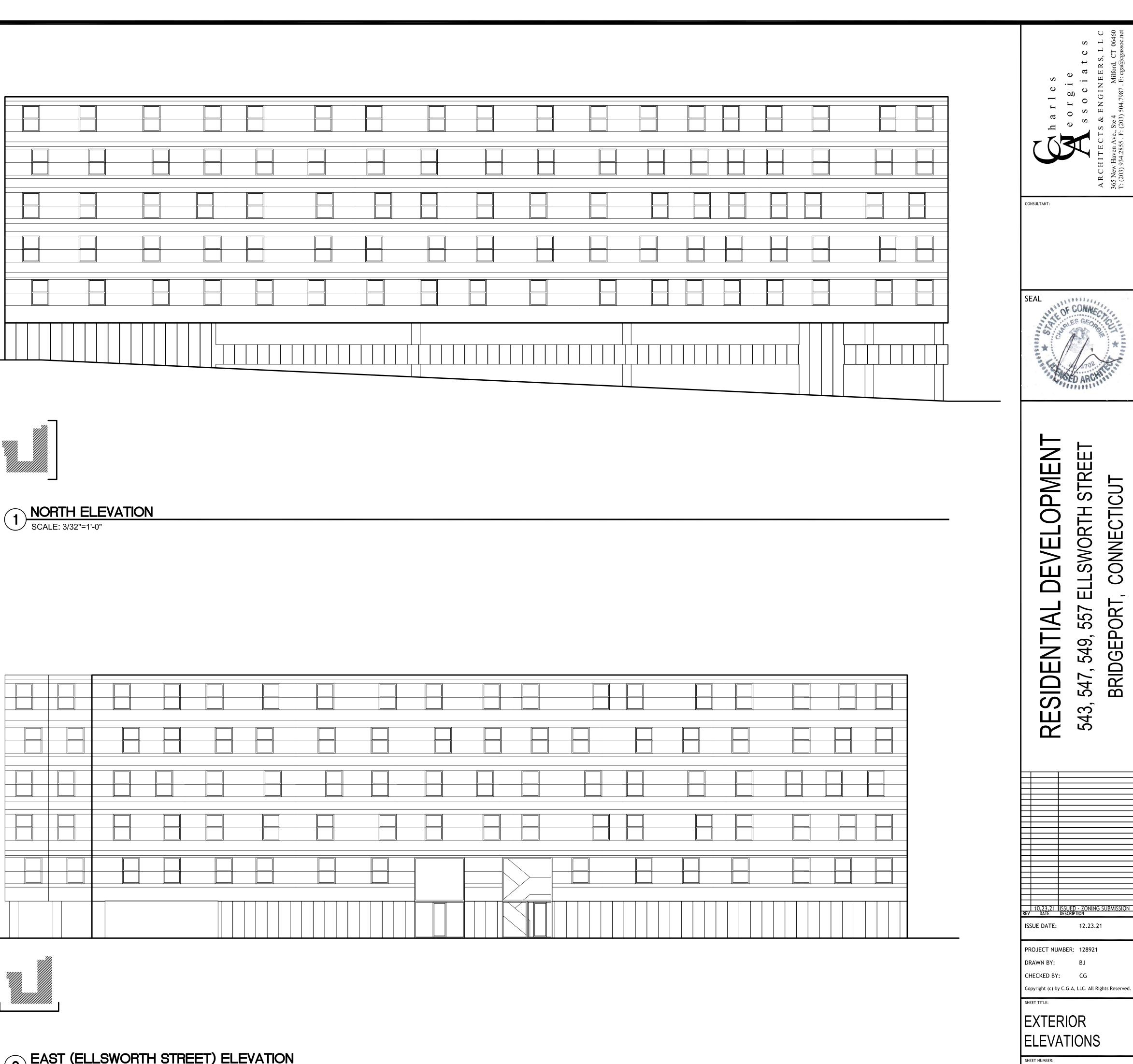




LIVING ROOM 16'-8" x 19'-8"





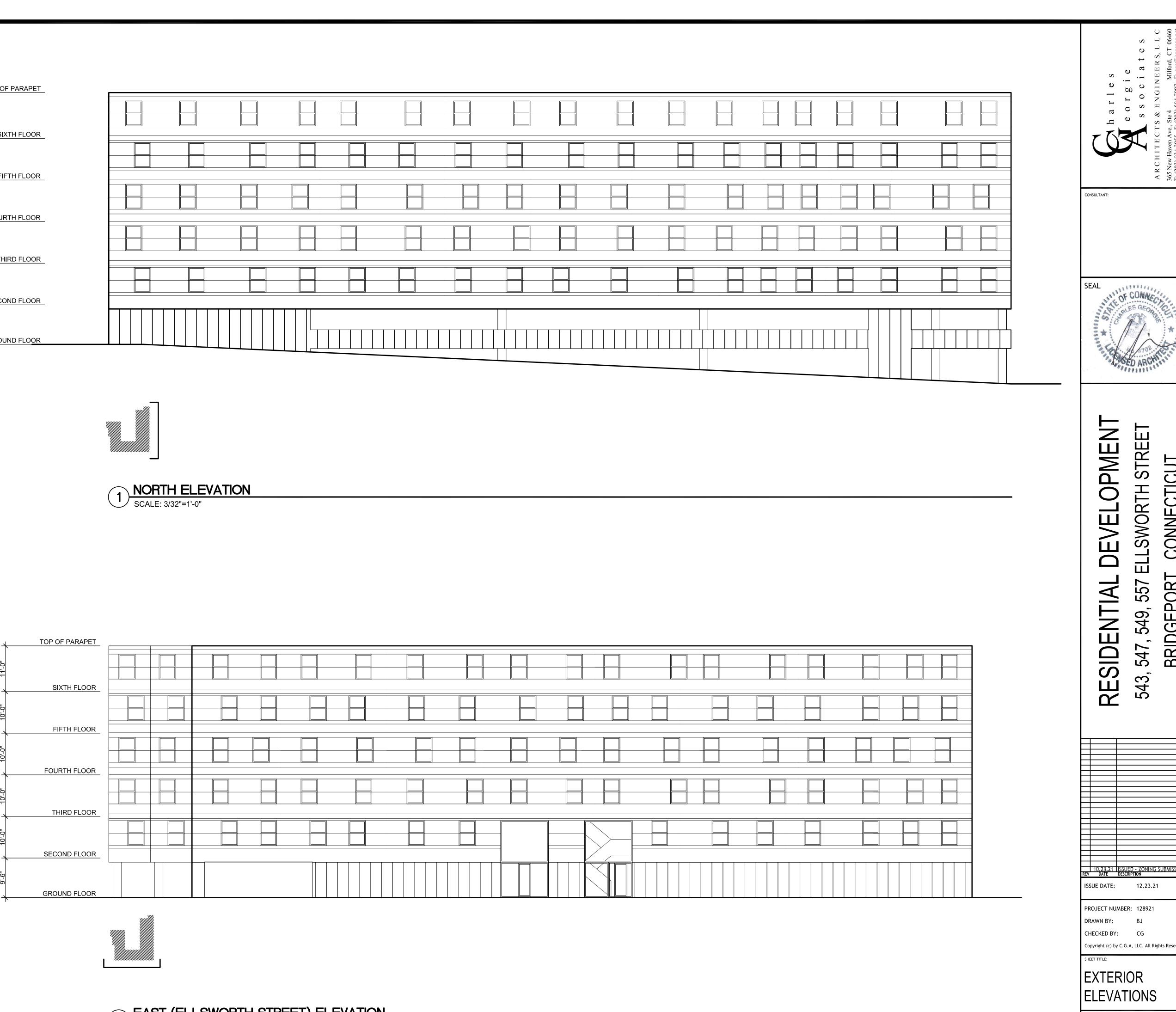


A201



## FACADE MATERIALS

		MATERIAL	COLOR
GFL FAG	CADE AND RAILING	PERFORATED ALUMINUM PANELS	DARK GREY
SECON	D - SIXTH FLOORS	CEMENT BOARD	LIGHT BEIGE
WINDO	WS AND DOORS	VINYL	ALMOND







# 2 EAST (ELLSWORTH STREET) ELEVATION SCALE: 3/32"=1'-0"

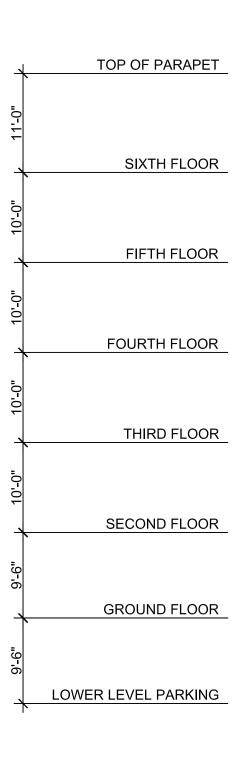


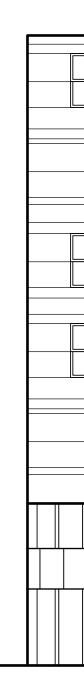


## FACADE MATERIALS

	MATERIAL	COLOR
GFL FACADE AND RAILING	PERFORATED ALUMINUM PANELS	DARK GRE
SECOND - SIXTH FLOORS	CEMENT BOARD	LIGHT BEIG
WINDOWS AND DOORS	VINYL	ALMOND

## SCALE: 3/32"=1'-0"

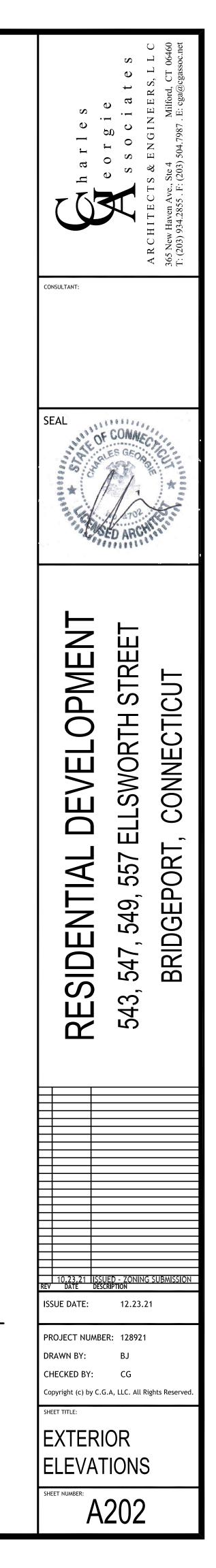


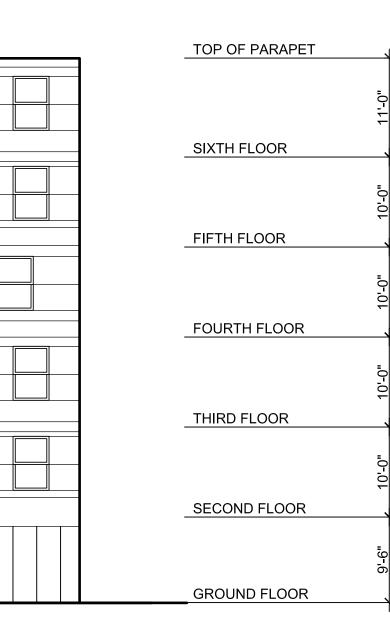


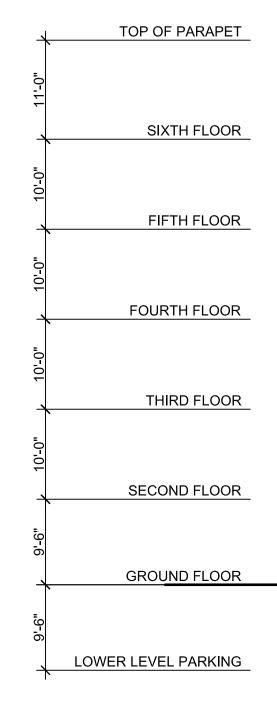


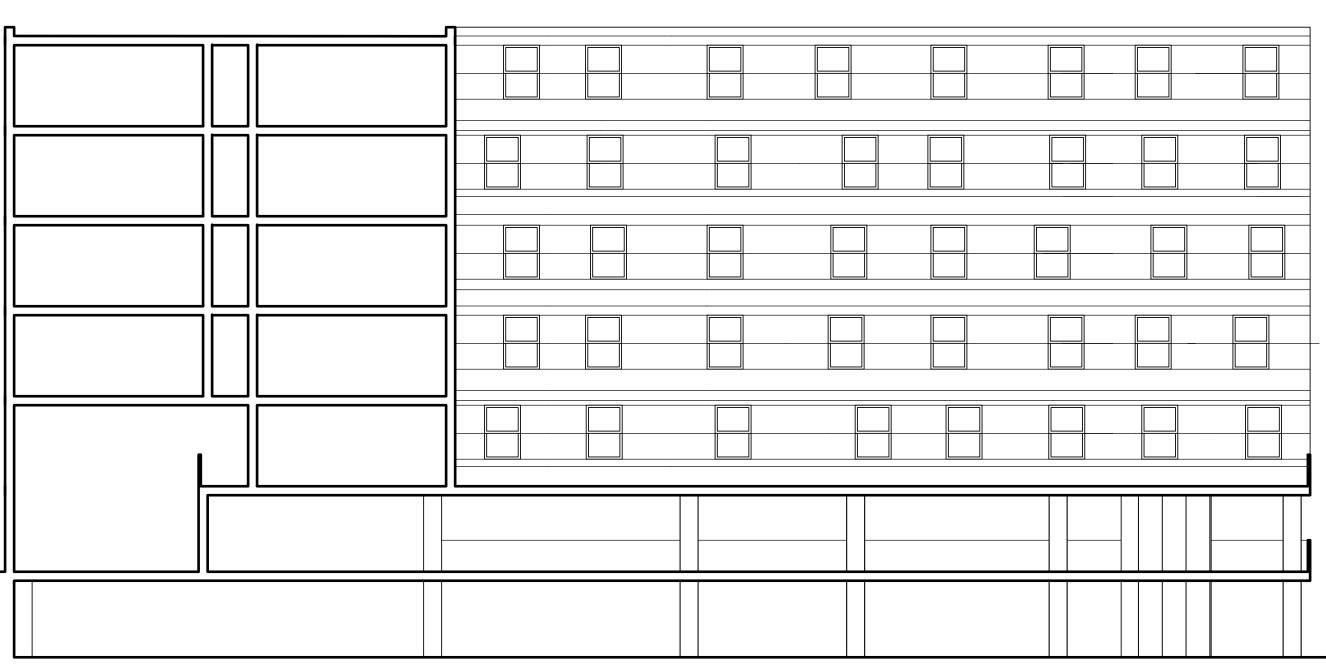


2 WEST ELEVATION SCALE: 3/32"=1'-0"







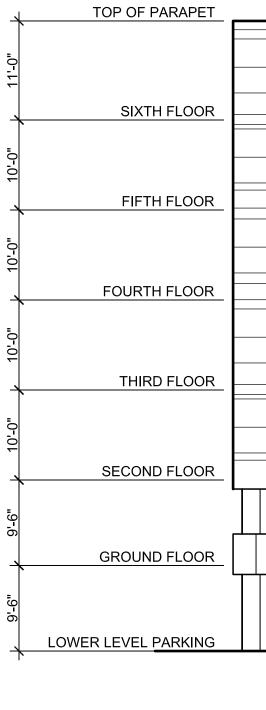




## FACADE MATERIALS

	MATERIAL	COLOR
GFL FACADE AND RAILING	PERFORATED ALUMINUM PANELS	DARK GREY
SECOND - SIXTH FLOORS	CEMENT BOARD	LIGHT BEIGE
WINDOWS AND DOORS	VINYL	ALMOND









## 1 NORTH COURTYARD ELEVATION SCALE: 3/32"=1'-0"



# 2 SOUTH COURTYARD ELEVATION SCALE: 3/32"=1'-0"

S O I J E H	e orgie s s o c i a t e s	A R C H I T E C T S & E N G I N E E R S, L L C 365 New Haven Ave., Ste 4 Milford, CT 06460 T: (203) 934.2855 . F: (203) 504.7987 . E: cga@cgassoc.net
SEAL	ES GEOR	CUL * COL
RESIDENTIAL DEVELOPMENT	543, 547, 549, 557 ELLSWORTH STREET	BRIDGEPORT, CONNECTICUT
ISSUE DATE:	UED - ZONINC CRIPTION 12.23.2 ER: 128921	
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### **Stormwater Management Report**

For the: **Proposed Residential Development** 

Located at: 543, 547, 549, 557 Ellsworth Street City of Bridgeport Connecticut

Prepared for Submission to: **City of Bridgeport, Connecticut** 

December 23, 2021

Prepared for: 547 Ellsworth NavCapMan, LLC 547 Ellsworth Street Bridgeport, CT 06605

#### Prepared by:



Architecture Engineering Environmental Land Surveying

**BL** Companies 100 Constitution Plaza, 10th Floor Hartford, Connecticut 06103 Phone: (860) 249-2200 Fax: (860) 249-2400

BL Project Number: 2102357



#### Contents

Executive Summary	1
FEMA Flood Insurance Rate Map	2
Hydrologic Modeling Methodology	2
Hydrologic Modeling	2
Existing Site Conditions and Hydrology Conditions	3
General Site Information	3
Soil Description	3
Developed Site Conditions and Hydrology Conditions	5
General Site Information	5
Proposed Hydrologic Conditions	5
Proposed Drainage Areas	6
Post-Development Hydrologic Analysis Results	6
Permanent BMP's and Water Quality	8
Summary	8



Appendix A: Location Maps

- Figure 1: USGS Location Map
- Figure 2: Aerial Location Map
- Figure 3: NRCS Soil Survey Map with Hydrologic Soil Group Data
- Figure 4: FEMA Federal Insurance Rate Map
- Figure 5: NOAA Atlas 14 Storm Data (Depth, Inches)
- Figure 6: NOAA Atlas 14 Storm Data (Intensity, Inches per Hour)

Figure 7: City of Bridgeport Zoning Map

- Appendix B: Pre-development Hydrology (2-, 10-, 25-, and 100-year storms)
- Appendix C: Post-development Hydrology (2-, 10-, 25-, and 100-year storms)
- Appendix D: Collection and Conveyance Calculations
- Appendix E: Water Quality Calculations

Best Management Practice (BMP) Treatment Train Efficiency Worksheet Water Quality Calculations – Water Quality Volume (WQV) Water Quality Calculations – Water Quality Flowrate (WQF) Water Quality Calculations – Infiltration Volume

- Appendix F: Drainage Maps
  - ED-1 Existing Drainage Plan
  - PD-1 Proposed Drainage Plan
  - PD-2 Proposed Hydraulic Plan

Appendix G: Geotechnical Report



#### **Executive Summary**

This report has been prepared in support of a Permit Application submission to the City of Bridgeport by 547 Ellsworth NavCapMan, LLC for a proposed residential development at 543, 547, 549, and 557 Ellsworth Street. The design and analysis as presented within this report generally complies with the *2002 Connecticut Guidelines for Soil Erosion,* and the *2004 Connecticut Stormwater Quality Manual.* 

The proposed residential development will not result in any adverse impacts to sensitive areas downstream of the proposed development.

#### Location

The proposed development is situated on four existing tax parcels, totaling approximately 46,195 square feet (s.f.). The property is bordered by a residential lot on the north side, a commercial lot to the south, Ellsworth Street on the east side and residential lots on the west side. Per the City of Bridgeport's Zoning Map, the property is located within the R-CC zone, abutting parcels on the east and south are located within the OR Zone, the abutting parcel to the north is located within the R-CC Zone, and abutting parcels on the west are located within the R-BB zone. A project location map and a copy of the City of Bridgeport's zoning map has been provided in Appendix A of this Report.

#### Property Description

The existing parcels redeveloped by the proposed residential development includes four residential lots with each containing a house, associated driveway, and grassed yard. Three of the lots also contain a garage. The existing topography, ranging from elevation 53 to 37, generally slopes from southeast to northwest. Based on the existing drainage patterns, the Site hydrology can be divided into two drainage areas; the majority of the site flows to the abutters lot and a smaller portion of the site flows (the existing sidewalk) down Ellsworth Street to the existing drainage network. Runoff from the subject parcels flows overland to the abutting lot to the north of the development. Runoff from the sidewalk drainage area flows overland to the discharge point.

#### **Project Description**

The proposed site improvements include the construction of a multi-unit residential building (123 units), associated parking area, driveway, parking garage, landscaped areas, site utilities, lighting, and a stormwater management system. To improve the existing stormwater quality for the site and support the overall proposed development, a



water quality devices has been incorporated into the stormwater design, specifically the installation of a subsurface infiltration system and a hydrodynamic separator. A complete summary of the supporting analysis and sizing is provided in subsequent sections of this report.

#### FEMA Flood Insurance Rate Map

Per the FEMA Flood Insurance Rate Map Number 09001C0436G for Fairfield County, Connecticut revised July 8, 2013, the parcel resides in Flood Hazard Zone X. Zone X is defined as "area of minimal flood hazard. A copy of the FEMA Flood Insurance Rate Map is included in Appendix A for reference.

#### Stormwater Analysis Summary

A HydroCAD model, using TR-55 and SCS methodology, was developed to evaluate the site's existing and proposed drainage conditions for 2-, 10-, 25-, and 100-year storm events. Water quality treatment, infiltration and stormwater mitigation has been provided for this project by the installation of a subsurface infiltration system and a hydrodynamic separator (CDS unit). The proposed Stormwater Best Management Practices (BMP) are upstream of the discharge point. These Stormwater BMPs will exceed the minimum required TSS removal rate of 80% per the 2004 Connecticut Stormwater Quality Manual.

#### Hydrologic Modeling Methodology

#### Hydrologic Modeling

The SCS Runoff Curve Number and TR-55 Methods were utilized to determine the peak runoff for each watershed impacted by the proposed development. All supporting calculations have been completed using the stormwater computer modeling program known as HydroCAD, version 10.00, developed by HydroCAD Software Solutions, LLC. Hydrographs for each watershed were developed using the SCS Synthetic Unit Hydrograph Method and rainfall depths per the NOAA Atlas 14 for Bridgeport, CT as shown in Table 1. The drainage areas, or sub catchments as labeled by the program, are depicted by hexagons on the attached drainage diagrams. Pre-development HydroCAD results can be found in Appendix B and Post-development HydroCAD results can be found in Appendix C.



<b>Return Period</b>	24-hour Rainfall Depth
2-year	3.47
10-year	5.35
25-year	6.52
100-year	8.33

## Table 1 – 24-HR Rainfall Depths per NOAA Atlas 14 (Bridgeport, CT)

#### Existing Site Conditions and Hydrology Conditions

#### General Site Information

As previously noted, the site generally slopes east to west. Runoff within the site flows overland to the abutting lot and the municipal drainage system. Based on the existing drainage patterns, the Site hydrology can be divided into two design points; runoff from the majority of the site flows overland to the abutter's residential lot on the northwestern side of the site and runoff from the existing sidewalk flows overland by to the existing municipal drainage system.

#### Soil Description

The soils included within this stormwater analysis were identified using available online resources created by the United States Department of Agriculture (USDA) Natural Resource Conservation Services (NRCS). They are as follows:

• Urban Land – Type D Soil

A copy of the USDA NRCS Hydrologic Soil Group map is located within Appendix A of this report.

#### Existing Hydrologic Conditions

The existing site drainage area analyzed within this study totals 48,058 s.f. (1.10 acres) and is approximately 30.4% impervious. Runoff from the western portion of the site travels overland by sheet flow to the abutter's residential lot. Runoff from the sidewalk along the eastern side of the site travels overland by sheet flow to the roadway and discharges to the municipal drainage network. Two design points have been identified in the existing hydrologic conditions. Design Point 1 (DP-1) is the northern abutter's residential lot and Design Point 2 (DP-2) is the existing drainage system in Ellsworth Street. In the existing hydrologic conditions all runoff flows to the design points undetained.



The following is a brief summary of the existing drainage areas as shown on the enclosed Existing Drainage Map (ED-1), in Appendix F.

Existing Drainage Area 1 (EDA-1): This area consists of the western portion of the site. EDA-1 is 46,195 s.f. in size and is 28.9% impervious. The curve number for this area is 84. Stormwater runoff from EDA-1 flows overland and to the abutter's residential lot on northern side of the property (Design Point 1).

Existing Drainage Area 2 (EDA-2): This area consists of the sidewalk along the eastern side of the site and is 1,863 s.f. in size. This drainage area is 67.7% impervious and has a curve number of 92. Stormwater runoff from EDA-2 flows overland and discharges to the existing municipal drainage system in Ellsworth Street (Design Point 2).

#### Existing Conditions Hydrologic Analysis Results

The results of the existing conditions hydrologic analysis area as follows and summarized in Tables 2 and 3 below.

Drainage Area	Area (square feet)	Composite Curve Number	Imperviousness Cover (%)	Time of Concentration (minutes)
EDA-1	46,195	84	28.9%	12.40
EDA-2	1,863	92	67.7%	5.00

#### Table 2 – Pre-Development (Existing Conditions) Drainage Characteristics

Note: Minimum Time of Concentration (T<sub>c</sub>) used for this analysis is 5 minutes.

Analysis	Description		Peak F	low (cfs	s)
Point		2-yr	10-yr	25-yr	100-yr
Design	Northern Abutter	2.00	3.59	4.58	6.09
Point 1	Residential Lot				
Design	Drainage System in	0.15	0.25	0.30	0.38
Point 2	Ellsworth Street				

#### Pro Dovolonment Conditions Peak Flows Table 2



#### **Developed Site Conditions and Hydrology Conditions**

#### General Site Information

The proposed development includes the construction of a multi-unit residential building (123 units), associated parking area, driveway, parking garage, landscaped areas, site utilities, lighting, and a stormwater management system. The existing drainage patterns have been maintained throughout the site. To improve the overall water quality for this site and support the proposed development, a water quality device will be installed to treat the captured stormwater prior to discharging it offsite.

All existing infrastructure outside of the project's limits shall remain in place without interruption in service or overall functionality.

The proposed project will disturb approximately 48,058 s.f..

#### Proposed Hydrologic Conditions

The proposed hydrologic analysis for this project maintains the methodologies, design points, and supporting assumptions described above. The intent of the proposed stormwater design is to mimic the existing drainage patterns, runoff flowrates, and runoff volumes to the greatest extend practical while improving the stormwater quality for the site.

The proposed site drainage area analyzed within this study maintains the original 48,058 s.f. described above. The proposed residential development is a consistent use compared to abutting lots and is approximately 78.5% impervious. This includes all paved surfaces and driveways as well as the proposed building's roof.

The intent of the proposed stormwater design is to mimic the existing drainage patterns for the drainage areas as described within the Existing Hydrology Conditions Section of this report. All calculations were based on the 2- 10- 25-, and 100-year stormfall events in order to accurately depict the proposed conditions. To mitigate any impact and improve the overall water quality for this site, stormwater treatment will be provided with the installation of a subsurface infiltration system and a hydrodynamic separator (CDS unit) to treat the stormwater runoff. Design calculations for the overall treatment effectiveness of the proposed system and water quality calculations are included in Appendix E. All stormwater quality treatment measures have been designed per *2004 Connecticut Stormwater Quality Manual*.



#### Proposed Drainage Areas

The following section briefly describes each drainage area as shown on the enclosed Proposed Drainage Map (PD-1), located in Appendix F of this report.

**Proposed Drainage Area 1A (PDA-1A):** PDA -1A is located on the roughly the center of the property and includes the building roof, driveway and courtyard area. PDA-1A is 36,188 s.f. in size and is 100.0% impervious. Runoff within this area flows across either the courtyard or driveway where it is captured in the proposed catch basin or trench drain. Roof runoff is directed to the proposed catch basin. Runoff captured in the proposed catch basin and trench drain flows to a hydrodynamic separator, then the subsurface infiltration system and ultimately discharges to Design Point 1, the northern abutter's residential lot.

**Proposed Drainage Area 1B (PDA-1B):** PDA -1B is located on the northern, western and southern border of the property and includes the transformer pad and the majority of the site's landscaped area. PDA-1B is 8,112 s.f. in size and is 0.6% impervious. Runoff from this area flows overland and discharges to Design Point 1, the northern abutter's residential lot.

**Proposed Drainage Area 2 (PDA-2):** PDA-2 is located on the eastern border of the site and includes the sidewalk and landscaped area. PDA-2 is 3,758 s.f. in size and is 39.7% impervious. Runoff from this area flows overland and ultimately discharges to Design Point 2, the municipal drainage system in Ellsworth Street.

#### Post-Development Hydrologic Analysis Results

The results of the post-development hydrologic analysis are as follows and summarized in Table 4 and 5 below:

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	Т	able 4 – Post De	velopment Drainag	e Characteristics	
	Drainage Area	Total Area (sf)	Composite Curve Number	Imperviousness Cover (%)	Time of Concentration (Minutes)
	PDA-1A	36,188	98	100.0%	5.0
ſ	PDA-1B	8,112	80	0.6%	10.0
	PDA-2	3,758	87	39.7%	10.6

#### Table 5 – Post-Development Conditions Peak Flows

Analysis Point	Description	Peak Flows (CFS)		5)	
		2-YR	10-YR	25-YR	100-YR
Design Point 1	Northern Abutter Residential Lot	1.35	3.56	4.22	5.14
Design Point 2	Drainage System in Ellsworth Street	0.20	0.34	0.42	0.56

For a complete comparison of pre- and post-development runoff rates for each design storm, refer to Table 6 shown below.

#### Table 6 Existing vs. Proposed Peak Runoff Rates

Peak Flow (CFS)						
	Design Storms					
Design Point	2-YR	10-YR	25-YR	100-YR		
V		-	20-1 K	100-11		
DP-1 – Northern Ab	utter Reside	ntial Lot	<del>.</del>			
Existing	2.00	3.59	4.58	6.09		
Proposed	1.35	3.56	4.22	5.14		
			7			
DP-2 – Drainage Sys	tem in Ellsw	orth Street				
Existing	0.15	0.25	0.30	0.38		
Proposed	0.20	0.34	0.42	0.56		
			~			
Total						
Existing	2.15	3.84	4.88	6.47		
Proposed	1.55	3.90	4.64	5.70		
Percent Change	-7.91%	1.56%	-4.92 %	-11.90%		



Table 6 above shows that in all storm events the peak flow rate to Design Point 1 is less in the post-development conditions. There is a slight increase in the post development peak flow rates to Design Point 2, the municipal drainage system in Ellsworth Street. The anticipated increase is de minimus in size since the existing drainage system can more than support this minor increase.

#### Permanent BMP's and Water Quality

Permanent Water Quality BMPs have been incorporated into the project design and include the installation of the subsurface infiltration system and a hydrodynamic separator (CDS unit) to treat the required water quality as well as provide stormwater mitigation and infiltration onsite. For location of the subsurface system and the hydrodynamic separator refer to the post development drainage map included in Appendix F of this report.

#### Summary

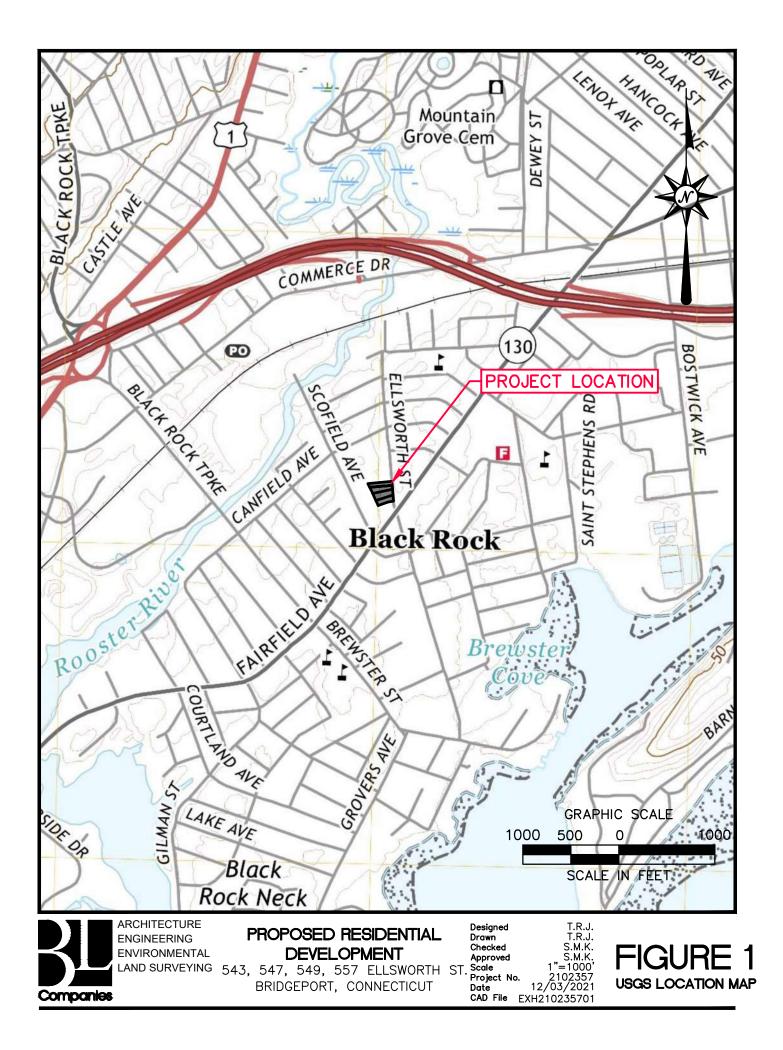
This stormwater analysis and report has been prepared to comply the 2002 Connecticut Guidelines for Soil Erosion, and the 2004 Connecticut Stormwater Quality Manual. The proposed development and proposed drainage infrastructure have been designed to convey and treat the stormwater runoff up to the 25-year design storm and will not result in any adverse impacts to abutting properties or roadways. All post-development stormwater drainage patterns have been maintained to match the pre-development conditions. Stormwater quality is provided with the installation of the subsurface infiltration system and hydrodynamic separator which will provide the minimum required 80% TSS removal and onsite infiltration.

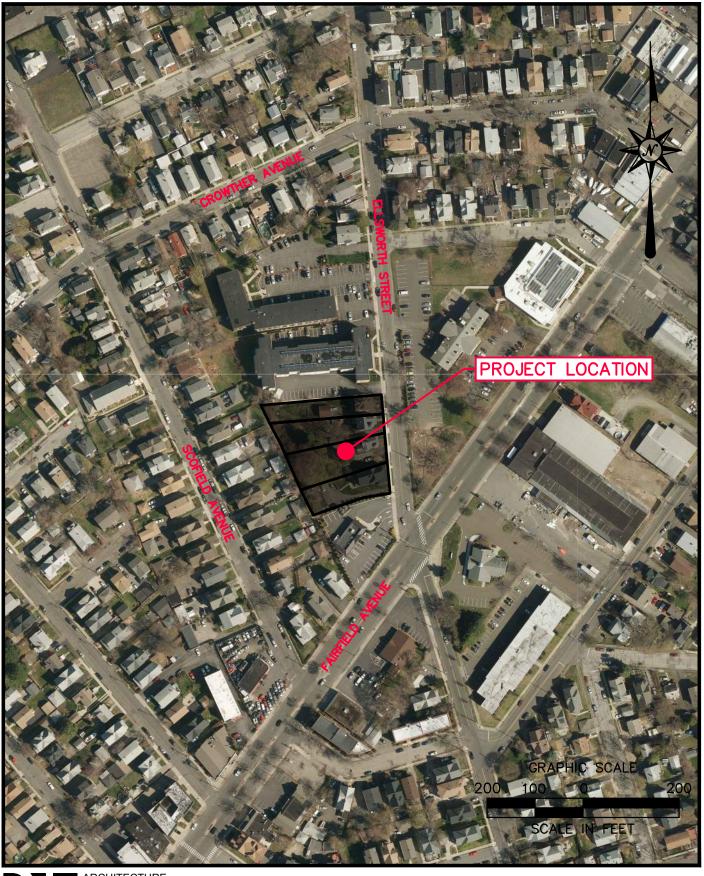
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#### APPENDIX A

LOCATION MAPS Figure 1: USGS Location Map Figure 2: Aerial Location Map Figure 3: NRCS Soil Survey Map with Hydrologic Soil Group Data Figure 4: FEMA Federal Insurance Rate Map Figure 5: NOAA Atlas 14 Storm Data (Depth) Figure 6: NOAA Atlas 14 Storm Data (Intensity) Figure 7: City of Bridgeport Zoning Map





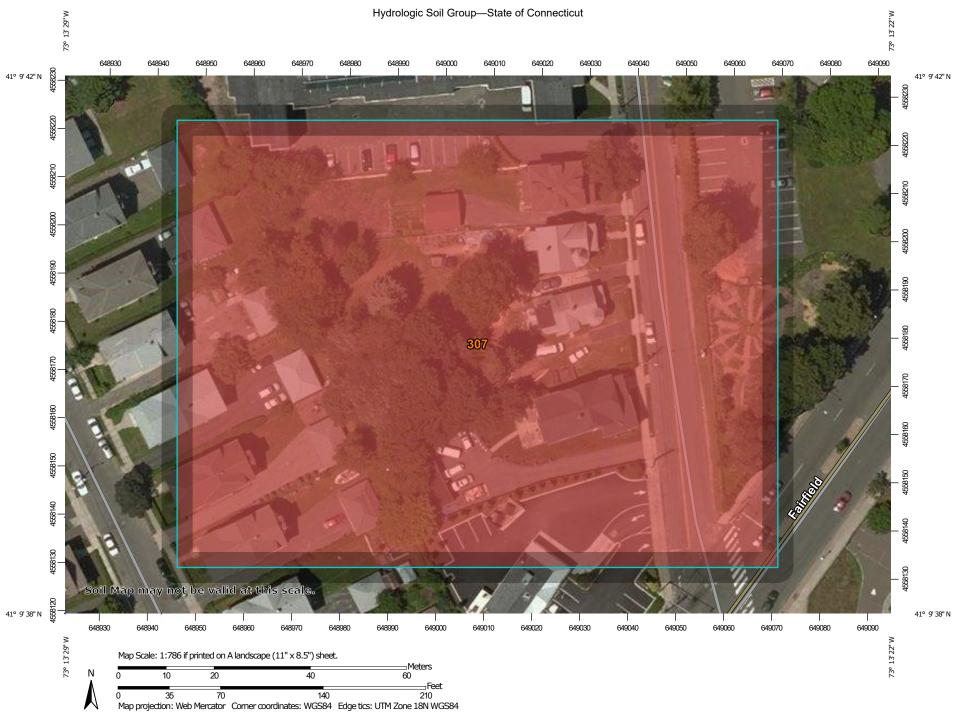


ARCHITECTURE ENGINEERING

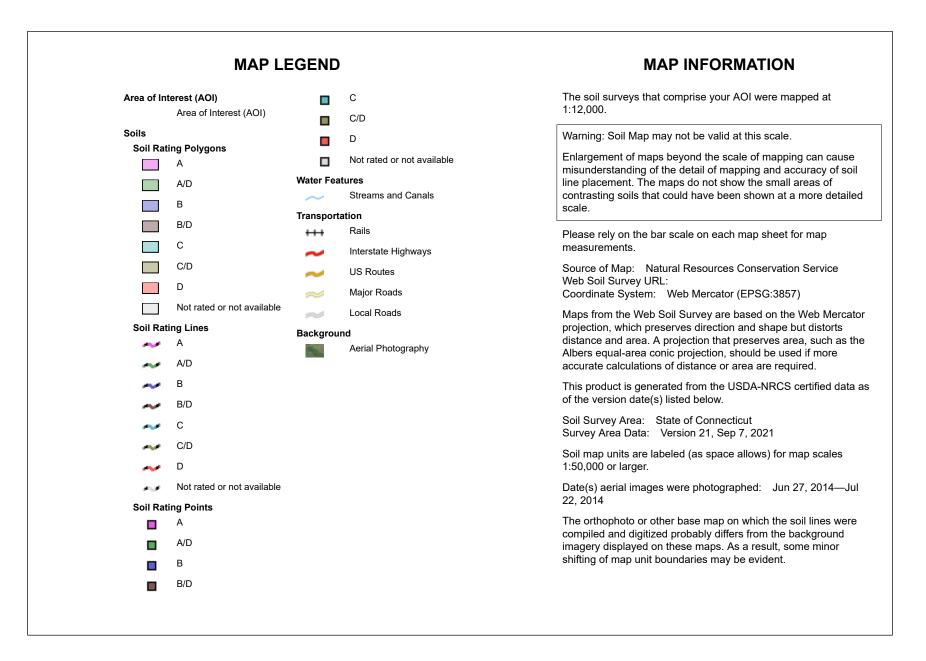
PROPOSED RESIDENTIAL ENGINEERINGDEVELOPMENTCnecked<br/>ApprovedS.M.K.<br/>ApprovedLAND SURVEYING543, 547, 549, 557ELLSWORTHST. Scale1"=200"<br/>Project No.BRIDGEPORT, CONNECTICUTDate12/03/2021<br/>CAD FileEXH210235702

Designed .R.J. Drawn Checked .R.J.





USDA Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey



### Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
307	Urban land	D	2.9	100.0%
Totals for Area of Intere	st		2.9	100.0%

### Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

### **Rating Options**

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

JSDA

Tie-break Rule: Higher

## NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) Report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS Report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study Report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study Report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control** structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study Report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Connecticut State Plane Zone (FIPS zone 0600). The horizontal datum was NAD 83, GRS 1980 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713- 3242, or visit its website at http://www.ngs.noaa.gov.

Base map information shown on FIRM panels produced for this coastal study revision was derived from United State Geological Survey 2008 High Resolution Orthophotography produced from 1 foot pixel cells from photography dated April 2008. The projection used in the preparation of this map was Connecticut State Plane Feet, FIPS Zone 0600. The horizontal datum used was North American Datum of 1983 (NAD 83).

The AE Zone category has been divided by a Limit of Moderate Wave Action (LIMWA). The LiMWA represents the approximate landward limit of the 1.5 foot breaking wave. The effects of wave hazards between the VE Zone and the LiMWA (or between the shoreline and the LiMWA for areaswhere VE Zones are not identified) will be similar to, but less severe than those in the VE Zone.

The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the profile baseline, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.

Based on updated topographic information, this map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction. As a result, the Flood Profiles and Floodway Data tables for multiple streams in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on the map. Also, the road to floodplain relationships for unrevised streams may differ from what is shown on previous maps.

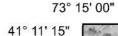
**Corporate limits** shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

For information on available products associated with this FIRM visit the Map Service Center (MSC) website at http://msc.fema.gov. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the MSC website.

If you have questions about this map, how to order products, or the National Flood Insurance Program in general, please call the FEMA Map Information eXchange (FMIX) at 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

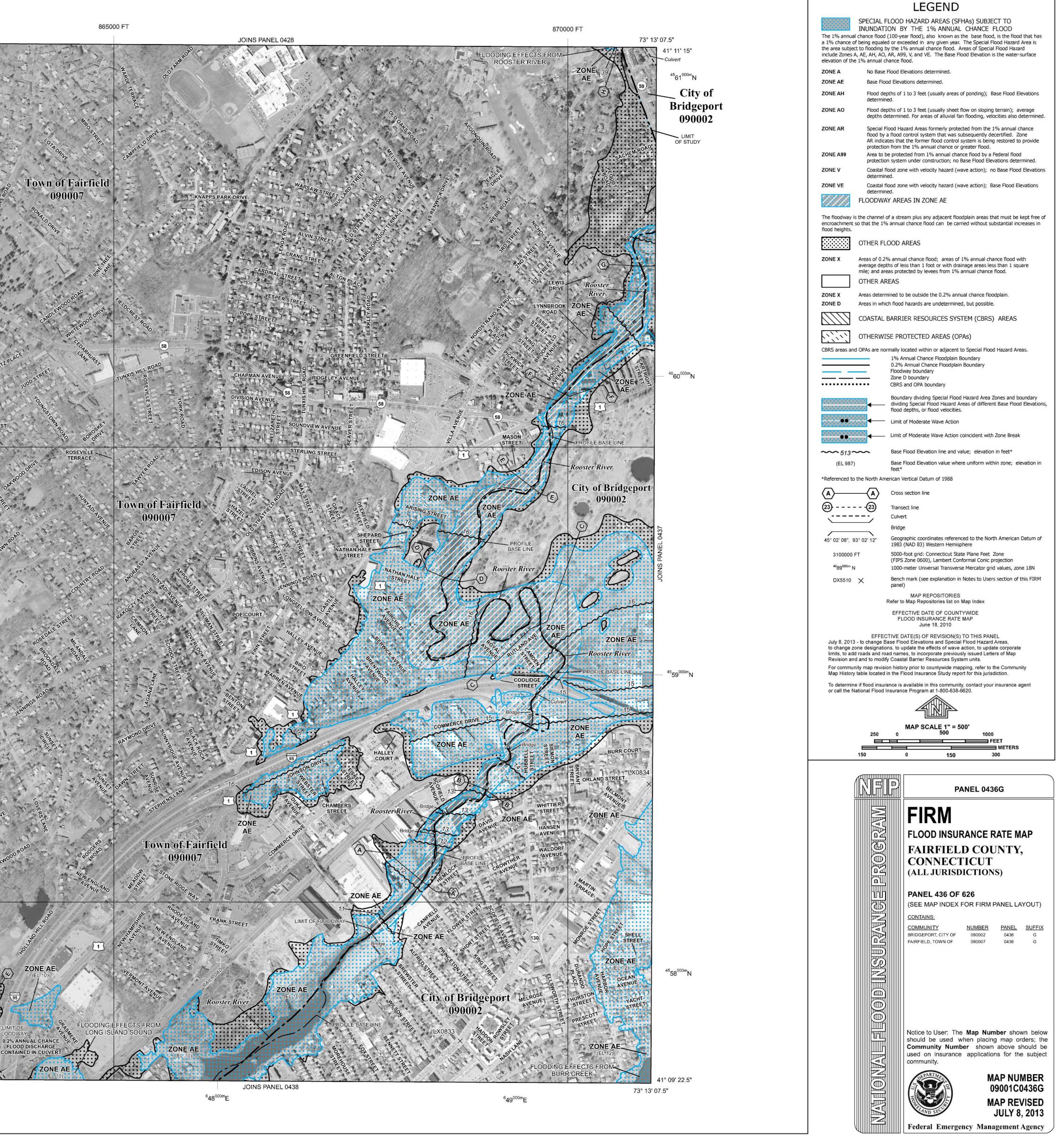
Only coastal structures that are certified to provide protection from the 1-percentannual chance flood are shown on this panel. However, all structures taken into consideration for the purpose of coastal flood hazard analysis and mapping are present in the FIRM database in S\_Gen\_Struct.



625000 FT 620000 FT Grasmer Brook 0.2% ANNUAL CHANCE FLOOD DISCHARGE CONTAINED IN CULVER

41° 09' 22.5"

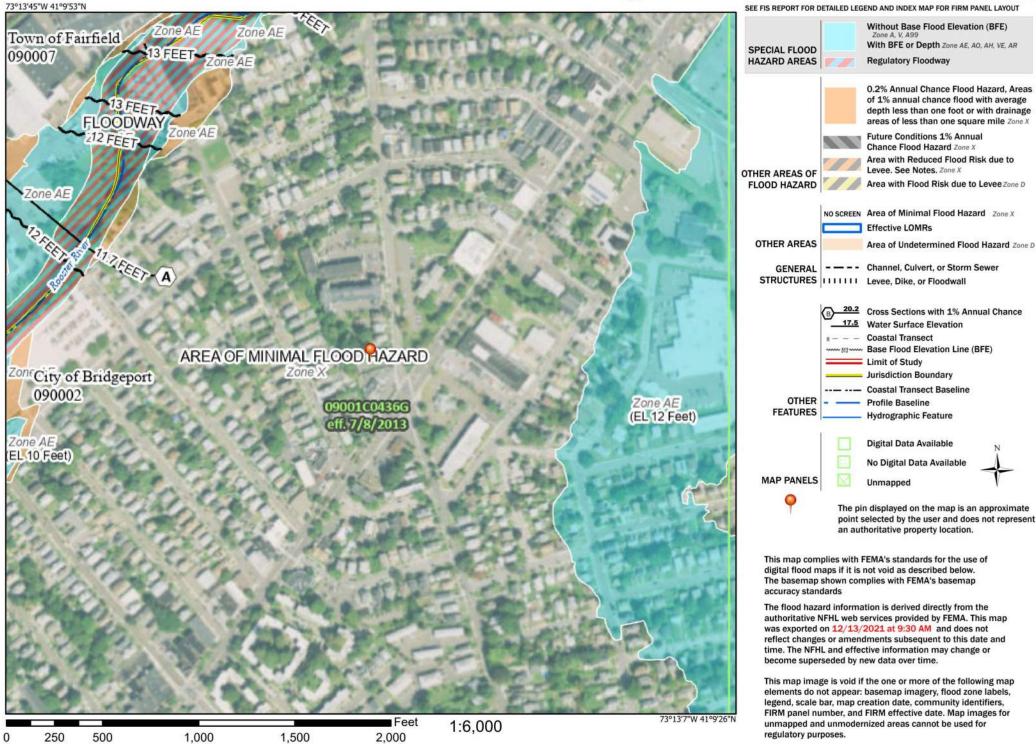
73° 15' 00" <sup>6</sup>47<sup>000m</sup>E



## National Flood Hazard Layer FIRMette



#### Legend



Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020



NOAA Atlas 14, Volume 10, Version 3 Location name: Bridgeport, Connecticut, USA\* Latitude: 41.1613°, Longitude: -73.2237° Elevation: 37.32 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

#### PF\_tabular | PF\_graphical | Maps\_&\_aerials

#### **PF** tabular

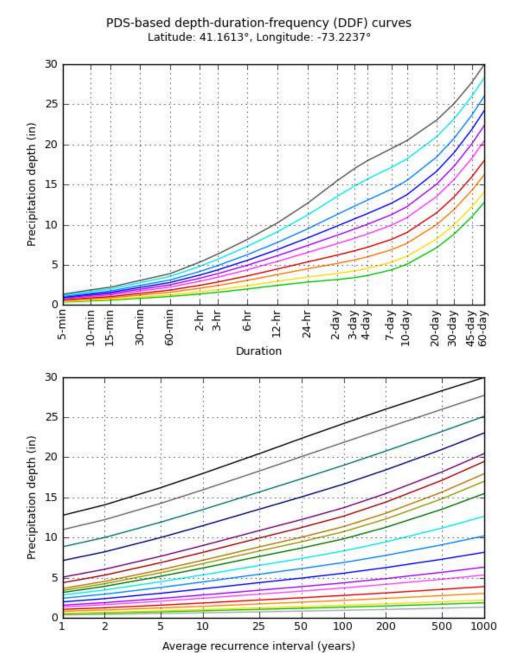
				Average	recurrence	interval (ve	ars)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.353</b> (0.281-0.438)	0.420	<b>0.529</b> (0.419-0.660)	0.620	<b>0.744</b> (0.564-0.973)	0.838	0.936	1.04	1.20	1.32
10-min	<b>0.500</b> (0.398-0.621)	<b>0.595</b> (0.472-0.739)	<b>0.750</b> (0.594-0.936)	<b>0.878</b> (0.691-1.10)	<b>1.05</b> (0.799-1.38)	<b>1.19</b> (0.880-1.58)	<b>1.33</b> (0.950-1.83)	<b>1.48</b> (1.00-2.09)	<b>1.70</b> (1.10-2.48)	<b>1.87</b> (1.19-2.80)
15-min	<b>0.588</b> (0.468-0.731)	<b>0.699</b> (0.556-0.870)	<b>0.881</b> (0.698-1.10)	<b>1.03</b> (0.812-1.30)	<b>1.24</b> (0.940-1.62)	<b>1.40</b> (1.04-1.86)	<b>1.56</b> (1.12-2.16)	<b>1.74</b> (1.18-2.46)	<b>2.00</b> (1.30-2.92)	<b>2.20</b> (1.39-3.29)
30-min	<b>0.820</b> (0.653-1.02)	<b>0.975</b> (0.775-1.21)	<b>1.23</b> (0.973-1.53)	<b>1.44</b> (1.13-1.81)	<b>1.73</b> (1.31-2.26)	<b>1.95</b> (1.44-2.60)	<b>2.17</b> (1.56-3.00)	<b>2.42</b> (1.64-3.43)	<b>2.77</b> (1.80-4.05)	<b>3.05</b> (1.93-4.55)
60-min	<b>1.05</b> (0.837-1.31)	<b>1.25</b> (0.994-1.56)	<b>1.58</b> (1.25-1.97)	<b>1.85</b> (1.45-2.32)	<b>2.22</b> (1.68-2.90)	<b>2.50</b> (1.85-3.33)	<b>2.79</b> (1.99-3.84)	<b>3.10</b> (2.10-4.39)	<b>3.54</b> (2.30-5.18)	<b>3.89</b> (2.46-5.81)
2-hr	<b>1.36</b> (1.09-1.68)	<b>1.63</b> (1.31-2.02)	<b>2.07</b> (1.65-2.57)	<b>2.44</b> (1.93-3.04)	<b>2.95</b> (2.25-3.83)	<b>3.33</b> (2.48-4.41)	<b>3.72</b> (2.69-5.12)	<b>4.17</b> (2.84-5.86)	<b>4.81</b> (3.14-6.99)	<b>5.33</b> (3.39-7.91)
3-hr	<b>1.57</b> (1.26-1.93)	<b>1.89</b> (1.52-2.32)	<b>2.41</b> (1.93-2.98)	<b>2.84</b> (2.26-3.53)	<b>3.44</b> (2.63-4.45)	<b>3.88</b> (2.91-5.14)	<b>4.35</b> (3.16-5.98)	<b>4.89</b> (3.33-6.85)	<b>5.67</b> (3.70-8.21)	<b>6.31</b> (4.01-9.32)
6-hr	<b>1.98</b> (1.60-2.42)	<b>2.39</b> (1.93-2.92)	<b>3.06</b> (2.46-3.75)	<b>3.61</b> (2.89-4.45)	<b>4.38</b> (3.38-5.64)	<b>4.95</b> (3.73-6.51)	<b>5.56</b> (4.06-7.60)	<b>6.26</b> (4.28-8.70)	<b>7.30</b> (4.78-10.5)	<b>8.16</b> (5.21-12.0)
12 <b>-</b> hr	<b>2.44</b> (1.99-2.95)	<b>2.95</b> (2.40-3.57)	<b>3.78</b> (3.07-4.60)	<b>4.47</b> (3.60-5.47)	<b>5.42</b> (4.21-6.94)	<b>6.13</b> (4.65-8.02)	<b>6.89</b> (5.06-9.37)	<b>7.78</b> (5.35-10.7)	<b>9.10</b> (5.98-13.0)	<b>10.2</b> (6.53-14.9)
24-hr	<b>2.84</b> (2.33-3.42)	<b>3.47</b> (2.84-4.18)	<b>4.50</b> (3.67-5.43)	<b>5.35</b> (4.34-6.50)	<b>6.52</b> (5.10-8.31)	<b>7.39</b> (5.65-9.63)	<b>8.33</b> (6.17-11.3)	<b>9.46</b> (6.52-13.0)	<b>11.2</b> (7.37-15.9)	<b>12.6</b> (8.12-18.3)
2-day	<b>3.16</b> (2.61-3.77)	<b>3.92</b> (3.24-4.69)	<b>5.17</b> (4.25-6.20)	<b>6.21</b> (5.07-7.49)	<b>7.64</b> (6.02-9.69)	<b>8.69</b> (6.70-11.3)	<b>9.84</b> (7.37-13.3)	<b>11.3</b> (7.80-15.4)	<b>13.5</b> (8.94-19.0)	<b>15.5</b> (9.97-22.2)
3-day	<b>3.41</b> (2.83-4.05)	<b>4.25</b> (3.52-5.06)	<b>5.62</b> (4.64-6.71)	<b>6.76</b> (5.54-8.11)	<b>8.32</b> (6.58-10.5)	<b>9.47</b> (7.33-12.3)	<b>10.7</b> (8.07-14.5)	<b>12.3</b> (8.54-16.7)	<b>14.8</b> (9.82-20.8)	<b>17.0</b> (11.0-24.3)
4-day	<b>3.65</b> (3.04-4.33)	<b>4.54</b> (3.77-5.38)	<b>5.98</b> (4.95-7.12)	<b>7.18</b> (5.90-8.59)	<b>8.82</b> (7.00-11.1)	<b>10.0</b> (7.79-12.9)	<b>11.4</b> (8.56-15.3)	<b>13.0</b> (9.05-17.6)	<b>15.7</b> (10.4-21.9)	<b>18.0</b> (11.6-25.6)
7 <b>-</b> day	<b>4.37</b> (3.65-5.15)	<b>5.32</b> (4.44-6.27)	<b>6.87</b> (5.72-8.13)	<b>8.16</b> (6.74-9.71)	<b>9.93</b> (7.91-12.4)	<b>11.2</b> (8.75-14.4)	<b>12.7</b> (9.55-16.9)	<b>14.4</b> (10.1-19.4)	<b>17.1</b> (11.4-23.8)	<b>19.5</b> (12.6-27.6)
10-day	<b>5.06</b> (4.25-5.94)	<b>6.05</b> (5.07-7.11)	<b>7.67</b> (6.41-9.04)	<b>9.01</b> (7.48-10.7)	<b>10.9</b> (8.67-13.5)	<b>12.2</b> (9.53-15.5)	<b>13.7</b> (10.3-18.1)	<b>15.5</b> (10.8-20.7)	<b>18.2</b> (12.1-25.1)	<b>20.5</b> (13.3-28.8)
20-day	<b>7.13</b> (6.03-8.31)	<b>8.22</b> (6.95-9.59)	<b>10.0</b> (8.41-11.7)	<b>11.5</b> (9.59-13.5)	<b>13.5</b> (10.8-16.6)	<b>15.1</b> (11.7-18.8)	<b>16.7</b> (12.5-21.6)	<b>18.4</b> (13.0-24.4)	<b>21.0</b> (14.1-28.8)	<b>23.0</b> (15.0-32.2)
30-day	<b>8.85</b> (7.52-10.3)	<b>10.0</b> (8.49-11.6)	<b>11.9</b> (10.1-13.9)	<b>13.5</b> (11.3-15.8)	<b>15.7</b> (12.6-19.0)	<b>17.3</b> (13.5-21.5)	<b>19.0</b> (14.2-24.3)	<b>20.8</b> (14.7-27.4)	<b>23.2</b> (15.6-31.7)	<b>25.1</b> (16.4-35.0)
45-day	<b>11.0</b> (9.37-12.7)	<b>12.2</b> (10.4-14.1)	<b>14.3</b> (12.1-16.6)	<b>16.0</b> (13.4-18.6)	<b>18.3</b> (14.7-22.1)	<b>20.1</b> (15.7-24.7)	<b>21.9</b> (16.3-27.7)	<b>23.7</b> (16.8-31.0)	<b>26.0</b> (17.5-35.3)	<b>27.7</b> (18.1-38.4)
60-day	<b>12.8</b> (10.9-14.7)	<b>14.1</b> (12.0-16.2)	<b>16.2</b> (13.8-18.8)	<b>18.0</b> (15.2-20.9)	<b>20.4</b> (16.5-24.6)	<b>22.4</b> (17.5-27.3)	<b>24.2</b> (18.1-30.5)	<b>26.0</b> (18.5-34.0)	<b>28.3</b> (19.1-38.3)	<b>30.0</b> (19.6-41.4)

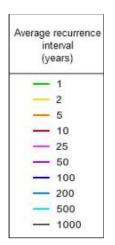
<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

Back to Top

**PF graphical** 





Dura	ation
— 5-min	— 2-day
- 10-min	— 3-day
- 15-min	— 4-day
— 30-min	— 7-day
- 60-min	- 10-day
- 2-hr	- 20-day
- 3-hr	- 30-day
- 6-hr	— 45-day
- 12-hr	— 60-day
24-hr	

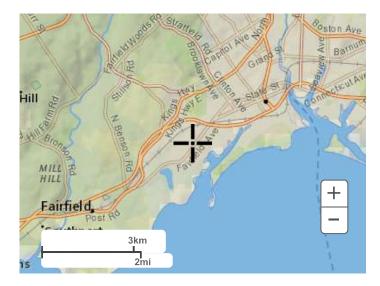
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Back to Top

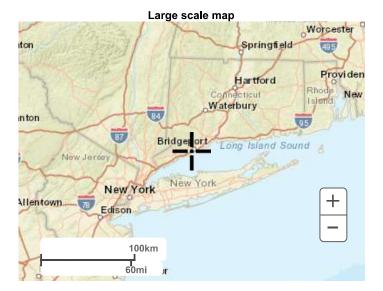
Maps & aerials

Small scale terrain



Large scale terrain





Large scale aerial



Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

**Disclaimer** 



NOAA Atlas 14, Volume 10, Version 3 Location name: Bridgeport, Connecticut, USA\* Latitude: 41.1613°, Longitude: -73.2237° Elevation: 37.32 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

#### PF\_tabular | PF\_graphical | Maps\_&\_aerials

#### **PF** tabular

				Avera	ge recurren	ce interval (v	/ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>4.24</b> (3.37-5.26)	<b>5.04</b> (4.00-6.26)	<b>6.35</b> (5.03-7.92)	<b>7.44</b> (5.84-9.34)	<mark>8.93</mark> (6.77-11.7)	<b>10.1</b> (7.46-13.4)	<b>11.2</b> (8.05-15.5)	<b>12.5</b> (8.50-17.7)	<b>14.4</b> (9.34-21.0)	<b>15.9</b> (10.0-23.7)
10-min	<b>3.00</b>	<b>3.57</b>	<b>4.50</b>	<b>5.27</b>	<b>6.32</b>	<b>7.12</b>	<b>7.96</b>	<b>8.87</b>	<b>10.2</b>	<b>11.2</b>
	(2.39-3.73)	(2.83-4.43)	(3.56-5.62)	(4.15-6.61)	(4.79-8.27)	(5.28-9.50)	(5.70-11.0)	(6.02-12.6)	(6.61-14.9)	(7.11-16.8)
15-min	<b>2.35</b>	<b>2.80</b>	<b>3.52</b>	<b>4.13</b>	<b>4.96</b>	<b>5.59</b>	<b>6.24</b>	<b>6.96</b>	<b>7.98</b>	<b>8.81</b>
	(1.87-2.92)	(2.22-3.48)	(2.79-4.40)	(3.25-5.18)	(3.76-6.48)	(4.14-7.46)	(4.47-8.62)	(4.72-9.85)	(5.18-11.7)	(5.58-13.2)
30-min	<b>1.64</b>	<b>1.95</b>	<b>2.46</b>	<b>2.88</b>	<b>3.46</b>	<b>3.89</b>	<b>4.35</b>	<b>4.84</b>	<b>5.54</b>	<b>6.09</b>
	(1.31-2.04)	(1.55-2.43)	(1.95-3.06)	(2.26-3.61)	(2.62-4.52)	(2.88-5.19)	(3.11-6.00)	(3.28-6.85)	(3.60-8.11)	(3.86-9.10)
60-min	<b>1.05</b>	<b>1.25</b>	<b>1.58</b>	<b>1.85</b>	<b>2.22</b>	<b>2.50</b>	<b>2.79</b>	<b>3.10</b>	<b>3.54</b>	<b>3.89</b>
	(0.837-1.31)	(0.994-1.56)	(1.25-1.97)	(1.45-2.32)	(1.68-2.90)	(1.85-3.33)	(1.99-3.84)	(2.10-4.39)	(2.30-5.18)	(2.46-5.81)
2-hr	<b>0.682</b>	<b>0.816</b>	<b>1.04</b>	<b>1.22</b>	<b>1.47</b>	<b>1.66</b>	<b>1.86</b>	<b>2.09</b>	<b>2.40</b>	<b>2.67</b>
	(0.546-0.840)	(0.653-1.01)	(0.826-1.29)	(0.966-1.52)	(1.12-1.91)	(1.24-2.21)	(1.34-2.56)	(1.42-2.93)	(1.57-3.50)	(1.69-3.95)
3-hr	<b>0.523</b>	<b>0.629</b>	<b>0.803</b>	<b>0.947</b>	<b>1.15</b>	<b>1.29</b>	<b>1.45</b>	<b>1.63</b>	<b>1.89</b>	<b>2.10</b>
	(0.421-0.643)	(0.505-0.774)	(0.642-0.991)	(0.753-1.18)	(0.877-1.48)	(0.969-1.71)	(1.05-1.99)	(1.11-2.28)	(1.23-2.73)	(1.34-3.10)
6-hr	<b>0.330</b>	<b>0.399</b>	<b>0.511</b>	<b>0.603</b>	<b>0.731</b>	<b>0.826</b>	<b>0.928</b>	<b>1.05</b>	<b>1.22</b>	<b>1.36</b>
	(0.268-0.403)	(0.322-0.487)	(0.411-0.626)	(0.483-0.743)	(0.564-0.942)	(0.623-1.09)	(0.677-1.27)	(0.715-1.45)	(0.798-1.75)	(0.869-2.00)
12-hr	<b>0.202</b>	<b>0.245</b>	<b>0.314</b>	<b>0.371</b>	<b>0.450</b>	<b>0.509</b>	<b>0.572</b>	<b>0.646</b>	<b>0.755</b>	<b>0.848</b>
	(0.165-0.245)	(0.199-0.296)	(0.254-0.382)	(0.299-0.454)	(0.350-0.576)	(0.386-0.666)	(0.420-0.778)	(0.444-0.892)	(0.496-1.08)	(0.542-1.23)
24-hr	<b>0.118</b>	<b>0.145</b>	<b>0.187</b>	<b>0.223</b>	<b>0.272</b>	<b>0.308</b>	<b>0.347</b>	<b>0.394</b>	<b>0.466</b>	<b>0.527</b>
	(0.097-0.142)	(0.118-0.174)	(0.153-0.226)	(0.181-0.271)	(0.213-0.346)	(0.236-0.401)	(0.257-0.471)	(0.272-0.541)	(0.307-0.660)	(0.338-0.761
2-day	<b>0.066</b>	<b>0.082</b>	<b>0.108</b>	<b>0.129</b>	<b>0.159</b>	<b>0.181</b>	<b>0.205</b>	<b>0.235</b>	<b>0.282</b>	<b>0.323</b>
	(0.054-0.078)	(0.067-0.098)	(0.089-0.129)	(0.106-0.156)	(0.125-0.202)	(0.140-0.235)	(0.153-0.278)	(0.162-0.320)	(0.186-0.397)	(0.208-0.462
3-day	<b>0.047</b>	<b>0.059</b>	<b>0.078</b>	<b>0.094</b>	<b>0.116</b>	<b>0.132</b>	<b>0.149</b>	<b>0.171</b>	<b>0.206</b>	<b>0.236</b>
	(0.039-0.056)	(0.049-0.070)	(0.064-0.093)	(0.077-0.113)	(0.091-0.146)	(0.102-0.170)	(0.112-0.202)	(0.119-0.232)	(0.136-0.289)	(0.152-0.337
4-day	<b>0.038</b>	<b>0.047</b>	<b>0.062</b>	<b>0.075</b>	<b>0.092</b>	<b>0.105</b>	<b>0.118</b>	<b>0.136</b>	<b>0.163</b>	<b>0.187</b>
	(0.032-0.045)	(0.039-0.056)	(0.052-0.074)	(0.061-0.090)	(0.073-0.116)	(0.081-0.135)	(0.089-0.160)	(0.094-0.184)	(0.108-0.228)	(0.121-0.266
7-day	<b>0.026</b>	<b>0.032</b>	<b>0.041</b>	<b>0.049</b>	<b>0.059</b>	<b>0.067</b>	<b>0.075</b>	<b>0.086</b>	<b>0.102</b>	<b>0.116</b>
	(0.022-0.031)	(0.026-0.037)	(0.034-0.048)	(0.040-0.058)	(0.047-0.074)	(0.052-0.086)	(0.057-0.101)	(0.060-0.115)	(0.068-0.142)	(0.075-0.164
10-day	<b>0.021</b>	<b>0.025</b>	<b>0.032</b>	<b>0.038</b>	<b>0.045</b>	<b>0.051</b>	<b>0.057</b>	<b>0.065</b>	<b>0.076</b>	<b>0.085</b>
	(0.018-0.025)	(0.021-0.030)	(0.027-0.038)	(0.031-0.045)	(0.036-0.056)	(0.040-0.065)	(0.043-0.076)	(0.045-0.086)	(0.051-0.105)	(0.055-0.120
20-day	<b>0.015</b>	<b>0.017</b>	<b>0.021</b>	<b>0.024</b>	<b>0.028</b>	<b>0.031</b>	<b>0.035</b>	<b>0.038</b>	<b>0.044</b>	<b>0.048</b>
	(0.013-0.017)	(0.014-0.020)	(0.018-0.024)	(0.020-0.028)	(0.023-0.035)	(0.024-0.039)	(0.026-0.045)	(0.027-0.051)	(0.029-0.060)	(0.031-0.067
30-day	<b>0.012</b>	<b>0.014</b>	<b>0.017</b>	<b>0.019</b>	<b>0.022</b>	<b>0.024</b>	<b>0.026</b>	<b>0.029</b>	<b>0.032</b>	<b>0.035</b>
	(0.010-0.014)	(0.012-0.016)	(0.014-0.019)	(0.016-0.022)	(0.017-0.026)	(0.019-0.030)	(0.020-0.034)	(0.020-0.038)	(0.022-0.044)	(0.023-0.049
45-day	<b>0.010</b>	<b>0.011</b>	<b>0.013</b>	<b>0.015</b>	<b>0.017</b>	<b>0.019</b>	<b>0.020</b>	<b>0.022</b>	<b>0.024</b>	<b>0.026</b>
	(0.009-0.012)	(0.010-0.013)	(0.011-0.015)	(0.012-0.017)	(0.014-0.020)	(0.015-0.023)	(0.015-0.026)	(0.016-0.029)	(0.016-0.033)	(0.017-0.036
60-day	<b>0.009</b>	<b>0.010</b> (0.008-0.011)	<b>0.011</b> (0.010-0.013)	<b>0.012</b> (0.011-0.015)	<b>0.014</b> (0.011-0.017)	<b>0.016</b>	<b>0.017</b> (0.013-0.021)	<b>0.018</b> (0.013-0.024)	<b>0.020</b> (0.013-0.027)	<b>0.021</b> (0.014-0.029

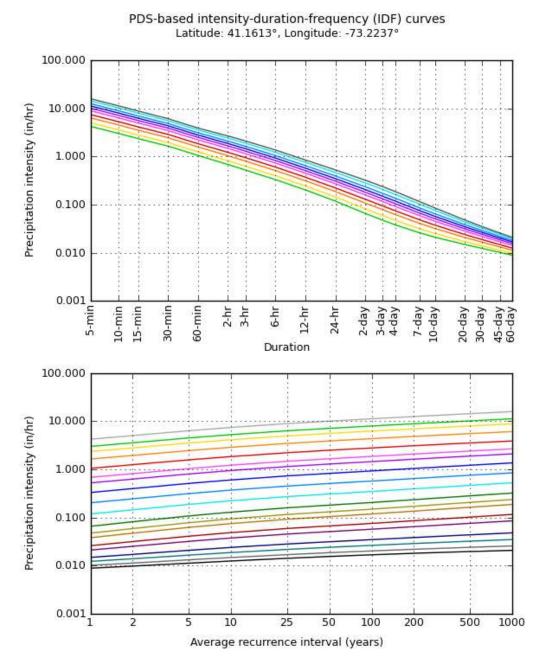
<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

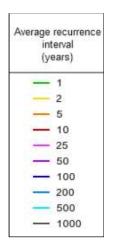
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

Back to Top

#### **PF graphical**





Dur	ation
— 5-min	— 2-day
- 10-min	— 3-day
- 15-min	— 4-day
- 30-min	— 7-day
- 60-min	- 10-day
- 2-hr	- 20-day
- 3-hr	- 30-day
6-hr	— 45-day
- 12-hr	- 60-day
24-hr	

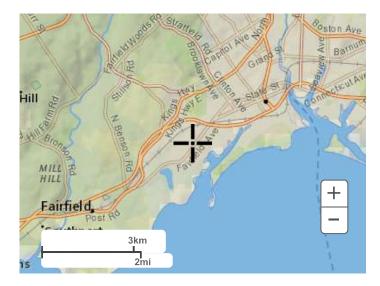
NOAA Atlas 14, Volume 10, Version 3

Created (GMT): Fri Dec 3 19:22:41 2021

Back to Top

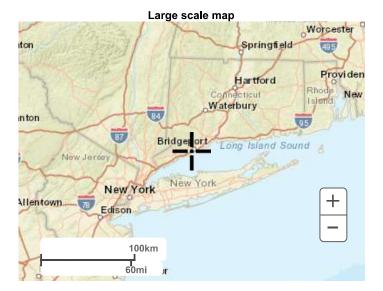
Maps & aerials

Small scale terrain



Large scale terrain





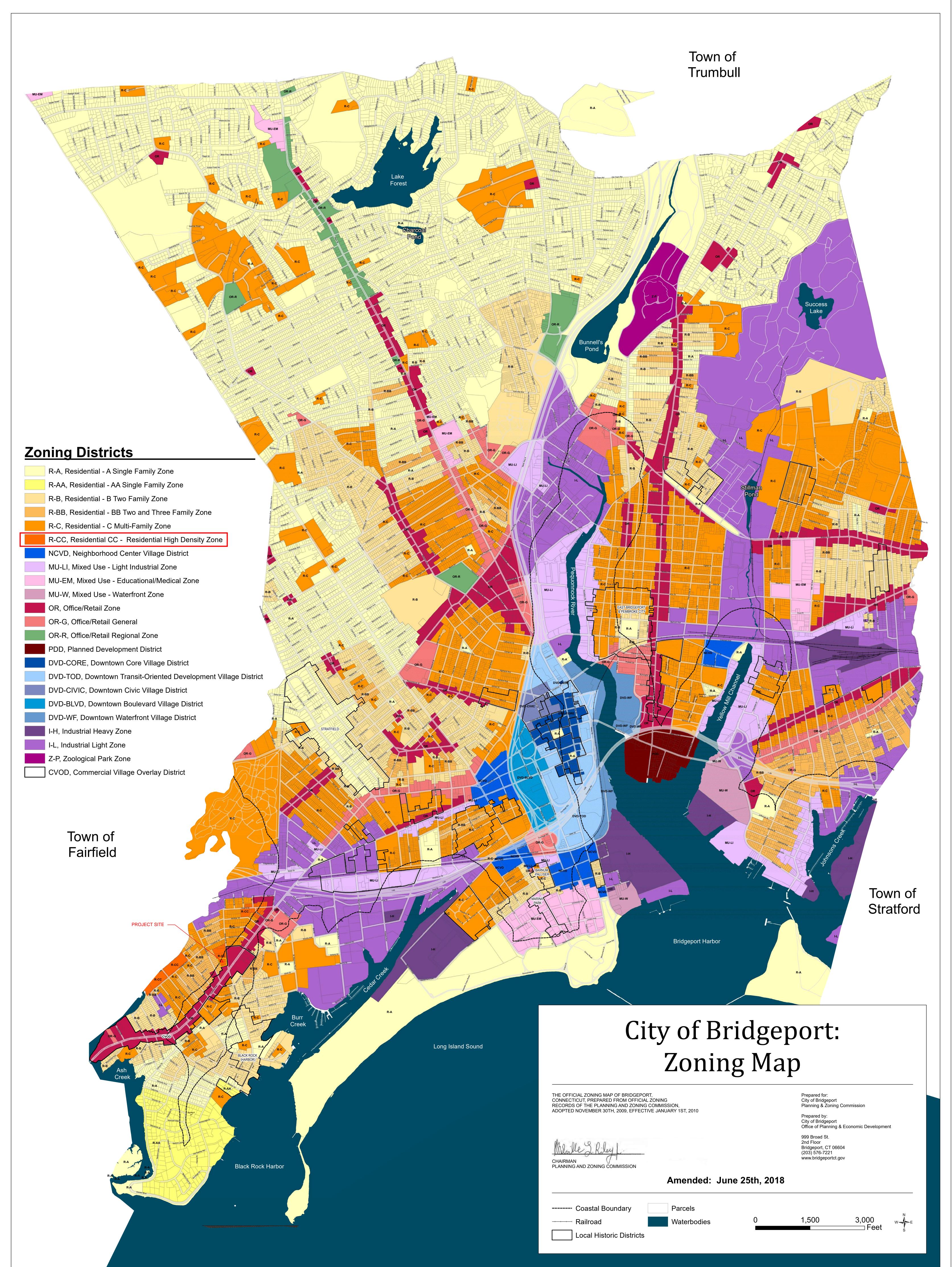
Large scale aerial



Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

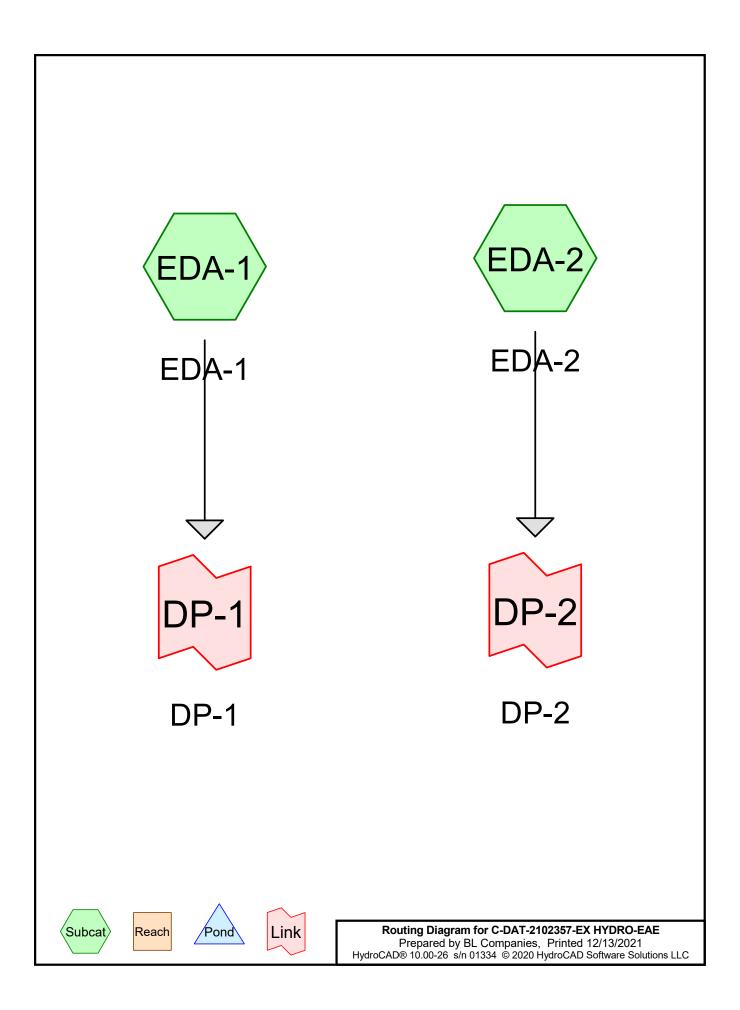
**Disclaimer** 





# APPENDIX B

# PRE-DEVELOPMENT HYDROLOGY (2-, 10-,25-, and 100-year storms)



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# **Project Notes**

Copied 10 events from CT-BRIDGEPORT\_NOAA14 24-hr S1 storm

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# Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
33,442	80	>75% Grass cover, Good, HSG D (EDA-1, EDA-2)
7,441	98	Paved parking, HSG D (EDA-1, EDA-2)
7,175	98	Unconnected roofs, HSG D (EDA-1)
48,058	85	TOTAL AREA

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# Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
0	HSG C	
48,058	HSG D	EDA-1, EDA-2
0	Other	
48,058		TOTAL AREA

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Printed 12/13/2021 Page 5

Ground Covers (an nodes)										
HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Su			
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Nu			
 0	0	0	33,442	0	33,442	>75% Grass				
						cover, Good				
0	0	0	7,441	0	7,441	Paved parking				
0	0	0	7,175	0	7,175	Unconnected				
						roofs				
0	0	0	48,058	0	48,058	TOTAL AREA				

# Ground Covers (all nodes)

#### Summary for Subcatchment EDA-1: EDA-1

Runoff = 2.00 cfs @ 12.12 hrs, Volume= 7,356 cf, Depth= 1.91"

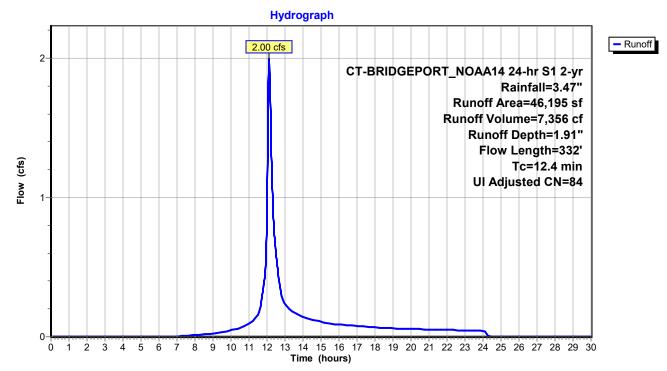
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 2-yr Rainfall=3.47"

A	rea (sf)	CN A	Adj Desc	ription							
	6,180	98	Pave	d parking,	HSG D						
	7,175	98	98 Unconnected roofs, HSG D								
	32,840	80									
	46,195	85	84 Weid	hted Avera	age, UI Adjusted						
	32,840			9% Perviou							
	13,355			1% Impervi							
	7,175			3% Unconr							
Tc	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·						
1.2	11	0.0450	0.16		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
1.3	15	0.0670	0.20		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
1.4	16	0.0625	0.19		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
1.0	12	0.0830	0.20		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
2.1	23	0.0430	0.18		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
1.2	14	0.0710	0.20		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
0.8	9	0.0670	0.18		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
0.1	6	0.0670	1.81		Shallow Concentrated Flow,						
0.4	00	0.0050	5 50		Short Grass Pasture Kv= 7.0 fps						
0.1	32	0.6250	5.53		Shallow Concentrated Flow,						
0.1	11	0.0710	1 07		Short Grass Pasture Kv= 7.0 fps						
0.1	14	0.0710	1.87		Shallow Concentrated Flow,						
0.1	11	0.0910	2.11		Short Grass Pasture Kv= 7.0 fps						
0.1	11	0.0910	2.11		Shallow Concentrated Flow,						
0.2	21	0.0476	1.53		Short Grass Pasture Kv= 7.0 fps						
0.2	21	0.0470	1.55		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps						
0.3	27	0.0370	1.35		Shallow Concentrated Flow,						
0.5	21	0.0370	1.55		Short Grass Pasture Kv= 7.0 fps						
0.4	29	0.0340	1.29		Shallow Concentrated Flow,						
0.4	23	0.0040	1.25		Short Grass Pasture Kv= 7.0 fps						
2.1	92	0.0110	0.73		Shallow Concentrated Flow,						
۲.۱	52	0.0110	0.70		Short Grass Pasture Kv= 7.0 fps						
12.4	220	Total									
12.4	552	TUIAI									

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Page 7



#### Subcatchment EDA-1: EDA-1

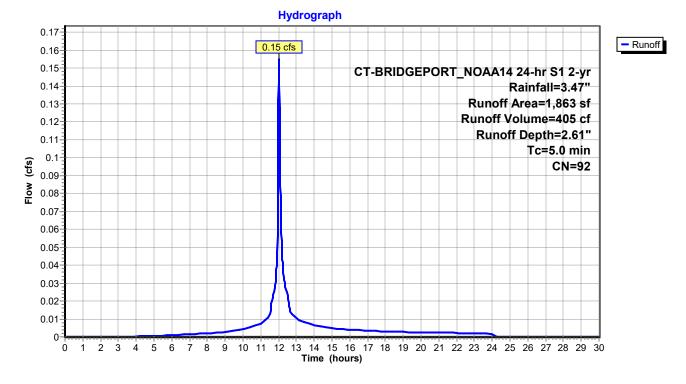
#### Summary for Subcatchment EDA-2: EDA-2

Runoff = 0.15 cfs @ 12.03 hrs, Volume= 405 cf, Depth= 2.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 2-yr Rainfall=3.47"

A	rea (sf)	CN	Description		
	1,261	98	Paved park	ing, HSG D	D
	602	80	>75% Gras	s cover, Go	ood, HSG D
	1,863	92	Weighted A	verage	
	602		32.31% Pei	rvious Area	а
	1,261		67.69% Imp	pervious Ar	rea
Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description
5.0					Direct Entry,

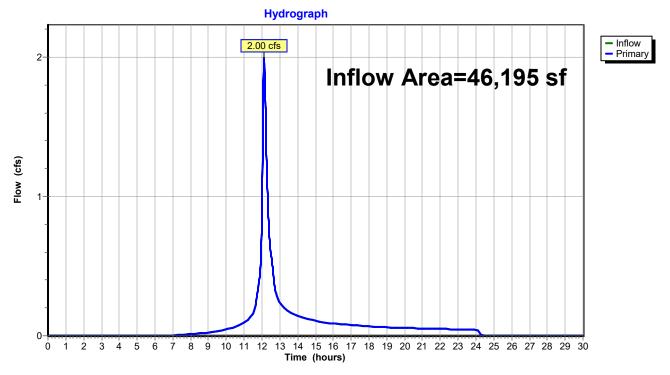
#### Subcatchment EDA-2: EDA-2



## Summary for Link DP-1: DP-1

Inflow Area	a =	46,195 sf, 28.91% Impervious, Inflow Depth = 1.91" for 2-yr event	
Inflow	=	2.00 cfs @ 12.12 hrs, Volume= 7,356 cf	
Primary	=	2.00 cfs $\overline{@}$ 12.12 hrs, Volume= 7,356 cf, Atten= 0%, Lag= 0.0 mi	in

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

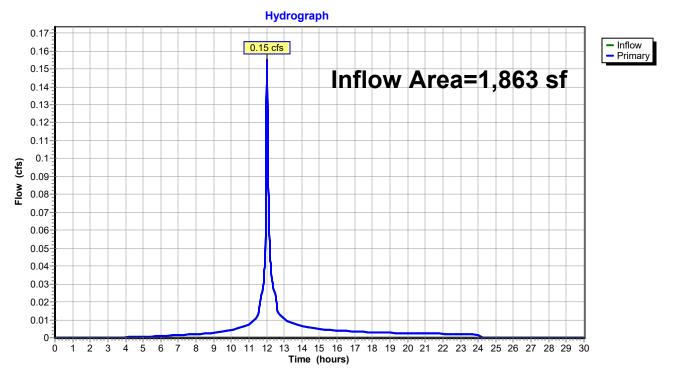


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	1,863 sf,  67.69% Ir	npervious,	Inflow Depth =	2.61"	for 2-yr event
Inflow	=	0.15 cfs @ 12.03 hrs,	Volume=	405 c	f	
Primary	=	0.15 cfs @ 12.03 hrs,	Volume=	405 c	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



#### Link DP-2: DP-2

#### Summary for Subcatchment EDA-1: EDA-1

Runoff = 3.59 cfs @ 12.12 hrs, Volume= 13,828 cf, Depth= 3.59"

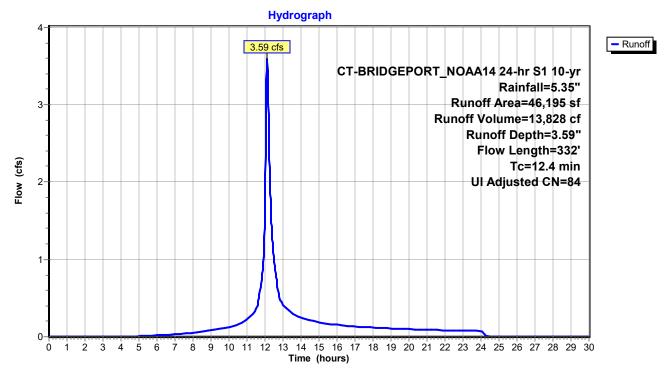
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 10-yr Rainfall=5.35"

A	rea (sf)	CN A	Adj Desc	ription							
	6,180	98	Pave	d parking,	HSG D						
	7,175	98									
	32,840	80	80 >75% Grass cover, Good, HSG D								
	46,195	85	84 Weig	hted Avera	age, UI Adjusted						
	32,840			9% Perviou							
	13,355			1% Impervi							
	7,175		53.73	3% Unconr	nected						
-		01		0							
Tc (mim)	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
1.2	11	0.0450	0.16		Sheet Flow,						
1.3	15	0.0670	0.20		Grass: Short n= 0.150 P2= 3.47" Sheet Flow,						
1.5	15	0.0070	0.20		Grass: Short $n = 0.150$ P2= 3.47"						
1.4	16	0.0625	0.19		Sheet Flow,						
1.4	10	0.0025	0.19		Grass: Short $n= 0.150$ P2= 3.47"						
1.0	12	0.0830	0.20		Sheet Flow,						
1.0	14	0.0000	0.20		Grass: Short n= 0.150 P2= 3.47"						
2.1	23	0.0430	0.18		Sheet Flow,						
			•••••		Grass: Short n= 0.150 P2= 3.47"						
1.2	14	0.0710	0.20		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
0.8	9	0.0670	0.18		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
0.1	6	0.0670	1.81		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
0.1	32	0.6250	5.53		Shallow Concentrated Flow,						
0.4		0.0740	4.07		Short Grass Pasture Kv= 7.0 fps						
0.1	14	0.0710	1.87		Shallow Concentrated Flow,						
0.4	4.4	0.0040	0.44		Short Grass Pasture Kv= 7.0 fps						
0.1	11	0.0910	2.11		Shallow Concentrated Flow,						
0.2	21	0.0476	1.53		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,						
0.2	21	0.0470	1.55		Short Grass Pasture Kv= 7.0 fps						
0.3	27	0.0370	1.35		Shallow Concentrated Flow,						
0.0	21	0.0070	1.00		Short Grass Pasture Kv= 7.0 fps						
0.4	29	0.0340	1.29		Shallow Concentrated Flow,						
0.1	20	0.0010	1.20		Short Grass Pasture Kv= 7.0 fps						
2.1	92	0.0110	0.73		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
12.4	332	Total			· · · · ·						

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Page 12



#### Subcatchment EDA-1: EDA-1

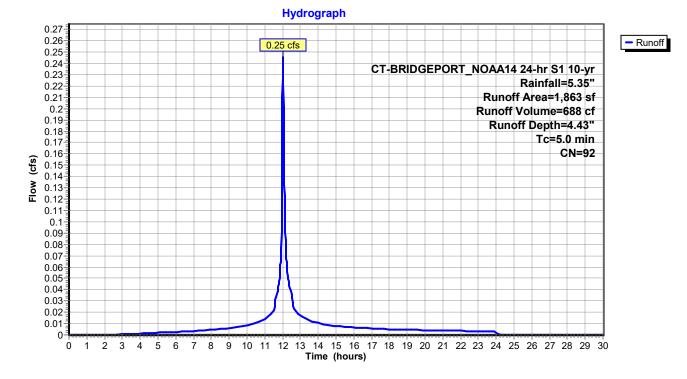
#### Summary for Subcatchment EDA-2: EDA-2

Runoff = 0.25 cfs @ 12.03 hrs, Volume= 688 cf, Depth= 4.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 10-yr Rainfall=5.35"

A	rea (sf)	CN	Description		
	1,261	98	Paved park	ing, HSG D	
	602	80	>75% Gras	s cover, Go	ood, HSG D
	1,863	92	Weighted A	verage	
	602		32.31% Pe	rvious Area	3
	1,261		67.69% Imp	rea	
Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description
5.0					Direct Entry,

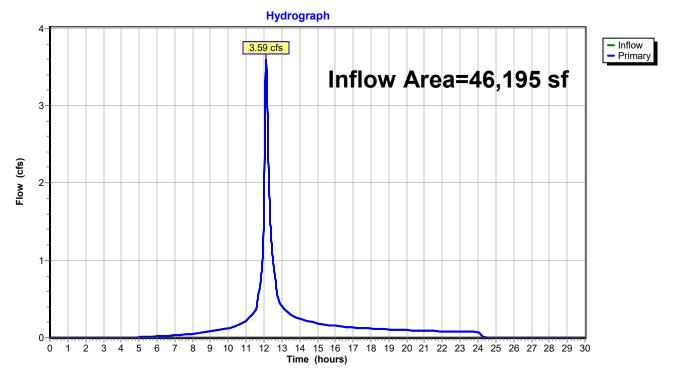
#### Subcatchment EDA-2: EDA-2



## Summary for Link DP-1: DP-1

Inflow Area	a =	46,195 sf, 28.91% Impervious, Inflow Depth = 3.59" for 10-yr event	
Inflow	=	3.59 cfs @ 12.12 hrs, Volume= 13,828 cf	
Primary	=	3.59 cfs @ 12.12 hrs, Volume= 13,828 cf, Atten= 0%, Lag= 0.0 r	min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

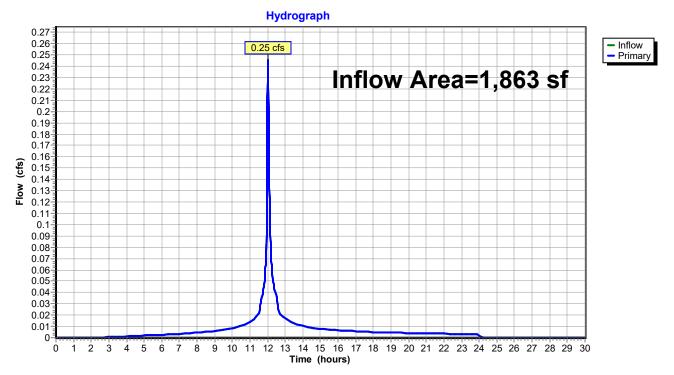


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	1,863 sf, 67.69% Impervious, Inflow	Depth = 4.43" for 10-yr event
Inflow	=	0.25 cfs @ 12.03 hrs, Volume=	688 cf
Primary	=	0.25 cfs @ 12.03 hrs, Volume=	688 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



#### Link DP-2: DP-2

#### Summary for Subcatchment EDA-1: EDA-1

Runoff = 4.58 cfs @ 12.12 hrs, Volume= 18,037 cf, Depth= 4.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 25-yr Rainfall=6.52"

A	rea (sf)	CN A	Adj Desc	ription		
	6,180	98	98 Paved parking, HSG D			
	7,175	98			oofs, HSG D	
	32,840	80	80 >75% Grass cover, Good, HSG D			
	46,195	85	84 Weig	hted Avera	age, UI Adjusted	
	32,840			9% Perviou		
	13,355			1% Impervi		
	7,175		53.73	3% Unconr	nected	
-		01		0		
Tc (mim)	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
1.2	11	0.0450	0.16		Sheet Flow,	
1.3	15	0.0670	0.00		Grass: Short n= 0.150 P2= 3.47" Sheet Flow,	
1.3	15	0.0070	0.20		Grass: Short $n = 0.150$ P2= 3.47"	
1.4	16	0.0625	0.19		Sheet Flow,	
1.4	10	0.0025	0.19		Grass: Short $n= 0.150$ P2= 3.47"	
1.0	12	0.0830	0.20		Sheet Flow,	
1.0	12	0.0000	0.20		Grass: Short n= 0.150 P2= 3.47"	
2.1	23	0.0430	0.18		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
1.2	14	0.0710	0.20		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
0.8	9	0.0670	0.18		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
0.1	6	0.0670	1.81		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
0.1	32	0.6250	5.53		Shallow Concentrated Flow,	
0.4		0.0740	4.07		Short Grass Pasture Kv= 7.0 fps	
0.1	14	0.0710	1.87		Shallow Concentrated Flow,	
0.4	4.4	0.0040	0.44		Short Grass Pasture Kv= 7.0 fps	
0.1	11	0.0910	2.11		Shallow Concentrated Flow,	
0.2	21	0.0476	1.53		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow,	
0.2	21	0.0470	1.00		Short Grass Pasture Kv= 7.0 fps	
0.3	27	0.0370	1.35		Shallow Concentrated Flow,	
0.0	21	0.0070	1.00		Short Grass Pasture Kv= 7.0 fps	
0.4	29	0.0340	1.29		Shallow Concentrated Flow,	
0.1	20	0.0010	1.20		Short Grass Pasture Kv= 7.0 fps	
2.1	92	0.0110	0.73		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
12.4	332	Total			· · · · ·	

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Hydrograph 5 - Runoff 4.58 cfs CT-BRIDGEPORT\_NOAA14 24-hr S1 25-yr Rainfall=6.52" 4-Runoff Area=46,195 sf Runoff Volume=18,037 cf Runoff Depth=4.69" Flow Length=332' 3-Flow (cfs) Tc=12.4 min UI Adjusted CN=84 2-1 0-1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 Ó

Time (hours)

#### Subcatchment EDA-1: EDA-1

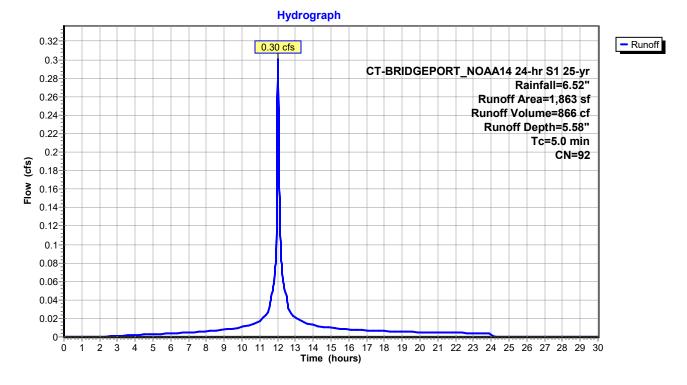
#### Summary for Subcatchment EDA-2: EDA-2

Runoff = 0.30 cfs @ 12.03 hrs, Volume= 866 cf, Depth= 5.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 25-yr Rainfall=6.52"

A	rea (sf)	CN	Description				
	1,261	98	Paved park	ing, HSG D	C		
	602	80	>75% Grass cover, Good, HSG D				
	1,863	92 Weighted Average					
	602		32.31% Pervious Area				
	1,261		67.69% Impervious Area				
Тс	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	,	(cfs)	Decemption		
5.0					Direct Entry,		

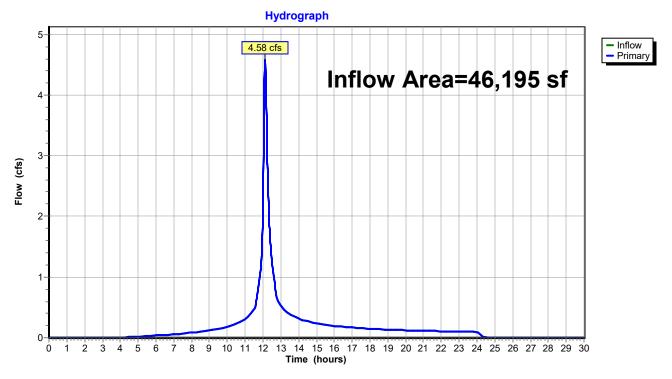
#### Subcatchment EDA-2: EDA-2



## Summary for Link DP-1: DP-1

Inflow Area =		46,195 sf, 28.91% Impervious, Inflow Depth = 4.69" for 25-yr	event
Inflow	=	4.58 cfs @ 12.12 hrs, Volume= 18,037 cf	
Primary	=	4.58 cfs @ 12.12 hrs, Volume= 18,037 cf, Atten= 0%, Lag	g= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

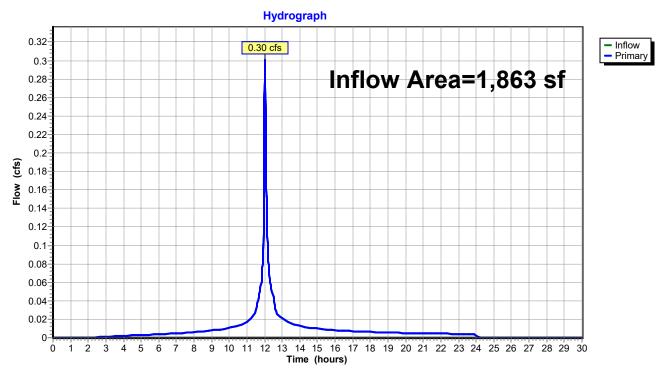


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	1,863 sf, 67.69% Impervious, Inflow Depth = 5.58" for 25-yr	event
Inflow	=	0.30 cfs @ 12.03 hrs, Volume= 866 cf	
Primary	=	0.30 cfs @ 12.03 hrs, Volume= 866 cf, Atten= 0%, La	g= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



#### Link DP-2: DP-2

## Summary for Subcatchment EDA-1: EDA-1

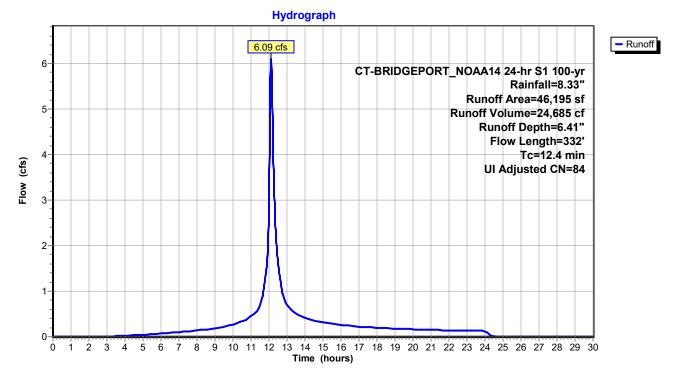
Runoff = 6.09 cfs @ 12.12 hrs, Volume= 24,685 cf, Depth= 6.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 100-yr Rainfall=8.33"

A	rea (sf)	CN /	Adj Desc	ription		
	6,180	98				
	7,175	98 Unconnected roofs, HSG D				
	32,840	80	80 >75% Grass cover, Good, HSG D			
	46,195	85	84 Weig	hted Avera	age, UI Adjusted	
	32,840		71.0	9% Perviou	is Area	
	13,355			1% Impervi		
	7,175		53.73	3% Unconr	nected	
т.	1 11.	0	V/.1	0	Description	
Tc (min)	Length	Slope	(ft/sec)	Capacity	Description	
(min)	(feet)	(ft/ft)	· /	(cfs)	Ohast Flow	
1.2	11	0.0450	0.16		Sheet Flow, Grass: Short n= 0.150 P2= 3.47"	
1.3	15	0.0670	0.20		Sheet Flow,	
1.5	15	0.0070	0.20		Grass: Short $n = 0.150$ P2= 3.47"	
1.4	16	0.0625	0.19		Sheet Flow,	
1.7	10	0.0025	0.15		Grass: Short n= 0.150 P2= 3.47"	
1.0	12	0.0830	0.20		Sheet Flow,	
		0.0000	0.20		Grass: Short n= 0.150 P2= 3.47"	
2.1	23	0.0430	0.18		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
1.2	14	0.0710	0.20		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
0.8	9	0.0670	0.18		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
0.1	6	0.0670	1.81		Shallow Concentrated Flow,	
0.4	00	0.0050	F F0		Short Grass Pasture Kv= 7.0 fps	
0.1	32	0.6250	5.53		Shallow Concentrated Flow,	
0.1	11	0 0710	1 07		Short Grass Pasture Kv= 7.0 fps	
0.1	14	0.0710	1.87		Shallow Concentrated Flow, Short Grass Pasture Kv= 7.0 fps	
0.1	11	0.0910	2.11		Shallow Concentrated Flow,	
0.1	11	0.0910	2.11		Short Grass Pasture Kv= 7.0 fps	
0.2	21	0.0476	1.53		Shallow Concentrated Flow,	
0.2	21	0.0470	1.00		Short Grass Pasture Kv= 7.0 fps	
0.3	27	0.0370	1.35		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
0.4	29	0.0340	1.29		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
2.1	92	0.0110	0.73		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
12.4	332	Total				

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Page 22



#### Subcatchment EDA-1: EDA-1

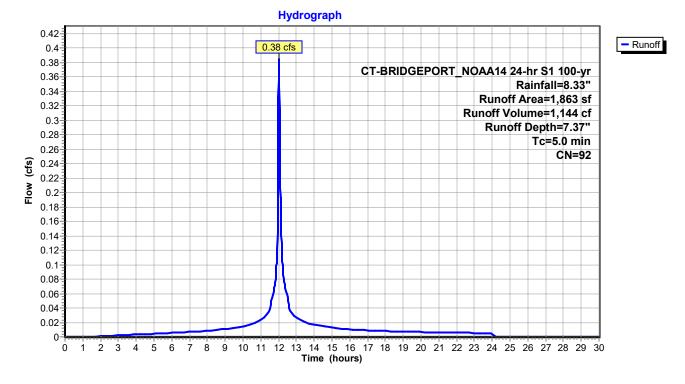
#### Summary for Subcatchment EDA-2: EDA-2

Runoff = 0.38 cfs @ 12.03 hrs, Volume= 1,144 cf, Depth= 7.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 100-yr Rainfall=8.33"

A	rea (sf)	CN	Description				
	1,261	98	Paved park	ing, HSG D	C		
	602	80	>75% Grass cover, Good, HSG D				
	1,863	92 Weighted Average					
	602		32.31% Pervious Area				
	1,261		67.69% Impervious Area				
Тс	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft	,	(cfs)	Decemption		
5.0					Direct Entry,		

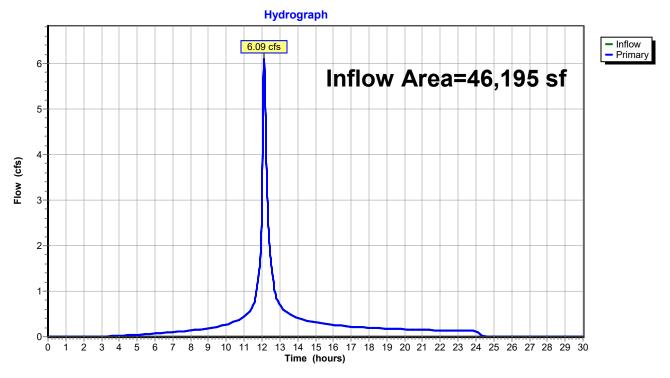
#### Subcatchment EDA-2: EDA-2



## Summary for Link DP-1: DP-1

Inflow Area	a =	46,195 sf, 28.91% Impervious, Inflow Depth = 6.41" for 10	)0-yr event
Inflow	=	6.09 cfs @ 12.12 hrs, Volume= 24,685 cf	
Primary	=	6.09 cfs $\overline{@}$ 12.12 hrs, Volume= 24,685 cf, Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

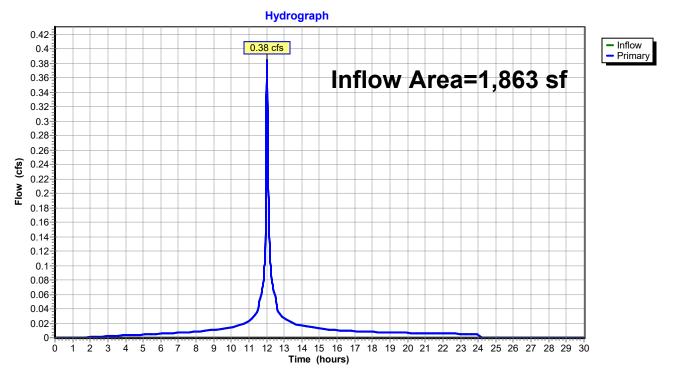


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	1,863 sf, 67.69% Impervious	, Inflow Depth = 7.37" for 100-yr event
Inflow	=	0.38 cfs @ 12.03 hrs, Volume=	1,144 cf
Primary	=	0.38 cfs @ 12.03 hrs, Volume=	1,144 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

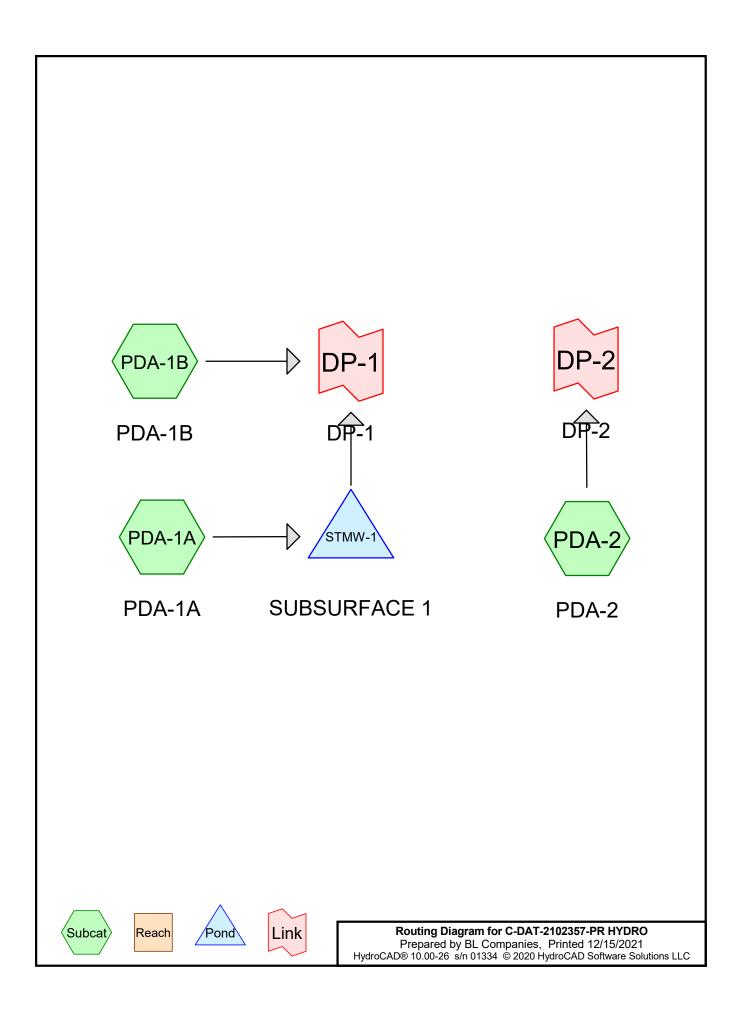


#### Link DP-2: DP-2



APPENDIX C

POST-DEVELOPMENT HYDROLOGY (2-, 10-,25-, and 100-year storms)



# **Project Notes**

Copied 10 events from CT-BRIDGEPORT\_NOAA14 24-hr S1 storm

# C-DAT-2102357-PR HYDRO

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# Area Listing (all nodes)

Area	CN	Description
(sq-ft)		(subcatchment-numbers)
10,329	80	>75% Grass cover, Good, HSG D (PDA-1B, PDA-2)
6,793	98	Courtyard (PDA-1A)
5,397	98	Paved parking, HSG D (PDA-1A, PDA-2)
25,539	98	Unconnected roofs, HSG D (PDA-1A, PDA-1B)
48,058	94	TOTAL AREA

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## Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
0	HSG A	
0	HSG B	
0	HSG C	
41,265	HSG D	PDA-1A, PDA-1B, PDA-2
6,793	Other	PDA-1A
48,058		TOTAL AREA

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HSG	A HSG-I	B HSG-C	HSG-E	D Other	Total	Ground	Su
(sq-	t) (sq-ft	) (sq-ft)	(sq-ft	t) (sq-ft)	(sq-ft)	Cover	Nu
	0	0 C	10,329	9 0	10,329	>75% Grass	-
						cover, Good	
	0	0 C	) (	0 6,793	6,793	Courtyard	
	0	0 C	5,397	7 0	5,397	Paved parking	
	0	0 C	25,539	9 0	25,539	Unconnected	
						roofs	
	0	0 0	41,26	5 6,793	48,058	TOTAL AREA	

## Ground Covers (all nodes)

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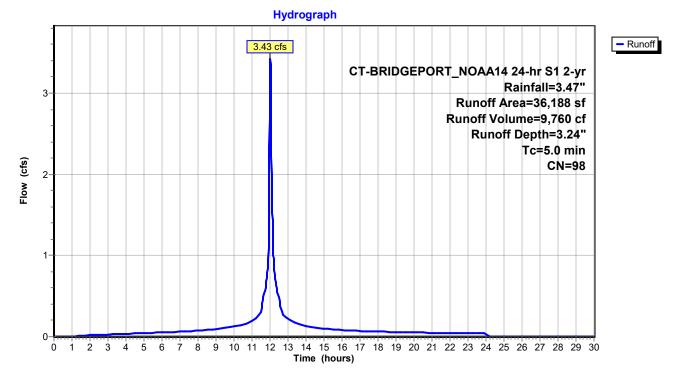
### Summary for Subcatchment PDA-1A: PDA-1A

Runoff 3.43 cfs @ 12.03 hrs, Volume= 9,760 cf, Depth= 3.24" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT NOAA14 24-hr S1 2-yr Rainfall=3.47"

	Area (sf)	CN	Description				
	3,905	98	Paved park	ing, HSG D			
	25,490	98	Unconnecte	d roofs, H	SG D		
*	6,793	98	Courtyard				
	36,188	98	Weighted A	verage			
	36,188		100.00% Im	pervious A	Area		
	25,490		70.44% Un	connected			
-		~		<b>o</b>			
To	0	Slop	,	Capacity	Description		
(min)	(feet)	(ft/ft	i) (ft/sec)	(cfs)			
5.0	)				Direct Entry,		

#### Subcatchment PDA-1A: PDA-1A



## Summary for Subcatchment PDA-1B: PDA-1B

Runoff 0.32 cfs @ 12.09 hrs, Volume= 1,090 cf, Depth= 1.61" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 2-yr Rainfall=3.47"

A	rea (sf)	CN D	escription				
	8,063		80 >75% Grass cover, Good, HSG D				
	49	98 U	Inconnecte	ed roofs, HS	SG D		
	8,112	80 V	Veighted A	verage			
	8,063	9	9.40% Per	vious Area			
	49	0	.60% Impe	ervious Area	а		
	49	1	00.00% Ur	nconnected			
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
0.4	5	0.1110	0.19		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.47"		
1.0	12	0.0830	0.20		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.47"		
3.9	48	0.0416	0.21		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.47"		
3.6	36	0.0277	0.16		Sheet Flow,		
					Grass: Short n= 0.150 P2= 3.47"		
1.1	60	0.0166	0.90		Shallow Concentrated Flow,		
					Short Grass Pasture Kv= 7.0 fps		
10.0	161	Total					

#### C-DAT-2102357-PR HYDRO Prepared by BL Companies

Hydrograph 0.36-- Runoff 0.34 0.32 cfs 0.32 CT-BRIDGEPORT\_NOAA14 24-hr S1 2-yr 0.3 Rainfall=3.47" 0.28 Runoff Area=8,112 sf 0.26-Runoff Volume=1,090 cf 0.24 Runoff Depth=1.61" 0.22-Flow Length=161' Flow (cfs) 0.2 Tc=10.0 min CN=80 0.18 0.16 0.14 0.12 0.1 0.08 0.06-0.04 0.02 0-1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 Ó

Time (hours)

## Subcatchment PDA-1B: PDA-1B

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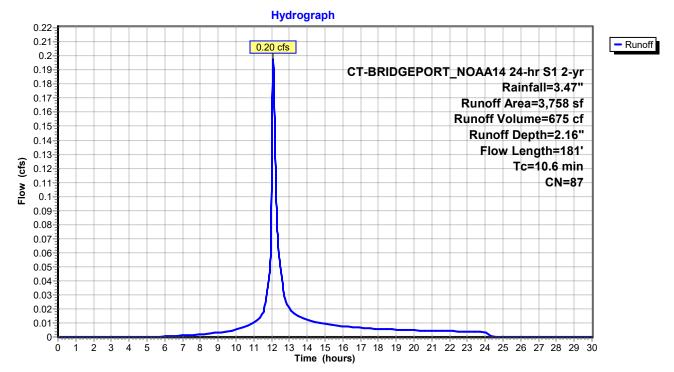
### Summary for Subcatchment PDA-2: PDA-2

0.20 cfs @ 12.09 hrs, Volume= Runoff = 675 cf, Depth= 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT NOAA14 24-hr S1 2-yr Rainfall=3.47"

_	A	rea (sf)	CN [	Description					
		1,492	98 F	Paved parking, HSG D					
		2,266	80 >	•75% Ġras	s cover, Go	bod, HSG D			
		3,758	87 V	Veighted A	verage				
		2,266	6	60.30% Per	vious Area				
		1,492	3	89.70% Imp	ervious Ar	ea			
	Тс	Length	Slope		Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	9.4	100	0.0200	0.18		Sheet Flow,			
						Grass: Short n= 0.150 P2= 3.47"			
	1.1	63	0.0200	0.99		Shallow Concentrated Flow,			
						Short Grass Pasture Kv= 7.0 fps			
	0.1	18	0.0100	2.03		Shallow Concentrated Flow,			
						Paved Kv= 20.3 fps			
	10.6	181	Total						

## Subcatchment PDA-2: PDA-2



### Summary for Pond STMW-1: SUBSURFACE 1

Inflow Area =	36,188 sf,100.00% Impervious,	Inflow Depth = 3.24" for 2-yr event
Inflow =	3.43 cfs @ 12.03 hrs, Volume=	9,760 cf
Outflow =	1.11 cfs @ 12.18 hrs, Volume=	5,923 cf, Atten= 68%, Lag= 9.0 min
Discarded =	0.01 cfs @ 12.18 hrs, Volume=	790 cf
Primary =	1.10 cfs $\overline{@}$ 12.18 hrs, Volume=	5,133 cf

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 38.79' @ 12.18 hrs Surf.Area= 2,043 sf Storage= 4,908 cf

Plug-Flow detention time= 297.0 min calculated for 5,921 cf (61% of inflow) Center-of-Mass det. time= 164.4 min (920.2 - 755.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	36.00'	0 cf	6.90'W x 296.31'L x 4.67'H Field A
			9,536 cf Overall - 9,536 cf Embedded = 0 cf x 40.0% Voids
#2A	36.00'	7,025 cf	StormTrap ST1 SingleTrap 4-0 x 21 Inside #1
			Inside= 82.7"W x 48.0"H => 23.79 sf x 14.06'L = 334.5 cf
			Outside= 82.7"W x 56.0"H => 32.18 sf x 14.06'L = 452.5 cf
			6.90' x 295.31' Core + 0.00' x 0.50' Border = 6.90' x 296.31' System
		7 025 cf	Total Available Storage

7,025 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	36.00'	0.090 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.25'	12.0" Round Culvert
			L= 8.0' RCP, end-section conforming to fill, Ke= 0.500
			Inlet / Outlet Invert= 38.25' / 36.00' S= 0.2813 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

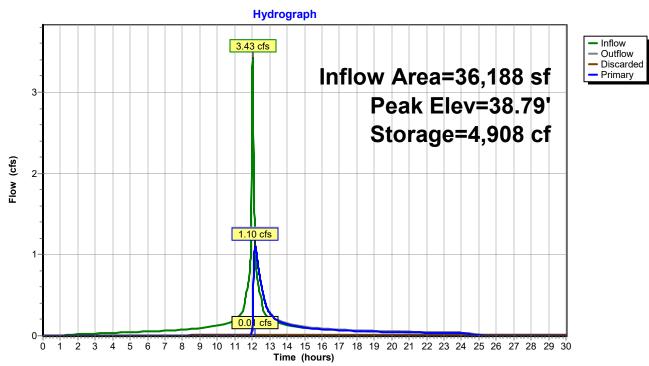
**Discarded OutFlow** Max=0.01 cfs @ 12.18 hrs HW=38.79' (Free Discharge) **1=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=1.10 cfs @ 12.18 hrs HW=38.79' (Free Discharge) ←2=Culvert (Inlet Controls 1.10 cfs @ 2.51 fps)

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Page 11

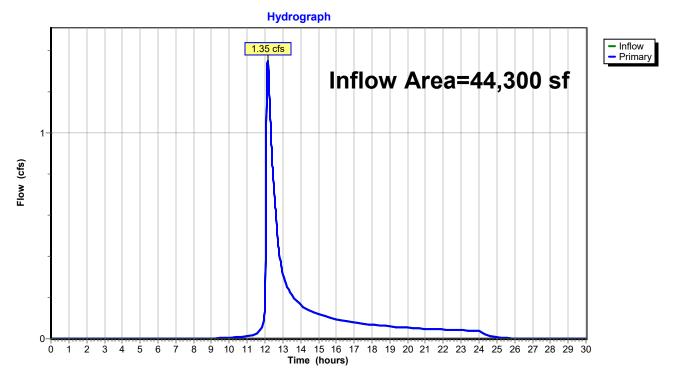


## Pond STMW-1: SUBSURFACE 1

## Summary for Link DP-1: DP-1

Inflow Area	a =	44,300 sf, 8	1.80% Impervious,	Inflow Depth = 1.69"	for 2-yr event
Inflow	=	1.35 cfs @ 12	2.16 hrs, Volume=	6,223 cf	
Primary	=	1.35 cfs @ 12	2.16 hrs, Volume=	6,223 cf, Atte	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

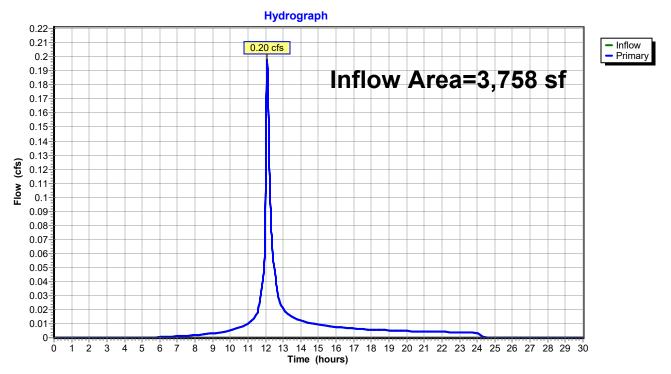


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	3,758 sf, 39.70% Impervious, Inflow Depth = 2.16" for 2-yr event	
Inflow	=	0.20 cfs @ 12.09 hrs, Volume= 675 cf	
Primary	=	0.20 cfs @ 12.09 hrs, Volume= 675 cf, Atten= 0%, Lag= 0.0 min	I

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



#### Link DP-2: DP-2

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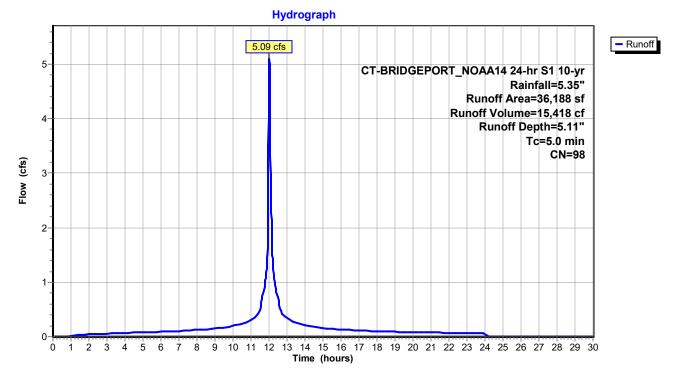
## Summary for Subcatchment PDA-1A: PDA-1A

Runoff 5.09 cfs @ 12.03 hrs, Volume= 15,418 cf, Depth= 5.11" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT NOAA14 24-hr S1 10-yr Rainfall=5.35"

	Area (sf)	CN	Description					
	3,905	98	Paved park	ing, HSG D	D			
	25,490	98	Unconnecte	ed roofs, H	ISG D			
*	6,793	98	Courtyard					
	36,188	98	Weighted Average					
	36,188		100.00% Impervious Area					
	25,490		70.44% Unconnected					
_								
To	5	Slope	,	Capacity	I I I I I I I I I I I I I I I I I I I			
(min)	) (feet)	(ft/ft	) (ft/sec)	(cfs)				
5.0	)				Direct Entry,			

#### Subcatchment PDA-1A: PDA-1A



## Summary for Subcatchment PDA-1B: PDA-1B

Runoff 0.63 cfs @ 12.09 hrs, Volume= 2,163 cf, Depth= 3.20" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 10-yr Rainfall=5.35"

A	rea (sf)	CN D	escription					
	8,063	80 >	80 >75% Grass cover, Good, HSG D					
	49	98 U	Inconnecte	ed roofs, HS	SG D			
	8,112	80 V	Veighted A	verage				
	8,063	9	9.40% Per	vious Area				
	49			ervious Area				
	49	1	00.00% Ui	nconnected				
-		01		0				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
0.4	5	0.1110	0.19		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.47"			
1.0	12	0.0830	0.20		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.47"			
3.9	48	0.0416	0.21		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.47"			
3.6	36	0.0277	0.16		Sheet Flow,			
					Grass: Short n= 0.150 P2= 3.47"			
1.1	60	0.0166	0.90		Shallow Concentrated Flow,			
					Short Grass Pasture Kv= 7.0 fps			
10.0	161	Total						

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Hydrograph 0.7 - Runoff 0.63 cfs 0.65 0.6 CT-BRIDGEPORT\_NOAA14 24-hr S1 10-yr Rainfall=5.35" 0.55-Runoff Area=8,112 sf Runoff Volume=2,163 cf 0.5 Runoff Depth=3.20" 0.45-Flow Length=161' Tc=10.0 min 0.4 Flow (cfs) CN=80 0.35 0.3 0.25 0.2 0.15 0.1 0.05 0-1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 Ó Time (hours)

## Subcatchment PDA-1B: PDA-1B

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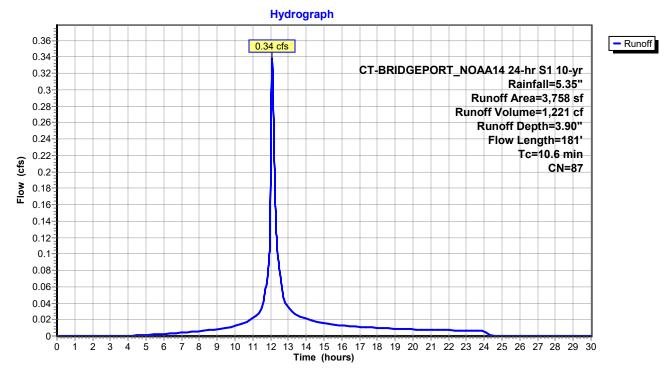
## Summary for Subcatchment PDA-2: PDA-2

0.34 cfs @ 12.09 hrs, Volume= Runoff = 1,221 cf, Depth= 3.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT NOAA14 24-hr S1 10-yr Rainfall=5.35"

_	A	rea (sf)	CN [	Description		
	1,492 98 Paved parking, HSG D				ing, HSG D	
		2,266	80 >	•75% Ġras	s cover, Go	bod, HSG D
		3,758	87 V	Veighted A	verage	
		2,266	6	60.30% Per	vious Area	
		1,492	3	89.70% Imp	ervious Ar	ea
	Тс	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.4	100	0.0200	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.47"
	1.1	63	0.0200	0.99		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	18	0.0100	2.03		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	10.6	181	Total			

## Subcatchment PDA-2: PDA-2



### Summary for Pond STMW-1: SUBSURFACE 1

Inflow Area =	36,188 sf,100.00% Impervious,	Inflow Depth = 5.11" for 10-yr event
Inflow =	5.09 cfs @ 12.03 hrs, Volume=	15,418 cf
Outflow =	2.95 cfs @ 12.09 hrs, Volume=	11,573 cf, Atten= 42%, Lag= 4.0 min
Discarded =	0.01 cfs @ 12.09 hrs, Volume=	840 cf
Primary =	2.94 cfs $\overline{@}$ 12.09 hrs, Volume=	10,733 cf

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 39.35' @ 12.09 hrs Surf.Area= 2,043 sf Storage= 5,888 cf

Plug-Flow detention time= 234.7 min calculated for 11,573 cf (75% of inflow) Center-of-Mass det. time= 126.2 min (873.2 - 747.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	36.00'	0 cf	6.90'W x 296.31'L x 4.67'H Field A
			9,536 cf Overall - 9,536 cf Embedded = 0 cf x 40.0% Voids
#2A	36.00'	7,025 cf	StormTrap ST1 SingleTrap 4-0 x 21 Inside #1
			Inside= 82.7"W x 48.0"H => 23.79 sf x 14.06'L = 334.5 cf
			Outside= 82.7"W x 56.0"H => 32.18 sf x 14.06'L = 452.5 cf
			6.90' x 295.31' Core + 0.00' x 0.50' Border = 6.90' x 296.31' System
		7 025 cf	Total Available Storage

7,025 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	36.00'	0.090 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.25'	12.0" Round Culvert
			L= 8.0' RCP, end-section conforming to fill, Ke= 0.500
			Inlet / Outlet Invert= 38.25' / 36.00' S= 0.2813 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

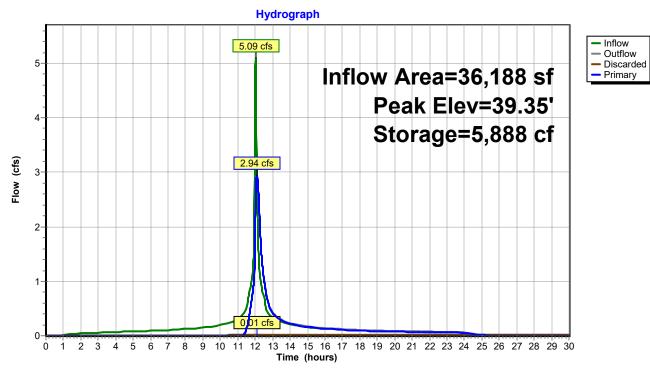
**Discarded OutFlow** Max=0.01 cfs @ 12.09 hrs HW=39.35' (Free Discharge) **1=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=2.94 cfs @ 12.09 hrs HW=39.35' (Free Discharge) ←2=Culvert (Inlet Controls 2.94 cfs @ 3.74 fps)

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Page 19

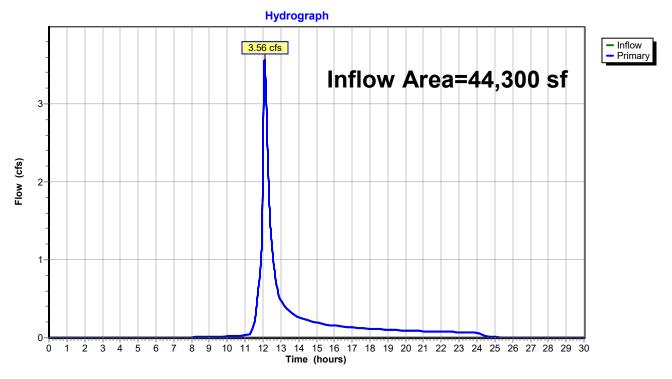


## Pond STMW-1: SUBSURFACE 1

## Summary for Link DP-1: DP-1

Inflow Area	a =	44,300 sf, 81.80% Impervious, Inflow Depth = 3.49" for 10-yr event	
Inflow	=	3.56 cfs @ 12.09 hrs, Volume= 12,896 cf	
Primary	=	3.56 cfs @ 12.09 hrs, Volume= 12,896 cf, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

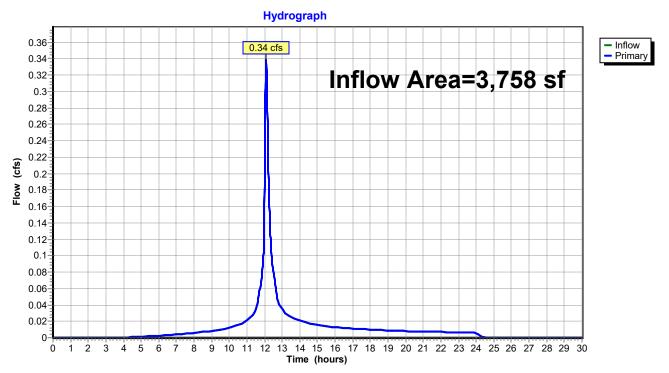


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	3,758 sf, 39.70% Impervious, Infle	ow Depth = 3.90" for 10-yr event
Inflow	=	0.34 cfs @ 12.09 hrs, Volume=	1,221 cf
Primary	=	0.34 cfs @ 12.09 hrs, Volume=	1,221 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



## Link DP-2: DP-2

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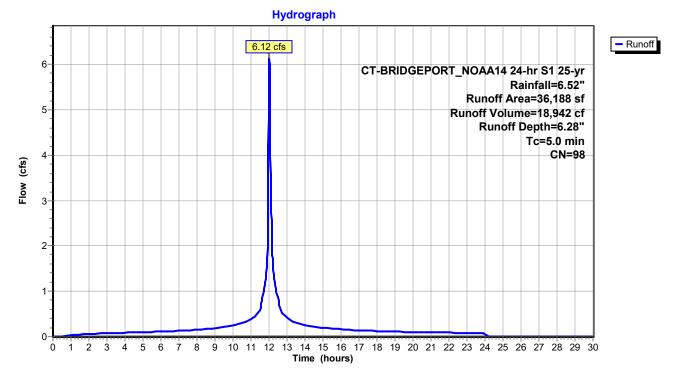
## Summary for Subcatchment PDA-1A: PDA-1A

Runoff 6.12 cfs @ 12.03 hrs, Volume= 18,942 cf, Depth= 6.28" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT NOAA14 24-hr S1 25-yr Rainfall=6.52"

	Area (sf)	CN	Description				
	3,905	98	Paved park	ing, HSG D	D		
	25,490	98	Unconnected roofs, HSG D				
*	6,793	98	Courtyard				
	36,188	98	Weighted A	verage			
	36,188		100.00% In	npervious A	Area		
	25,490		70.44% Un	connected			
-				0			
	c Length	Slope		Capacity	1		
(mir	n) (feet)	(ft/ft	) (ft/sec)	(cfs)			
5.	0				Direct Entry,		

## Subcatchment PDA-1A: PDA-1A



## Summary for Subcatchment PDA-1B: PDA-1B

Runoff 0.82 cfs @ 12.08 hrs, Volume= 2,875 cf, Depth= 4.25" =

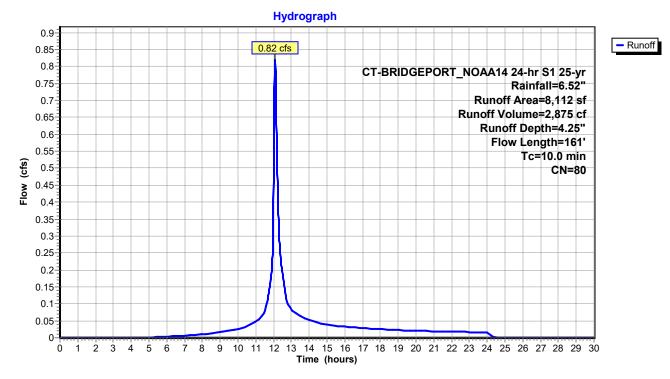
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 25-yr Rainfall=6.52"

A	rea (sf)	CN D	escription			
	8,063	80 >75% Grass cover, Good, HSG D				
	49	98 U	Inconnecte	ed roofs, HS	SG D	
	8,112	80 V	Veighted A	verage		
	8,063	9	9.40% Per	vious Area		
	49	0	.60% Impe	ervious Area	а	
	49	1	00.00% Ür	nconnected		
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
0.4	5	0.1110	0.19		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
1.0	12	0.0830	0.20		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
3.9	48	0.0416	0.21		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
3.6	36	0.0277	0.16		Sheet Flow,	
					Grass: Short n= 0.150 P2= 3.47"	
1.1	60	0.0166	0.90		Shallow Concentrated Flow,	
					Short Grass Pasture Kv= 7.0 fps	
10.0	161	Total				

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## Subcatchment PDA-1B: PDA-1B

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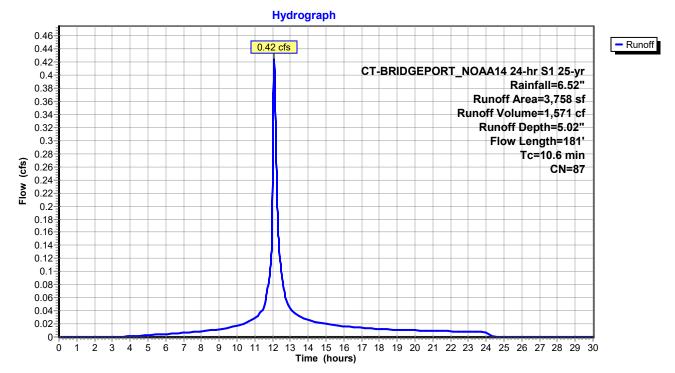
### Summary for Subcatchment PDA-2: PDA-2

0.42 cfs @ 12.09 hrs, Volume= Runoff = 1,571 cf, Depth= 5.02"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 25-yr Rainfall=6.52"

_	A	rea (sf)	CN [	Description		
	1,492 98 Paved parking, HSG D				ing, HSG D	
		2,266	80 >	•75% Ġras	s cover, Go	bod, HSG D
		3,758	87 V	Veighted A	verage	
		2,266	6	60.30% Per	vious Area	
		1,492	3	89.70% Imp	ervious Ar	ea
	Тс	Length	Slope		Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.4	100	0.0200	0.18		Sheet Flow,
						Grass: Short n= 0.150 P2= 3.47"
	1.1	63	0.0200	0.99		Shallow Concentrated Flow,
						Short Grass Pasture Kv= 7.0 fps
	0.1	18	0.0100	2.03		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	10.6	181	Total			

## Subcatchment PDA-2: PDA-2



### Summary for Pond STMW-1: SUBSURFACE 1

Inflow Area =	36,188 sf,100.00% Impervious,	Inflow Depth = 6.28" for 25-yr event
Inflow =	6.12 cfs @ 12.03 hrs, Volume=	18,942 cf
Outflow =	3.41 cfs @ 12.10 hrs, Volume=	15,093 cf, Atten= 44%, Lag= 4.2 min
Discarded =	0.01 cfs @ 12.10 hrs, Volume=	864 cf
Primary =	3.40 cfs $\overline{@}$ 12.10 hrs, Volume=	14,229 cf

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 39.56' @ 12.10 hrs Surf.Area= 2,043 sf Storage= 6,252 cf

Plug-Flow detention time= 211.8 min calculated for 15,093 cf (80% of inflow) Center-of-Mass det. time= 114.8 min (858.5 - 743.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	36.00'	0 cf	6.90'W x 296.31'L x 4.67'H Field A
			9,536 cf Overall - 9,536 cf Embedded = 0 cf x 40.0% Voids
#2A	36.00'	7,025 cf	StormTrap ST1 SingleTrap 4-0 x 21 Inside #1
			Inside= 82.7"W x 48.0"H => 23.79 sf x 14.06'L = 334.5 cf
			Outside= 82.7"W x 56.0"H => 32.18 sf x 14.06'L = 452.5 cf
			6.90' x 295.31' Core + 0.00' x 0.50' Border = 6.90' x 296.31' System
		7 025 cf	Total Available Storage

7,025 cf I otal Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	36.00'	0.090 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.25'	12.0" Round Culvert
			L= 8.0' RCP, end-section conforming to fill, Ke= 0.500
			Inlet / Outlet Invert= 38.25' / 36.00' S= 0.2813 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

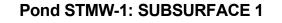
**Discarded OutFlow** Max=0.01 cfs @ 12.10 hrs HW=39.56' (Free Discharge) **1=Exfiltration** (Controls 0.01 cfs)

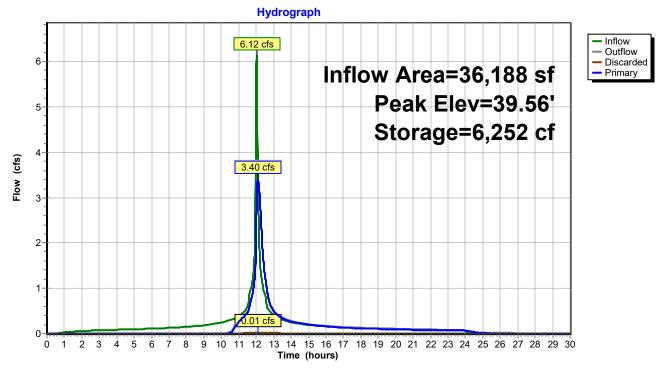
Primary OutFlow Max=3.40 cfs @ 12.10 hrs HW=39.56' (Free Discharge) ←2=Culvert (Inlet Controls 3.40 cfs @ 4.33 fps)

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Page 27

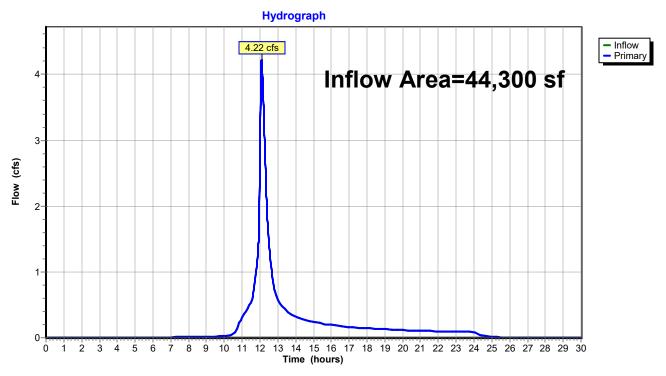




## Summary for Link DP-1: DP-1

Inflow Area	a =	44,300 sf, 81.80% Impervious, Inflow Depth = 4.63" for 25-yr event	
Inflow	=	4.22 cfs @ 12.09 hrs, Volume= 17,104 cf	
Primary	=	4.22 cfs @ 12.09 hrs, Volume= 17,104 cf, Atten= 0%, Lag= 0.0 min	

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

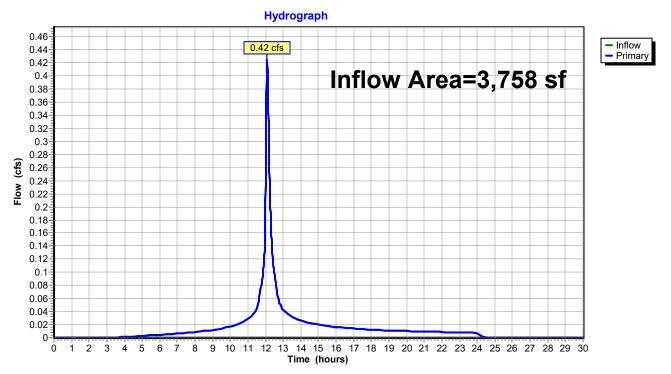


Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	3,758 sf,	39.70% Impervious	, Inflow Depth =	5.02" for 25-yr event
Inflow	=	0.42 cfs @ 1	12.09 hrs, Volume=	1,571 cf	
Primary	=	0.42 cfs @ 1	12.09 hrs, Volume=	1,571 cf	, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



## Link DP-2: DP-2

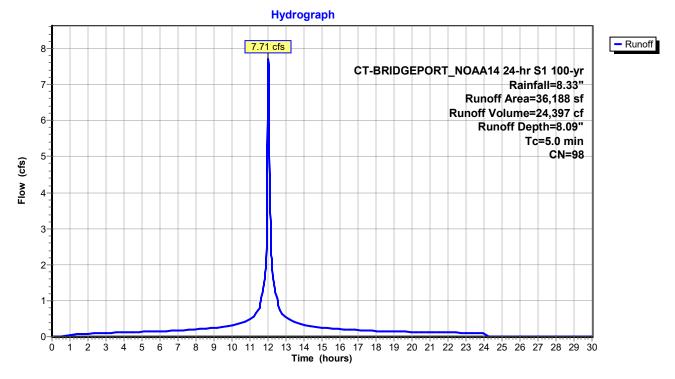
## Summary for Subcatchment PDA-1A: PDA-1A

Runoff = 7.71 cfs @ 12.03 hrs, Volume= 24,397 cf, Depth= 8.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 100-yr Rainfall=8.33"

	Area (sf)	CN	Description		
	3,905	98	Paved park	ing, HSG D	D
	25,490	98	Unconnecte	ed roofs, H	ISG D
*	6,793	98	Courtyard		
	36,188	98	Weighted A	verage	
	36,188		100.00% In	npervious A	Area
	25,490		70.44% Un	connected	l
	Tc Length	Slop	e Velocity	Capacity	Description
(m	•			(cfs)	I
	in) (feet)	(11/1	(it/sec)	(CIS)	
!	5.0				Direct Entry,

## Subcatchment PDA-1A: PDA-1A



## Summary for Subcatchment PDA-1B: PDA-1B

Runoff = 1.12 cfs @ 12.08 hrs, Volume= 4,012 cf, Depth= 5.94"

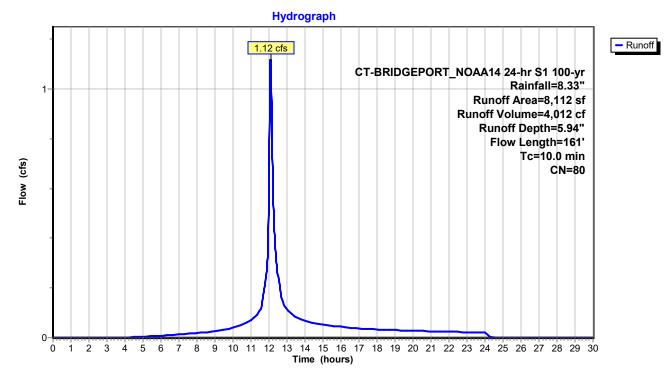
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 100-yr Rainfall=8.33"

A	rea (sf)	CN D	escription								
	8,063	80 >	80 >75% Grass cover, Good, HSG D								
	49	98 U	Inconnecte	ed roofs, HS	SG D						
	8,112	80 V	Veighted A	verage							
	8,063	9	9.40% Per	vious Area							
	49			ervious Area							
	49	1	00.00% Ui	nconnected							
_					- · · ·						
Tc	Length	Slope	Velocity	Capacity	Description						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
0.4	5	0.1110	0.19		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
1.0	12	0.0830	0.20		Sheet Flow,						
					Grass: Short n= 0.150 P2= 3.47"						
3.9	48	0.0416	0.21		Sheet Flow,						
			0.40		Grass: Short n= 0.150 P2= 3.47"						
3.6	36	0.0277	0.16		Sheet Flow,						
	00	0.0400	0.00		Grass: Short n= 0.150 P2= 3.47"						
1.1	60	0.0166	0.90		Shallow Concentrated Flow,						
					Short Grass Pasture Kv= 7.0 fps						
10.0	161	Total									

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Page 32



## Subcatchment PDA-1B: PDA-1B

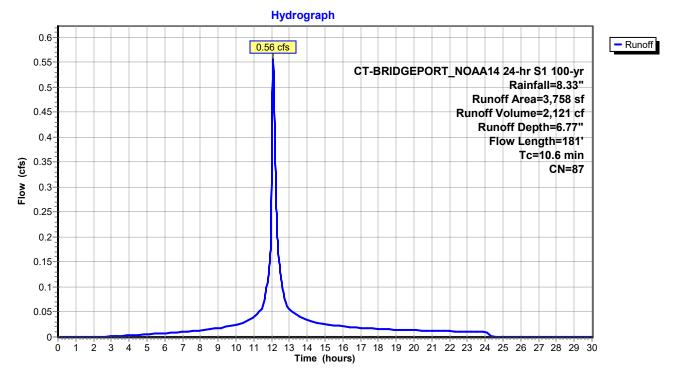
### Summary for Subcatchment PDA-2: PDA-2

Runoff = 0.56 cfs @ 12.09 hrs, Volume= 2,121 cf, Depth= 6.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs CT-BRIDGEPORT\_NOAA14 24-hr S1 100-yr Rainfall=8.33"

_	A	rea (sf)	CN [	Description								
		1,492	98 F	Paved parking, HSG D								
		2,266	80 >	•75% Ġras	s cover, Go	ood, HSG D						
		3,758	87 V	Veighted A	verage							
		2,266	6	60.30% Per	vious Area							
		1,492	3	39.70% Imp	pervious Ar	ea						
	Тс	Length	Slope		Capacity	Description						
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)							
	9.4	100	0.0200	0.18		Sheet Flow,						
						Grass: Short n= 0.150 P2= 3.47"						
	1.1	63	0.0200	0.99		Shallow Concentrated Flow,						
						Short Grass Pasture Kv= 7.0 fps						
	0.1	18	0.0100	2.03		Shallow Concentrated Flow,						
_						Paved Kv= 20.3 fps						
	10.6	181	Total									

#### Subcatchment PDA-2: PDA-2



#### Summary for Pond STMW-1: SUBSURFACE 1

Inflow Area =	36,188 sf,100.00% Impervious,	Inflow Depth = 8.09" for 100-yr event
Inflow =	7.71 cfs @ 12.03 hrs, Volume=	24,397 cf
Outflow =	4.05 cfs @ 12.10 hrs, Volume=	20,544 cf, Atten= 47%, Lag= 4.6 min
Discarded =	0.01 cfs @ 12.10 hrs, Volume=	895 cf
Primary =	4.04 cfs $\overline{@}$ 12.10 hrs, Volume=	19,650 cf

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 39.89' @ 12.10 hrs Surf.Area= 2,043 sf Storage= 6,834 cf

Plug-Flow detention time= 184.1 min calculated for 20,544 cf (84% of inflow) Center-of-Mass det. time= 101.4 min (841.4 - 740.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	36.00'	0 cf	6.90'W x 296.31'L x 4.67'H Field A
			9,536 cf Overall - 9,536 cf Embedded = 0 cf x 40.0% Voids
#2A	36.00'	7,025 cf	StormTrap ST1 SingleTrap 4-0 x 21 Inside #1
			Inside= 82.7"W x 48.0"H => 23.79 sf x 14.06'L = 334.5 cf
			Outside= 82.7"W x 56.0"H => 32.18 sf x 14.06'L = 452.5 cf
			6.90' x 295.31' Core + 0.00' x 0.50' Border = 6.90' x 296.31' System
		7 025 cf	Total Available Storage

7,025 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	36.00'	0.090 in/hr Exfiltration over Surface area
			Conductivity to Groundwater Elevation = 34.00'
#2	Primary	38.25'	12.0" Round Culvert
			L= 8.0' RCP, end-section conforming to fill, Ke= 0.500
			Inlet / Outlet Invert= 38.25' / 36.00' S= 0.2813 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.79 sf

**Discarded OutFlow** Max=0.01 cfs @ 12.10 hrs HW=39.89' (Free Discharge) **1=Exfiltration** (Controls 0.01 cfs)

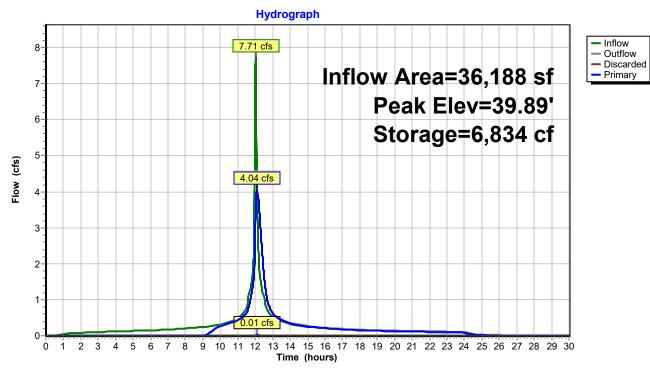
Primary OutFlow Max=4.04 cfs @ 12.10 hrs HW=39.89' (Free Discharge) ←2=Culvert (Inlet Controls 4.04 cfs @ 5.14 fps)

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Page 35

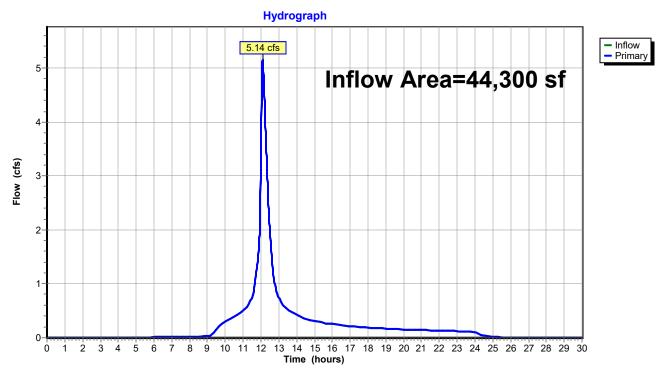


Pond STMW-1: SUBSURFACE 1

## Summary for Link DP-1: DP-1

Inflow Area	a =	44,300 sf, 81.80% Impervious, Inflow Depth = 6.41" for 100-yr event
Inflow	=	5.14 cfs @ 12.09 hrs, Volume= 23,662 cf
Primary	=	5.14 cfs @ 12.09 hrs, Volume= 23,662 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

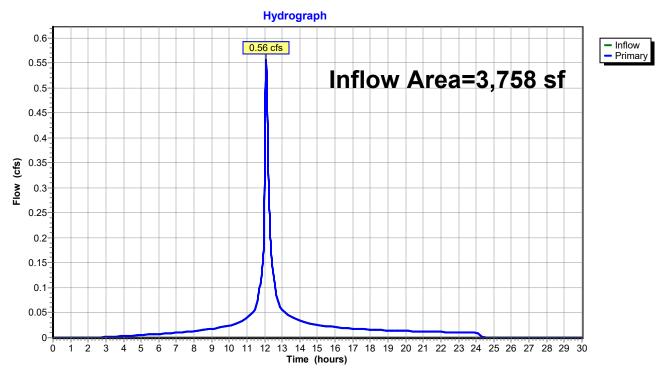


#### Link DP-1: DP-1

## Summary for Link DP-2: DP-2

Inflow Area	a =	3,758 sf	, 39.70% Impervious	Inflow Depth = 6.77'	' for 100-yr event
Inflow	=	0.56 cfs @	12.09 hrs, Volume=	2,121 cf	
Primary	=	0.56 cfs @	12.09 hrs, Volume=	2,121 cf, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs



## Link DP-2: DP-2



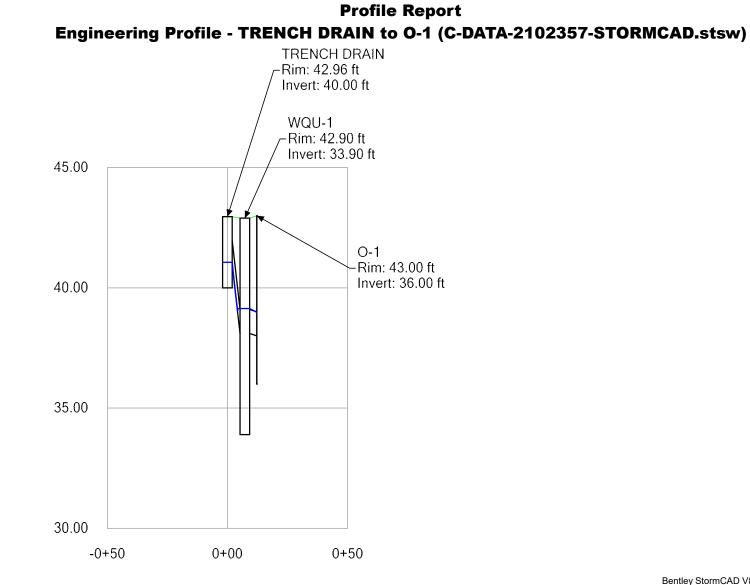
# APPENDIX D

Collection and Conveyance Calculations

## FlexTable: Conduit Table

Start Node	Stop Node	Invert (Start) (ft)	Invert (Stop) (ft)	Cover (Start) (ft)	Cover (Stop) (ft)	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Diameter (in)	Material	Manning's n	Velocity (ft/s)
TRENCH DRAIN	WQU-1	40.96	38.10	1.00	3.80	7.2	0.395	12.0	Concrete	0.013	6.32
CB-1	WQU-1	38.50	38.10	3.00	3.80	16.0	0.025	12.0	Concrete	0.013	7.98
WQU-1	O-1	38.10	38.00	3.80	4.00	5.0	0.020	12.0	Concrete	0.013	8.05

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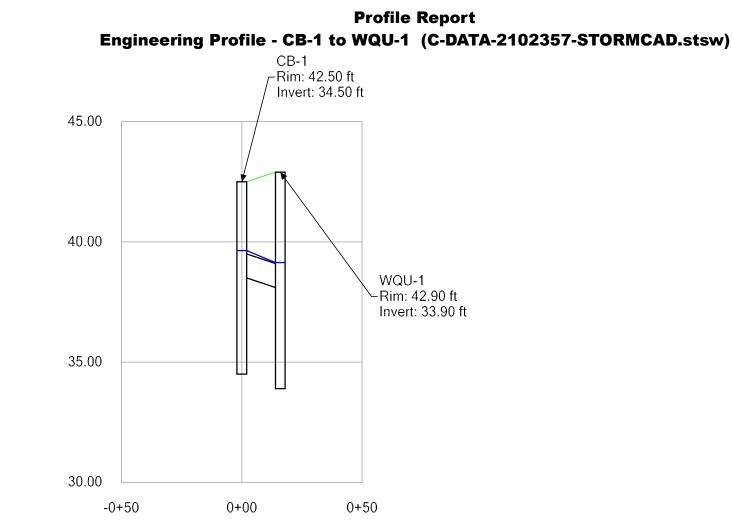


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Elevation (ft)

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Appendix E

Water Quality Calculations Best Management Practice (BMP) Treatment Train Efficiency Worksheet Water Quality Calculations – Water Quality Volume (WQV) Water Quality Calculations – Water Quality Flowrate (WQF) Water Quality Calculations – Infiltration Volume

Best Managem	ent Practice (BMP) Treatment Train Efficie	ncy Worksheet							
Prepared for: Proposed Residential Development 543, 547, 549, 557 Ellsworth Street Bridgeport, Connecticut									
Prepared by: BL Companies 100 Constitution Plaza, 10th Floor Hartford Connecticut									
Date prepared: December 15, 2021									
Et=[1-(1-E1)(1-E2)(1-E3)(1-E4)(1-E?)]*100	BMP         BMP Description           E1         Hydrodynamic Separator (CDS unit)** Subsurface Infiltration Chambers*	<u>Type of Treatment</u> primary Secondary	Efficiency Rate % 80 80	<u>BMP</u> Hydrodynamic Separator (CDS unit)** Subsurface Infiltration Chambers*	<u>Type pf Treatment</u> primary Secondary	<u>TSS Removal</u> <u>Rate</u> 0.80 0.80	<u>Starting TSS</u> <u>Load</u> 1.00 0.20	<u>Amount</u> <u>Removed</u> 0.80 0.16	<u>Remaining</u> <u>Load</u> 0.20 0.04
Overall Treatment Train Efficiency (Et)=	96 % Total Suspended Solids (TSS) Removal			Overall Treatment Train Efficiency (%)					96
* 80% require per CT DEP ** Manufacter's specifications									

#### TSS Removal Rates (adapted from Schueler, 1996, & EPA, 1993)

BMP List	Design		Brief Design Requirements
	Rate	Average TSS	
		Removal Rates	
Extended Detention Pond	70%	60-80%	Sediment forebay
Wet Pond (a)	70%	60-80%	Sediment forebay
Constructed Wetland (b)	80%	65-80%	Designed to infiltrate or retain
Water Quality Swale	70%	60-80%	Designed to infiltrate or retain
Infiltration Trench	80%	75-80%	Pretreatment critical
Infiltration Basin	80%	75-80%	Pretreatment critical
		(predicted)	
Dry Well	80%	80% (predicted)	Rooftop runoff
			(uncontaminated only)
Sand Filter (c)	80%	80%	Pretreatment
Organic Filter (d)	80%	80%+	Pretreatment
Water Quality Inlet	25%	15-35% w/	Off-line only; 0.1" minimum Water Quality Volume (WQV) storage
		cleanout	
Sediment Trap (Forebay)	25%	2.5% w/	Storm flows for 2-year event must not cause erosion; 0.1" minimum WQV storage
Sediment Trap (Forebay)	2370	cleanout	Storin nows for 2-year event must not eause crosion, 0.1 minimum w Q v storage
Drainage Channel	25%	25%	Check dams; non-erosive for 2-yr.
Deep Sump and Hooded Catch Basin	25%	25% w/ cleanout	Deep sump general rule = 4 x pipe diameter or 4.0' for pipes 18" or less
Street Sweeping	10%	10%	Discretionary non-structural credit, must be part of approved plan

#### Water Quality Calculations

#### **Determine Water Quality Volume**

From CT 2004 Stormwater Quality Manual:

$$WQV = \frac{(1'')(R)(A)}{12}$$

R = 0.05 + 0.009(I)

WQV = water quality volume (ac-ft) R = volumetric runoff coefficient

- I = percent impervious cover
- A = site area in acres

Area	Total	Area	Impervio	ous Area	Impervious Cover	Volumetric Runoff Coefficient		lity Volume QV)	Water Quality Volume Provided
ID	ac	ft <sup>2</sup>	ac	ft <sup>2</sup>	%	R	acre-feet	ft <sup>3</sup>	ft <sup>3</sup>
SITE	1.060	46,195	0.866	37,729	81.70	0.785	0.069	3,006	3,951

#### Water Quality Calculations

#### **Determine Water Quality Flow**

From CT 2004 Stormwater Quality Manual:

$$CN = \frac{1000}{\left[10 + 5P + 10Q - 10(Q^2 + 1.25QP)^{\frac{1}{2}}\right]}$$

$$Q = \frac{\left[WQV(acre - feet) \times \left[12(inches / foot)\right]\right]}{DrainageArea(acres)}$$

$$WQF = (q_u)(A)(Q)$$

CN = Runoff Curve Number

P = design preciptation, inches, (1" for water quality storm)

Q = runoff depth (in watershed inches)

 $T_c$  = time of concentration

 $I_a$  = Initial abstraction, inches, from Table 4-1, Chapter 4, TR-55

q<sub>u</sub> = unit peak discharge,

WQF = water quality flow (cfs)

Hydrodynamic	Т	otal Area		Imp A	rea	Imp Cover	R	WQV	Q	Р	CN		T <sub>c</sub>	l <sub>a</sub>	I <sub>a</sub> /P	q <sub>u</sub> *	WQF
Separator	ft <sup>2</sup>	ac	mi <sup>2</sup>	ft <sup>2</sup>	ac	%	-	acre-feet	in	in	-	mins	hours	in	-	cfs/mi²/in	cfs
CDS unit	46,195	1.060	0.0017	37,729	0.866	81.70	0.785	0.069	0.78	1.00	98	5.0	0.08	0.041	0.041	650	0.84

#### **Infiltration Volume Calculations**

	A					
	Total Site Area (AC)	Impervious Area	Infiltration	Required	Proposed Volu	Infiltration ume
	(AC)	s.f.	(ac-ft)	(cu ft)	(ac-ft)	(cu ft)
SITE	1.06	37,729	0.072	3,144	0.091	3,951

HydroCAD® 10.00-26 s/n 01334 © 2020 HydroCAD Software Solutions LLC

### Stage-Area-Storage for Pond STMW-1: SUBSURFACE 1

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
36.00	2,043	0	38.60	2,043	4,566
36.05	2,043	88	38.65	2,043	4,654
36.10					
	2,043	176	38.70	2,043	4,742
36.15	2,043	263	38.75	2,043	4,829
36.20	2,043	351	38.80	2,043	4,917
36.25	2,043	439	38.85	2,043	5,005
36.30	2,043	527	38.90	2,043	5,093
36.35	2,043	615	38.95	2,043	5,181
36.40	2,043	702	39.00	2,043	5,268
36.45	2,043	790	39.05	2,043	5,356
36.50	2,043	878	39.10	2,043	5,444
36.55	2,043	966	39.15	2,043	5,532
36.60	2,043	1,054	39.20	2,043	5,620
36.65	2,043	1,141	39.25	2,043	5,707
36.70	2,043	1,229	39.30	2,043	5,795
36.75	2,043	1,317	39.35	2,043	5,883
36.80	2,043	1,405	39.40	2,043	5,971
36.85	2,043	1,493	39.45	2,043	6,059
36.90	2,043	1,581	39.50	2,043	6,146
36.95	2,043	1,668	39.55	2,043	6,234
37.00	2,043	1,756	39.60	2,043	6,322
37.05	2,043	1,844	39.65	2,043	6,410
37.10	2,043	1,932	39.70	2,043	6,498
37.15	2,043	2,020	39.75	2,043	6,585
37.20	2,043	2,107	39.80	2,043	6,673
37.25	2,043	2,195	39.85	2,043	6,761
37.30	2,043	2,283	39.90	2,043	6,849
37.35	2,043	2,371	39.95	2,043	6,937
37.40	2,043	2,459	40.00	2,043	7,025
37.45	2,043	2,546	40.05	2,043	7,025
37.50	2,043	2,634	40.10	2,043	7,025
37.55	2,043	2,722	40.15	2,043	7,025
37.60	2,043	2,810	40.20	2,043	7,025
37.65	2,043	2,898	40.25	2,043	7,025
37.70	2,043	2,985	40.30	2,043	7,025
37.75	2,043	3,073	40.35	2,043	7,025
37.80	2,043	3,161	40.40	2,043	7,025
37.85	2,043	3,249	40.45	2,043	7,025
37.90	2,043	3,337	40.50	2,043	7,025
37.95	2,043	3,424	40.55	2,043	7,025
38.00	2,043	3,512	40.60	2,043	7,025
38.05	2,043	3,600	40.65	2,043	7,025
			40.05	2,043	7,025
38.10	2,043	3,688			
38.15	2,043	3,776			
38.20	2,043	3,863			
38.25	2,043	<mark>3,951</mark> 🔊			
38.30	2,043	4,039 🖡			
38.35	2,043	4,127	infiltrati	ion and water	
38.40	2,043	4,215			
38.45	2,043	4,303	quality	volume	
38.50	2,043	4,390			
38.55	2,043	4,478			
00.00	2,040	4,470			
			I		



### APPENDIX F

DRAINAGE MAPS

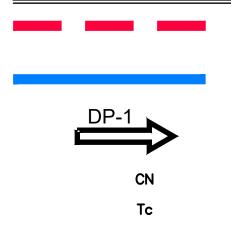
ED-1 – Existing Drainage Plan PD-1 – Proposed Drainage Plan

PD-2 – Proposed Hydraulic Map

# EXISTING HYDROLOGY

DRAINAGE AREA	TOTAL AREA	IMPERVIOUS AREA (S.F.)	PERVIOUS AREA (S.F.)	PERCENT IMPERVIOUS (%)	CN	
EDA-1	46,195	13,355	32,840	28.9%	85	
EDA-2	1,863	1,261	602	67.7%	92	
TOTAL AREA:	48,058	14,616	33,442			

## LEGEND

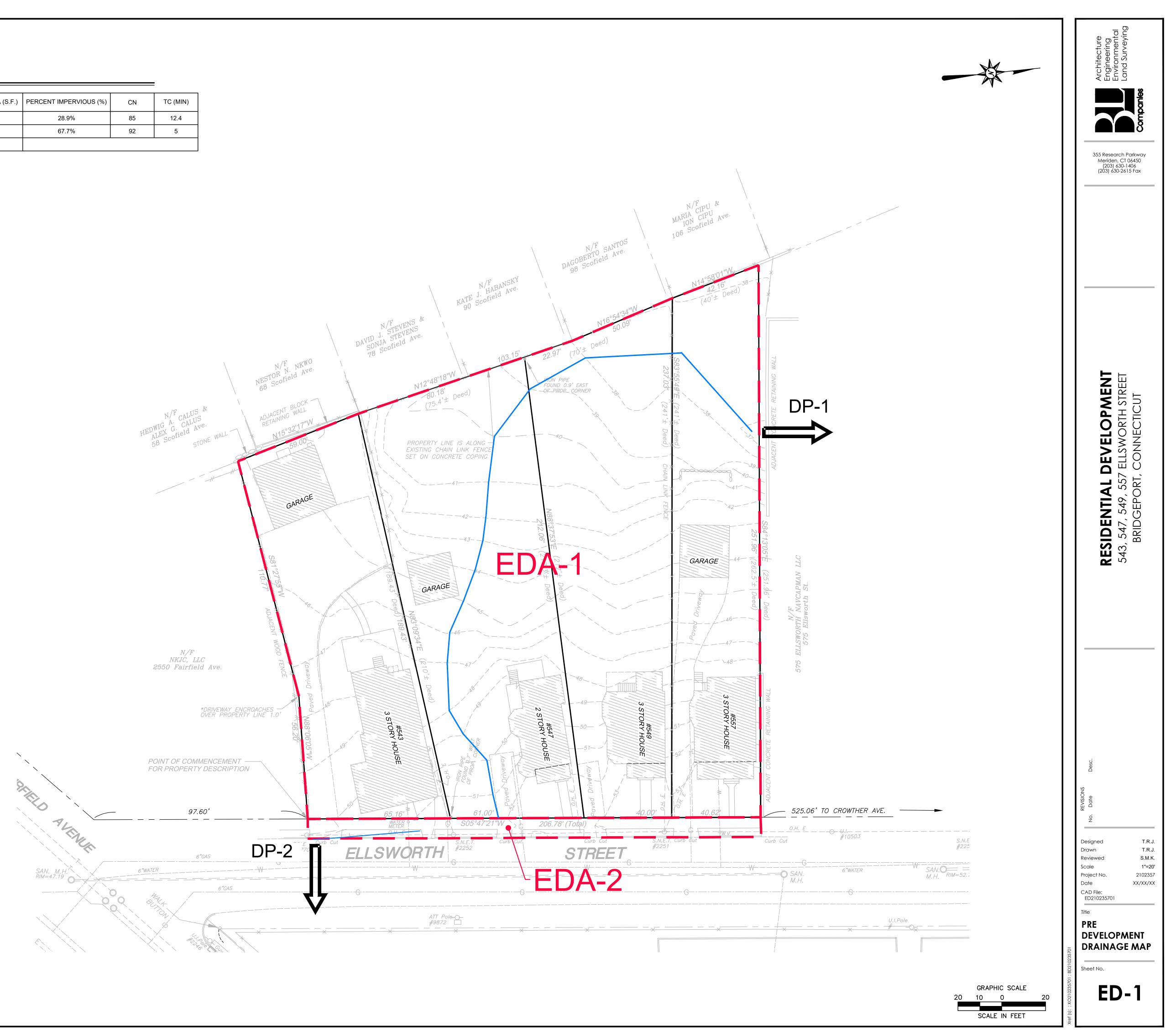


EXISTING DRAINAGE AREA BOUNDARY TIME OF CONCENTRATION PATH DESIGN POINT CURVE NUMBER

TIME OF CONCENTRATION

## NOTES

- THE FOLLOWING IS CONSIDERED "IMPERVIOUS AREA": BITUMINOUS DRIVEWAYS, BITUMINOUS WALKS, BITUMINOUS CURB, CONCRETE DRIVEWAYS, CONCRETE WALKS, CONCRETE PADS, CONCRETE CURB AND BUILDINGS.
- AND BUILDINGS. 2. ONSITE TOPOGRAPHY BASED ON ROSE TISO & CO. FIELD SURVEY PERFORMED ON 10/02/2014.



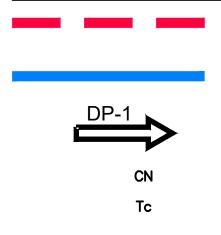
FOR PERMITTING PURPOSES ONLY
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# PROPOSED HYDROLOGY

DRAINAGE AREA	TOTAL AREA	IMPERVIOUS AREA (S.F.)	PERVIOUS AREA (S.F.)	PERCENT IMPERVIOUS (%)	CN
PDA-1A	36,188	36,188	0	100.0%	98
PDA-1B	8,112	49	8,063	0.6%	80
PDA-2	3,758	1,492	2,266	39.7%	87
TOTAL AREA:	48,058	37,729	10,329		

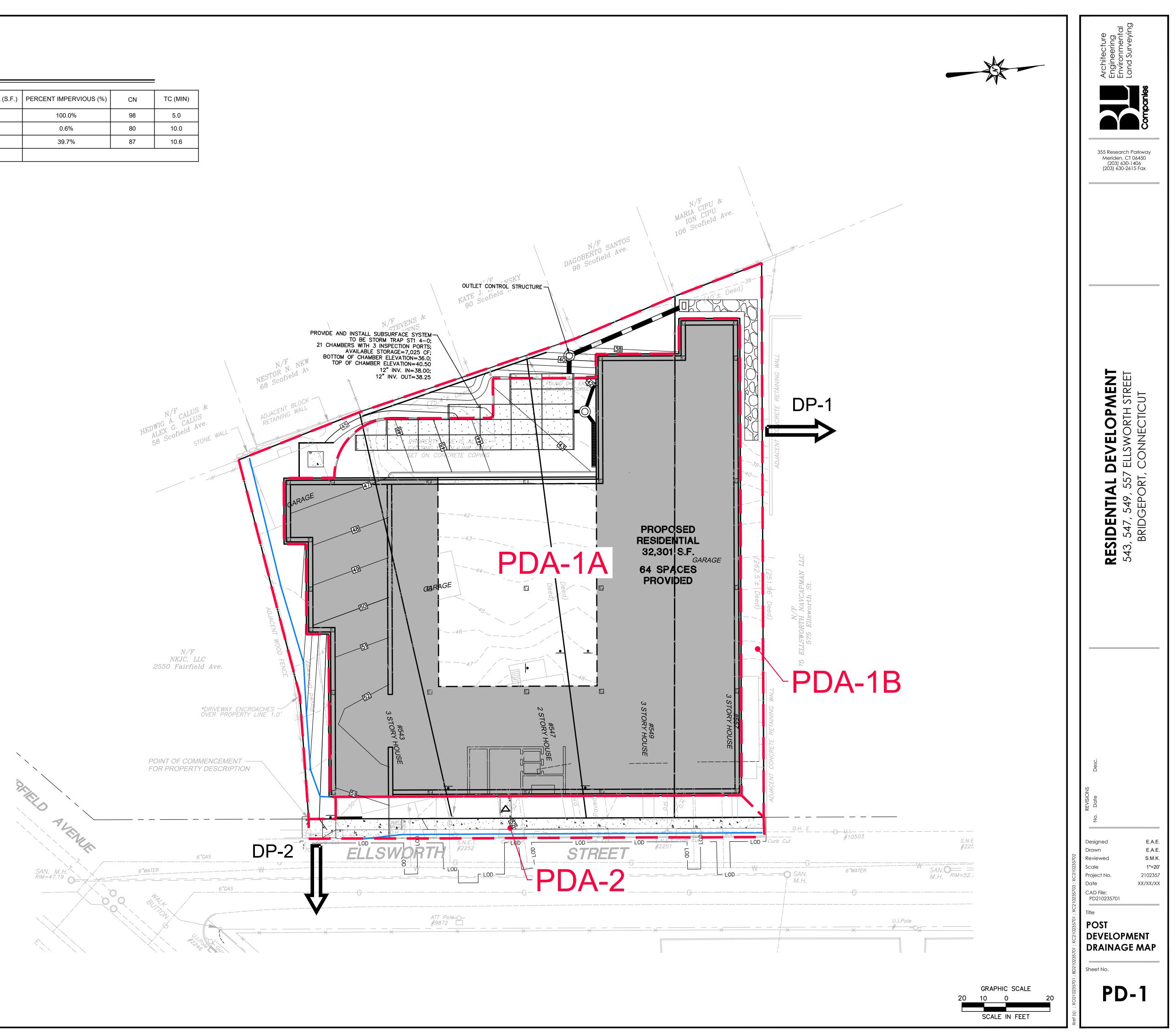
## LEGEND



PROPOSED DRAINAGE AREA BOUNDARY TIME OF CONCENTRATION PATH DESIGN POINT CURVE NUMBER TIME OF CONCENTRATION

## NOTES

- THE FOLLOWING IS CONSIDERED "IMPERVIOUS AREA": BITUMINOUS DRIVEWAYS, BITUMINOUS WALKS, BITUMINOUS CURB, CONCRETE DRIVEWAYS, CONCRETE WALKS, CONCRETE PADS, CONCRETE CURB AND BUILDINGS.
- 2. ONSITE TOPOGRAPHY BASED ON ROSE TISO & CO. FIELD SURVEY PERFORMED ON 10/02/2014.



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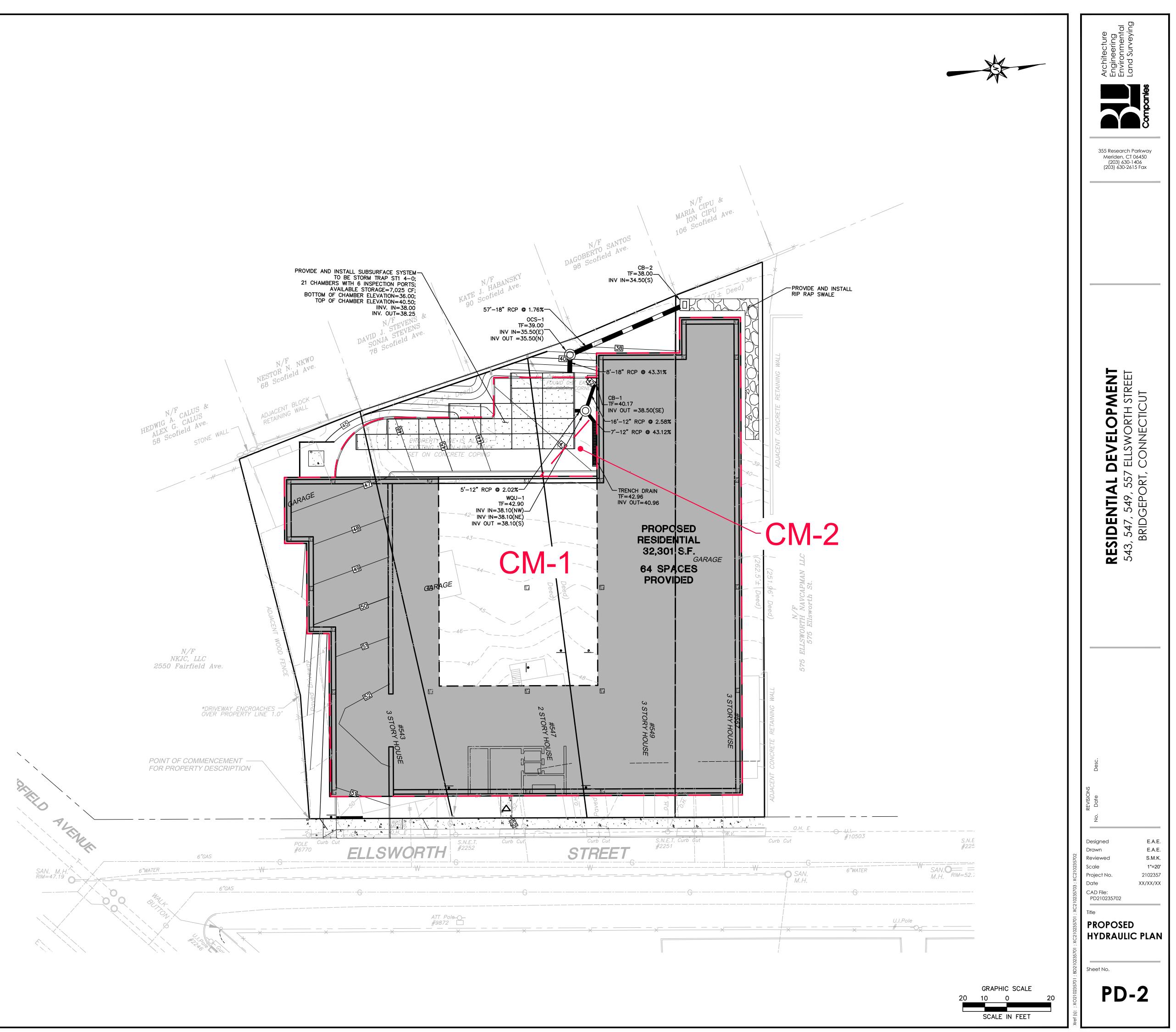
## NOTES

- 1. THE FOLLOWING IS CONSIDERED "IMPERVIOUS AREA": BITUMINOUS DRIVEWAYS, BITUMINOUS WALKS, BITUMINOUS CURB, CONCRETE DRIVEWAYS, CONCRETE WALKS, CONCRETE PADS, CONCRETE CURB
- AND BUILDINGS. 2. ONSITE TOPOGRAPHY BASED ON ROSE TISO & CO. FIELD SURVEY PERFORMED ON 10/02/2014.



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### APPENDIX G

### Geotechnical Report

## Geotechnical Engineering Report For Proposed Construction of:

Apartment Building 543-557 Ellsworth Street Bridgeport, CT

Prepared for: Jacabacci Construction Association, Inc. 30 Oakland Avenue Milford, CT 06460

> Prepared by: Atlantic Consulting & Engineering, LLC 525 John Street Bridgeport, CT 06604

> > January 29, 2016

### ENGINEERING REPORT TABLE OF CONTENTS

- 1.00 GENERAL SUMMARY
- 2.00 INTRODUCTION
  - 2.10 OBJECTIVE OF STUDY
  - 2.20 GEOTECHNICAL SCOPE OF SERVICES
  - 2.30 SITE AND PROJECT DESCRIPTION
- 3.00 SUBSURFACE EXPLORATIONS
- 4.00 SUBSURFACE CONDITIONS
- 5.00 IMPLICATIONS OF SUBSURFACE CONDITIONS
  - 5.10 FILL/TOPSOIL
  - 5.20 ALLUVAIL DEPOSITS
  - 5.30 ROCK
  - 5.40 GROUNDWATER
- 6.00 DESIGN OBSERVATIONS
  - 6.10 PIERS AND SPREAD FOOTINGS
  - 6.20 SLAB ON GRADE
  - 6.30 PAVED AREAS
  - 6.40 SEISMIC CHARACTERISTICS/LIQUEFACTION POTENTIAL
  - 6.50 SOIL LATERAL LOADS

#### 7.00 CONSTRUCTION AND EARTHWORK CONSIDERATIONS

- 7.10 FLOOR SLABS
- 7.20 PAVEMENTS
- 7.30 MATERIALS, PLACEMENT AND COMPACTION
- 7.40 CONSTRUCTION MONITORING SERVICES
- 8.00 FINAL COMMENTS
- FIGURE 1 : BORING LOCATION PLAN
- APPENDIX A : BORING LOGS 1 Through 19

#### 1.00 GENERAL SUMMARY

Based on the studies performed as discussed herein, we have prepared the following conclusions and recommendations.

- 1.) Variable density fill, alluvial, rock and weathered rock deposits are present in the portions of the proposed construction area that were investigated. Liquefaction potential is low based on density and gradation of soils, depth of water table and rock depth.
- 2.) Unsuitable materials (fill) are unacceptable design bearing surfaces. The existing naturally deposited inorganic sand and silt materials can be used to support the bottom of footings.
- 3.) If required, raises in grade materials beneath the slabs and pavement should consist of structural fill.
- 3.) Replacement fills for footing, slab and pavement support as required should consist of "structural fill" as defined in paragraph 7.30 and be placed and compacted to 95 percent of the optimum dry density per ASTM D-1557.
- 4.) <u>Groundwater is expected to impact portions the excavation</u> or cut areas of the proposed project so a dewatering plan needs to be developed primarily for the eastern portion of the building.
- 5.) Footings shall be excavated to naturally deposited inorganic materials as defined herein and the grade can be raised using structural fill since the acceptable bearing material is below the frost line. Bearing surfaces within the proposed footing areas are to be at least 3.5 feet below the existing grade which is a moot point if the underground parking is constructed.
- 6.) Provided bearing surfaces are prepared as described herein, an allowable soil bearing capacity of 8,000 pounds per square foot may be used for design purposes in sizing the footings and foundations. If structural fill is used to raise the bearing grade more than 12 inches, 6,000 pounds per square foot can be used in the design.
- 7.) Rock encountered during the exploration is relatively shallow in a few areas so the bearing capacity can be assumed to be 30,000 pounds per square foot, if encountered. Rock excavation of both boulders and possibly hammering blasting is anticipated in the western portion of the building footprint.
- 8.) Based on the permeability of the soils, footing drains are not required
- 9.) All work to prepare in-place materials and to construct foundation systems should be performed under the observation of the geotechnical engineer. Specific important details of our geotechnical engineering study and recommendations are enclosed herein.

#### 2.00 INTRODUCTION

This report presents the results of an engineering study performed by Atlantic Consulting & Engineering (ACE), at the site of the proposed Apartment Building located at 543-557 Ellsworth Street in Bridgeport, CT. Included in this report are a summary of subsurface conditions observed and the implications of these conditions with respect to the design and construction of the proposed structure. Please note that this report is subject to the limitations contained in Section 8.00.

#### 2.10 OBJECTIVE OF STUDY

The objective of our scope of services was to explore subsurface conditions within the proposed structure and develop geotechnical recommendations for the design of the foundation support for the proposed structure. Included are design criteria for proposed slab on grade and pavement sections.

#### 2.20 GEOTECHNICAL SCOPE OF SERVICES

The scope of services performed by ACE to meet the above stated objectives for geotechnical services included the following:

Inspection of the test borings conducted by Soiltesting, Inc. between January 4th and 15th, 2016.

Evaluation of the fill samples and the underlying ALLUVIAL DEPOSITS.

Recommendations were prepared for foundation and pier support for the proposed structure.

Recommendations for slab and pavement section design have been prepared.

General recommendations have been made as to earthwork and foundation construction procedures to be followed during the construction phase of this project.

#### 2.30 SITE AND PROJECT DESCRIPTION

The site is located on the western side of Ellsworth Street in Bridgeport, CT. Commercial sites are south the subject property and residential is located to the north and west. The subject site contains three wooden residential structures. The topography slopes generally from east to west with grades varying from elevation 50 at Ellsworth Street to a low area of elevation 38 in the northwest corner. The three story apartment building with underground parking is planned to be constructed generally in the center of the site.

#### 3.00 SUBSURFACE EXPLORATIONS

Subsurface explorations performed for this project consisted of hollow stem augured borings. Borings were terminated in alluvial deposits and on bedrock in some cases.

Test borings were located and drilled by Soiltesting, Inc. Approximate locations of borings are shown on the Boring Location Plan. Nineteen (19) test borings were advanced throughout the site. Copies of the test boring logs are included in Appendix A, along with a boring location plan. Test boring locations should be considered accurate only to the degree implied by

measuring method used to determine them. The test borings were conducted using a truck mounted drill rig. Soil samples from the test borings were classified both on site and in the lab and on site.

#### 4.00 SUBSURFACE CONDITIONS

All explorations revealed naturally deposited inorganic material beneath the fill and topsoil layers. Medium dense gravel, silts and sands underlain by rock were predominant throughout the exploratory effort. This material appears to be well draining and stable to work on and is desirable as bearing material and should be prepared as outlined below. Shallow rock was encountered in some borings. Water may affect the excavation work and stability of in situ soils.

#### 5.00 IMPLICATIONS OF SUBSURFACE CONDITIONS

#### 5.10 FILL/TOPSOIL

The borings showed that topsoil, subsoil and/or fill were present throughout the site. Between 2 and 4.6 feet of fill, loamy subsoil and fill were overlying naturally deposited materials. These materials are unsuitable to support footings, nor be re-used for structural fill. They are all above the elevation of the subsurface parking, but will need to be addressed in the proposed paved areas of the site.

#### 5.20 ALLUVIAL DEPOSTS

Throughout the site beginning immediately beneath the topsoil and fill an alluvial deposit was encountered. The material is a medium to dense compact grey or brown sand, silt and gravel mix. This alluvial material overlies the rock and ranges in depth from as shallow as 7 feet at boring B-6 and 10 feet at boring B-3. Boulders and cobbles overly the subject stratum to a depth of 10 feet in boring B-5. The remaining explorations indicate the alluvial deposits occur well above and below the proposed bottom of footing elevation. The characteristics of this material make it suitable for footing support, and this should be the design bearing material for the project. Some of this material **may** meet the structural fill requirements outlined in section 7.30 and therefore could be reused as structural fill for raises in grade beneath footings and slabs, furthermore it appears to be suitable to raise the grade in paved areas provided the final 8 to 12 inches area prepared in accordance with Paragraph 7.30 below.

#### 5.30 <u>ROCK</u>

Rock was encountered in many of the borings as auger refusal and coring occurred far below the anticipated bottom of footing elevation in most cases, however, as indicated above, rock elevations were high in borings B-3 and B-6, the northwest corner of the site and building and boulders were present to a depth of 10 feet in boring B-5 which is within the western central edge of the proposed footprint. Otherwise the rock depths indicated in the exploration fall beneath the proposed depth of construction. It is probable that there are large nested boulders that are above the bedrock revealing that possibility.

#### 5.40 GROUNDWATER

Groundwater was encountered in the explorations; typically, the water table is "perched" above many rock formations. The elevation of the water table is well below design bottom of footing

in most case except in the eastern portion of the building footprint where the elevation varies between 8 to 12 feet below existing grade. It is in this area that it is anticipated that dewatering will be needed to keep the bearing surfaces dry for the deeper footings that are proposed. The water table may be considered as "perched" above the ledge and most likely fluctuates with the highest elevation being in the early spring, therefore having a high probability of affecting footing excavation.

#### 6.00 DESIGN OBSERVATIONS

It is our recommendation that removal of the existing fill followed by replacement with suitable compacted structural fill beneath the bottom of strip and pier footings (if necessary) or construction of the footings directly on the Alluvial Stratum which all indication will be the case. If in-place material is determined by the Geotechical Engineer to be acceptable after visual observations, then areas beneath the slabs can be prepared as described in Section 7.10. Where bearing surfaces require a raise in grade, structural fill can be placed above the existing alluvial deposits as described in Section 7.30.

#### 6.10 PIER and SPREAD FOOTINGS

Excavation to naturally deposited inorganic materials is an effective approach for this project due to the relatively shallow depth of the unsuitable materials in the major portion of the construction area. Spread footings can bear directly on alluvial deposits or structural fill can be used to raise the grade to a minimum of 42 inches below finish grade if any shallow footings are used. There would most likely be an excavation to approximately 6 to 7 feet below grade to remove the unsuitable soils. Since the water table is relatively high in the eastern portion of the footprint, there would need to be a concerted effort and plan to keep the water table 24 inches below working surfaces to be developed by the dewatering contractor. When structural fill is used to raise the grade to the bottom of footing, the compacted area shall extend 12 inches beyond the edge of the footing for every 12 inches of structural fill placed, for example if 2 feet of fill were used to raise the grade for a 4x4 footing, the actual area of structural fill should be 8x8 (2 feet along each side).

#### 6.20 SLAB ON GRADE

It is recommended that a 4 to 6 inch slab on grade be used to support floor loads. This may also be supporting the garage floor. The slab should over-lie 8 inches of free draining sand and gravel. Which can also be accomplished by the following: excavate 8 inches below bottom of slab having the Geotechnical Engineer observe proof rolling prior to placement of and compaction testing of the structural fill or free draining sand.

#### 6.30 PAVED AREAS

The subgrade soil for pavement will consist of varying depths of the existing fill, subsoil and alluvial materials currently in place at the site, some of which are poorly draining. Our proposed pavement cross section consists of the following:

Roadways and Auto Parking Areas

- 4 inch Two 2" Bituminous Concrete Courses (Class 1 and 2)
- 4 inch Process Aggregate Base
- 8 inch Structural fill placed on compacted subgrade proofrolled prior to lift placement with a 20 ton vibratory roller.

The above cross section is considered acceptable provided the existing materials are proofrolled and approved by the engineer. All subsequent replacement fills required beneath the subbase should consist of compacted structural fill. Any areas where weaving is observed should be locally excavated and replaced using structural fill. Given the fact that some paved areas may be within the loose loamy subsoil, the depth of excavation depth may need to be increased to attain stable supporting soils. Proof-rolling in the presence of the engineer will enable determination of the stability of that soil.

#### 6.40 SEISMIC CHARACTERISTICS and LIQUEFACTION POTENTIAL

For structural design, the IBC Seismic Site Soil Classification is considered to be "D". The site classification is reduced to "A" if the bottom of all the footings were less than 10 feet from the rock surface which is not the case for the majority of the building. The mapped spectral response acceleration for 1 second period is S1=0.064 and for short periods Ss=0.270. For transfer of ground shear into the naturally deposited inorganic sands, a factor of 0.35 can be assumed.

Based on the results of the borings and the SPT sampling, the subsurface conditions at the site should be considered as having an extremely low or negligible potential for liquefaction due to the density and gradation of the silt and sand coupled with the shallow depth of the rock.

#### 6.50 SOIL LATERAL LOADS

Foundation walls and retaining walls should be designed to resist lateral loading. At optimum densities and in moist conditions, the design lateral loads in pounds per square foot per foot of depth shall be 40. Submerged or saturated soil pressure used in design shall include the weight of buoyant soil plus hydrostatic loading.

#### 7.00 CONSTRUCTION AND EARTHWORK CONSIDERATIONS

Development of the proposed site may entail some soil and foundation oriented problems especially with respect to the existing fill and potential groundwater within the footprint of the proposed building areas. Grading problems may also occur if the work is carried out in wet weather due to the silt content of some of the onsite materials. The recommendations presented in this report are predicated upon site preparations, foundation wall construction, floor slabs and pavement construction monitored under controlled conditions and the direction of the geotechnical consultant.

It is recommended that placement of the concrete for piers and footings take place shortly following the preparation of the design bearing surface, since the introduction of water may adversely affect its structural characteristics. **Dewatering should take place throughout the operation if excavation near the water table takes place**. To insure minimum disturbance to bearing surfaces, the water table should be 24 inches below all working areas.

Incidental rock excavation is expected to take place in the vicinity of boring B-6 which is within the northwest corner of the proposed footprint. Additionally there is boulder excavaction anticipated in the vicinity of boring B-5 (western edge of footprint)

#### 7.10 FLOOR SLABS

Prior to placement of new structural fill, or free-draining sand, gravel base course materials, all deleterious materials, including topsoil and fill should be removed from within the limits of the building to the minimum depth below finish floor as determined by the structural engineer. The exposed subgrade materials should then be proofrolled with a minimum of 4 passes of a 20 ton roller in the presence of the undersigned. Any observed soft or weaving areas should be locally excavated and replaced with compacted structural fill. The final 8 inches of free draining sand and gravel shall be placed as defined in section 7.30. A 4 to 6 inch slab on grade is recommended for the use described herein, depending on the proposed loading.

#### 7.20 PAVEMENTS

Prior to placement of new pavement section materials, the in-place fill materials should be removed to a minimum depth of 16 inches below the bottom of finish pavement grades unless the alluvial stratum is encountered at which point it may remain in place. Existing bearing surfaces should be proofrolled and subgrade should then be prepared as outlined under Section 7.10 and 7.30. Raises in grade below pavement section materials should be performed using structural fill, acceptable on site material and processed base as described in section 6.30

#### 7.30 MATERIALS, PLACEMENT AND COMPACTION

Structural fill to be used in backfilling within the building areas below footings and pavements, below the recommended 8 inch sand-gravel floor slab base course, and beneath the recommended pavement section, should be free from ice, snow, roots, stumps, and other deleterious materials. Structural fill should consist of a sandy GRAVEL or gravely SAND material having a liquid limit and plasticity limit not exceeding 40 and 15, respectively, and conform to the following gradation requirements:

<u>Sieve Size</u>	Percent Finer by Weight
3.5 inch	100
No. 4	30 - 65
No. 10	20 - 50
No. 40	5 - 30
No. 100	0 - 10

**Free draining sand and gravel** for the pavement base course, whether existing or to be placed, should be free of ice, snow, roots, stumps, rubbish, and other deleterious materials and should consist of hard durable sand and gravel conforming to the following gradation requirements:

<u>Sieve Size</u>	Percent Finer by Weight
2 inch	100
1/2 inch	50 - 85
No. 4	40 - 75
No. 50	8 - 28
No. 100	0 - 10

All building areas, structural fill base course free draining sand-gravel fill, pavement base course and pavement sub-base material, should be placed in lifts not exceeding 8 inches in loose lift thickness and should be compacted to at least 95 percent of maximum dry density per ASTM D-1557. New structural fill required exterior to structural element (footings, foundation or retaining walls and pavements) zone of bearing should be compacted to at least 93 percent of the maximum dry density per ASTM D-1557.

If it is necessary to re-use existing acceptable on-site materials, compaction can be carried out by placing the material in lifts not exceeding 6 inches and should be compacted to a minimum of 95 percent of maximum dry density per ASTM D-1557. This cannot be conducted in wet weather, nor if the moisture content of the material is at a level where the desired compaction cannot be physically achieved. Proctor tests, ASTM D-1557, will have to be conducted on samples of any fill desired to be reused. All reused material shall be free of roots, stumps, ice, snow, organic and any other deleterious materials.

#### 7.40 CONSTRUCTION MONITORING SERVICES

It is recommended that Atlantic Consulting & Engineering and Fairfield Testing Laboratory be retained to provide geotechnical engineering and construction monitoring services during the excavation, foundation, and construction phases of the project. The purpose of these services is to observe compliance with the design concepts, contract documents, and geotechnical recommendations and to allow orderly design changes during construction in the event that subsurface conditions differ from those anticipated prior to the start of construction.

During construction, the Atlantic Consulting & Engineering and Fairfield Testing Laboratories field representatives are recommended to be present to provide controlled inspections including with the following:

- 1. Observe the general progress of site work.
- 2. Perform the required field control tests for earthwork, including proof-rolling sub-grades and placement of structural fill.
- 3. Observe earthwork operations to ensure that the minimum compactive effort and maximum lift height restrictions are enforced.
- 4. Observe, evaluate, and judge the suitability of prepared bearing surfaces including the possibility of using existing fill materials below slabs.
- 5. Observe and evaluate unanticipated subsurface conditions, when and where encountered and alternate procedures, which are proposed to address those unanticipated subsurface conditions.
- 6. Conduct inspections of concrete and masonry, reinforcing steel, and structural steel and framing inspections required by the city and state and directed by The Statement of Special Inspections.
- 7. Review the proposed design and installation of dewatering system.

#### 8.00 FINAL COMMENTS

This report has been prepared for specific application to the subject project in accordance with generally accepted soil and foundation engineering practices. No other warranty, expressed or implied, is made. In the event that any changes in the nature, design or location of structures are planned, the conclusions and recommendations contained in the report should not be considered valid, unless the changes are reviewed and conclusions of this report modified or verified in writing.

The analyses and recommendations submitted in this report are based in part upon the data obtained from the referenced test borings. The nature and extent of variations between explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendation of this report.

Atlantic Consulting & Engineering should perform a general review of final design and specifications in order to determine that earthwork and foundation recommendations have been properly interpreted and implemented in the design specifications.

Respectfully Submitted by

James E. Quill

James E. Quill, PE CT PE#14358

# Figure 1

### **Boring Location Plan**

## **APPENDIX A**

### Boring Logs 1 through 19

Conducted between January 4 and January 14, 2016

# SOILTESTING, INC.

TOJacabacci Construction Associates Inc.	DATE	January 26, 2016
ADDRESS		
SITE LOCATION Proposed 4 Story Apt Building, 543 - 557 Ellsworth Street, Bridgeport, C		
REPORT SENT TO Bill Jacabacci, CPE		
SAMPLES SENT TO Storage (Max. 60 days)		

90 Donovan Road Oxford, Connecticut 06478-1028 203-262-9328

Branch Office: White Plains, New York 10607 914-946-4850 јов NO. G267-0245-15 Phone (203) 262-9328

Telefax (203) 264-3414 WHITE PLAINS, N.Y. (914) 946-4850

## SOILTESTING, INC.

#### 90 DONOVAN ROAD - OXFORD, CONN. 06478-1028

GEOTECHNICAL / ENVIRONMENTAL SUBSURFACE INVESTIGATIONS - Test Borings - Core Drilling Monitoring Wells - Recovery Wells - Direct Push/Probe Sampling UNDERPINNING - HELICAL PILES - SOIL NAILS

January 26, 2016

Jacabacci Construction Associates Inc. 30 Oakland Avenue Milford, CT 06460 203-257-3928

Attn: Bill Jacabacci, CPE

Re: Proposed 4 Story Apt Building 543 - 557 Ellsworth Street Bridgeport, CT G267-0245-15

Dear Mr. Jacabacci,

Enclosed are boring logs and location plan for the above referenced project site.

If you have any questions, please do not hesitate to contact us.

Very truly yours, **SOILTESTING, INC.** 

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Jámes A. DeAngelis President 58<sup>th</sup> ANNIVERSARY 58<sup>th</sup> ANNIVERSARY 190 04FORD SI

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	SOII 90		STI			<i>.</i>	CLIEN	NT:	Jaca	bacci (	Constructio	on Associ	ation Inc	SHEET <u>1</u> 0 HOLE NO.	F <u>1</u> B-1
			RD, C				PROJ	ECT NO	D.	G267	-0245-15				5-1
			3) 26				PROJ	ECT NA	AME					BORING LOCATIONS	
	N	<u>(</u> 91	4) 94	46-48	850						557 Ellswo		et	per Plan	
0	REMAN - I	DRILI	ER				LOCA	TION		Bridg	geport, CT	•			
	TP/ad										CASING	SAMPLER	CORE BAR	OFFSET	
110	PECIUR							TYPE			HSA	SAMPLER	NWD4	DATE START	1/4/16
R			OBSE	=R\/A		5	1	SIZE			4 1/4"	1 3/8"	21⁄8"	DATE FINISH	1/15/16
	<u>10</u> FT					-			IER WI	Г.		140#	BIT	SURFACE ELEV.	.,
<u>۲_</u>	FT AF	TER_	_но	URS			ľ	HAMN	IER FA	LL		30"	dia	GROUND WATER ELEV.	
			5	SAMI	ΡLΕ					Ι	[				
	CASING BLOWS PER FOOT	NO	Туре	PEN	REC	DEPTH @ BOT	ON (FOR	WS PEI I SAMP CE ON 6 - 12	LER TUBE)	CORE TIME PER FT (MIN)	DENSITY OR CONSIST MOIST	STRATA CHANGE DEPTH ELEV	1	ENTIFICATION OF SOIL DR, LOSS OF WASH WA IN ROCK, ETC.	
		1	SS	24"	7"	2'0"	2	2			moist		1.5" ASPHALT		
				0.4"	4"	410"	4	4			stiff			I sand, tr roots, ash ( fill )	
		2	SS	24"	<u>4"</u>	4'0"	2 13	4			moist compact	4'0"	BIK FMC SAND	& ASH, tr F gravel ( fill )	
5		3	SS	24"	16"	6'0"	4	4			moist		Brn SILT, sm F	M sand ( poss fill )	
						0101	4	5			stiff	6'0"			
		4	SS	24"	21"	8'0"	12 24	21 25			moist dense		BULLER FUC SANE	) & SILT, lit F gravel	
		5	SS	24"	17"	10'0"	12	16			moist		Brn FMC SAND	) & SILT, lit F gravel, tr C gravel	
0					10"	4010"	21	35			dense		0.005		
		6	SS	24"	19"	12'0"	25	16 18			wet dense		SAME		
											Gense				
5		7	SS	24"	15"	17'0"	21	26			wet		Bro SILT & VE	F to M SAND, tr F gravel	
			- 29		10		27	31			hard			I LO WI OAND, II T GIAVEI	
20															
		8	SS	10"	10"	20'10"	28	60/4"			v dense		Brn highly deco	mposed BEDROCK	
					- 01							00101			
		_1_	C	60"	56"	28'0"	RQD =	<u>= 86%</u> T		1.5 1.5			AUGER REFUS BEDROCK	JAL	
5										1.5			BEBROOK		
										1.5					
										1.5		28'0"		E.O.B. 28'0"	
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	TE. 0.,4	ارم	con	ditio	ne r	Avealor	t hv ti	his in	/eetin	ation	represent				
U	con	ditio	ns a	t spe	cific	c locatio	ons al	nd ma	y not	repres	epresent				
R	CON DUND SU					cations				CASING	G THEN	CA	SING TO	FT. HOLE NO.	B-1
	AUGER				BED	PISTON		T = TH	INWAL	L	V = VANE T				
	R = WEIG = SPLIT TI										)S			C = COARSE M = MEDIUM	
> =	- SPLIT I		SAIVIP			17.3.A. =		- 10 C			20 - 35% Al			= FINE	

OXFORD, CT 06478 CT (23) 329-932         PROJECT NO.         G267-9245-15           POREMAN, URALLER BD/85         PROJECT NO.         643-557 Ellsworth Street         PROVIDE CONTINUE PROVIDE CONTINUE BD/85           NOP-OCTOR         EVENT         CONTINUE PROVIDE CONTINUE BD/85         DATE START         1/12/16           BD/85         CONTON         Product VALUE BD/85         DATE START         1/12/16           CASING BC/UARD WATER OBSERVATIONS AT_ET ATTER_DUCKS         TATAT         TATER NOURS         ATTATER         DATE START           CASING BC/UARD WATER OBSERVATIONS AT_ET ATTER_DUCKS         RUGWS FER 6 IN CONS PROFIL         CONSERT CONSERT         STRATA CONSERT         TATATA SURVECT NOUND WATER CLEV         DATE START         1/12/16           CASING BC/UARD WATER OBSERVATIONS AT_ET ATTER_DUCKS         RUGWS FER 6 IN CONSERT         CONSERT         DETMINATION         TATATATATATATATATATATATATATATATATATATA		SOI					).	CLIEN	IT:	Jaca	bacci (	Constructio	on Associ	ation Inc	SHEET_1_OF_ HOLE NO.	
CT (20.3) 263-9326         PHOLECT NAME         State Street         DORING LOCATIONS           BD/ins         LOCATION         Bridgeport, CT         por Plan           BD/ins         Income Street         por Plan           Normal Color         Provide Street         por Plan           Normal Color         Provide Street         por Plan           CROUND WATER DESERVATIONS         Size LD         4 Xr         1 30°           ALB_FT AFTER HOURS         NAMMER WT.         4 Xr         1 30°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         4 Xr         1 30°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         4 Xr         1 30°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         4 Xr         1 30°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         4 Xr         1 30°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         4 Xr         1 30°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         CONSIST         STRATA           FIELD IDENTIFICATION OF SOLL REMARKS         NAMMER WT.         MAMER WT.         100°           BLOWS IN NUTRE DESERVATIONS         NAMMER WT.         MAMER WT.         STRATA           FIELD COLOR COLOR DES OF MANNES         NAMME								PROI		<u>ר</u>	G267	-0245-15			HOLE NO.	B-2
NY (914) 945-8450         543-57 Elsworth Street         per Plan           Boljas         COATION         Bridgeport, CT         Image of the street of the str				•							0207	-0240-10			BORING LOCATIONS	
BD/jas         CANC 3 AMPLE         CANC 3 AMPLE <thcanc 3="" ample<="" th="">         CANC 3 AMPLE</thcanc>			•	•							543-5	57 Ellswo	orth Stree	et		
INSPECTOR         CASING         SAMPLE         CPSET         T/1/1/16           GROUND WATER 08558/MTONS         SL2 L.D.         13/8         13/8         DF2 FM FT AFTER_UPONS         DF2 FM FT AFT	FO		DRILL	.ER				LOÇA	TION		Bridg	jeport, Cl	Г			
TYPE         HSA         SS         DATE START         1/12/16           GRUND WARE OBSERVATIONS AT_ET AFTER_0_HOURS         SIZE LD         4 ½ 1 3/8"         DATE FINISH         1/12/16           AT_ET AFTER_0_HOURS         HAMMER VT. HAMMER VIL         30"         GROUND WARER LEV.         SROUND WATER REV.           CASING         SAMPLE         BLOWS PER 8 IN 00 SAMPLER         DOINS DEPTH         TRATA (FOC CONST         FIELD IDENTIFICATION OF SOLR REMARKS IN ROCK, ETC.           FPER         SAMPLE         BLOWS PER 8 IN 00 SAMPLER         DOINS DEPTH         FIELD IDENTIFICATION OF SOLR REMARKS IN ROCK, ETC.           0         1 as 2/4 16'         2'         4         stiff         FIELD IDENTIFICATION OF SOLR REMARKS IN ROCK, ETC.           0         1 as 2/4 16'         2'         4         stiff         Start AROTS, an F sand           10         1 as 2/4 16'         2'         4         stiff         Growed         Start AROTS, an F sand           10         1 as 2/4'         10'         16'         Start AROTS, an F sand         Gry FM SAND & SILT, if cobbies           10         1 as 2/4'         10'         10'         2'         2'         Gry FM SAND & SILT, if cobbies           20         1 as 2/4'         10'         2'         2'         Gry FM	INS											CASING	SAMPI FR	CORE BAR	OFESET	
GROUND WATER OBSERVATIONS AT_EFT AFTER_HOURS         SIZE ID. TATE_FT AFTER_HOURS         SIZE ID. TATE_FT AFTER_HOURS         SIZE ID. TATE_FT AFTER_HOURS         SIZE ID. TATE_FT AFTER_HOURS         TATE_FT AFTER_HT         TATE         GROUND WATER BELV         GROUND WATER BELV         GROUND WATER BELV         TATE		LOTOR							TYPE					CONLIDAN		1/12/16
AT_FT         ATTER_HOURS         HAMMER FALL         30"         GROUND WATER ELEV.           Image: CASING Model of the period of the perio	GR		TER	OBSE	ERVA	TION	S								DATE FINISH	
E         SAMPLE         BLOWS FER 0 IN CORE PER         DENSITY CORE (FORCE ON TUBE) FER (FORCE ON TUBE) FER (FORC						S			HAMN	IER WI		·		BIT		
CASING B         Corrections (Corrections at other controls) (Corrections at other controls) (Corrections) (	AT_	FTAF	TER_						HAMN	1ER FA	LL		30"		GROUND WATER ELEV.	
BLOWS         NO         Type         PEN         PEC         BLOWS         PEC         ON SAMPLER         TWO         DePTH         CONSIST         DEPTH         INCL. COLOR. COLOR, SD F WASH WATER, SEAMS           BLOWS         1         1         82         24'         16'         20'         2         3         ELEVE         IN ROCK, ETC.           IN ROCK         1         83         24'         16'         20'         2         3         ELEVE         IN ROCK, ETC.           IN ROCK         1         83         24'         16'         20'         IN ROCK, ETC.         IN ROCK, ETC.           IN ROCK         12         36         40         IN ROCK         IN ROCK, ETC.         IN ROCK, ETC.         IN ROCK, ETC.           IN ROCK         12         30         40         IN ROCK         IN ROCK, ETC.         IN ROCK, ETC.         IN ROCK, ETC.           IN ROCK         14'         17'         18'         IN ROCK         IN ROCK, ETC.         IN ROCK, ETC				5	SAMI T		1	-					075.171			DEMADKS
FOOT         I         Iss         24'         16'         20'         2         3         MOIST         ELEV           1         1         5         24'         16'         20''         2         4         moist           1         2         16'         20''         2         4         moist         moist           1         3         85         24'         16''         20''         3         moist           1         4         5         25         22''         8'''         4'''         3''''         3'''''           1         4         4         5         24'''''         16''''''''''''''''''''''''''''''''''''	Ξ	CASING										1	1			
FOOT         I         Iss         24'         16'         20'         2         3         MOIST         ELEV           1         1         5         24'         16'         20''         2         4         moist           1         2         16'         20''         2         4         moist         moist           1         3         85         24'         16''         20''         3         moist           1         4         5         25         22''         8'''         4'''         3''''         3'''''           1         4         4         5         24'''''         16''''''''''''''''''''''''''''''''''''	EPT		NO	Туре	PEN	REC		(FOR			PER	CONSIST	DEPTH		IN ROCK, ETC.	
Image: second								0-6	6 - 12	12- 18		MOIST	ELEV			
2         ss         24"         14"         40"         moist         moist           5         3         ss         24"         16"         00"         12         30         moist           4         ss         24"         16"         00"         12         30         moist           4         ss         24"         16"         00"         12         30         moist           4         ss         24"         16"         14         14         14         14           10         5         ss         24"         10"         12"         23         moist           10         5         ss         24"         10"         11"         11"         compact           10         5         ss         24"         10"         11"         11"         11"           10         5         5         11"         11"         11"         11"         11"           20         6         ss         24"         10"         220"         23         25         moist           21         9         ss         24"         12"         20"         25         14"         1			1	SS	24"	16"								Brn SILT & RO	OTS, sm F sand	
5         3         8         24'         16'         60'         12         30'         40         well         SAME: It cobbles           10         4         ss         24'         16'         10'         14         14         14         dense well         Bin FM SAND, tr sitt           10         5         ss         24'         10'         10'         22         28         well         GryBm FM SAND, tr sitt           10         6         ss         24'         20'         120'         14         17         well         GryBm FM SAND, tr sitt           16         7         ss         10'''         8''         15'''''         16''''''''''''''''''''''''''''''''''''			2		2/1"	1/1	10"					1	2'0"			
Image: Section of the sectio			2	- 55	24	14	40					4			, shir glavel, it sit	
4       ss       24*       20       80*       12       15       wet       compact wet       compact wet       compact wet       Gry FM SAND, tr solt         10       5       ss       24*       10*       14       17       wet       dense       Gry FM SAND, tr solt         10       6       5       24*       10*       14       17       wet       dense       Gry FM SAND, tr solt         15       7       35       10*       8       50/4*       v dense       vdense       Gry FM SAND & SILT, tr cobbles         20       7       35       10*       8*       50/4*       v dense       vdense       Gry FM SAND & SILT, it cobbles, boulders         20       8       ss       24*       13*       220*       23       25       wet       Gry FM SAND, sm silt, lit C send, F gravel         25       9       ss       24*       12*       27       22       23       wet       dense       32'0*         36       9       ss       24*       10*       10*       32'0*       E.O.B. 32'0*       E.O.B. 32'0*         36       9       ss       24*       10*       10*       10*       10*       10* <t< td=""><td>5</td><td></td><td>3</td><td>SS</td><td>24"</td><td>16"</td><td>6'0"</td><td></td><td></td><td></td><td></td><td>moist</td><td></td><td>SAME; lit cobbl</td><td>es</td><td></td></t<>	5		3	SS	24"	16"	6'0"					moist		SAME; lit cobbl	es	
10       5       ss       24"       16"       10"       28       28       vel       site       GryFM SAND & SILT, tr orbbies       site       site <td></td> <td></td> <td></td> <td></td> <td>2/1</td> <td>20"</td> <td>8:0"</td> <td></td> <td></td> <td></td> <td></td> <td>1</td> <td></td> <td></td> <td>tr cilt</td> <td></td>					2/1	20"	8:0"					1			tr cilt	
10       5       85       24"       10"       28       28       wet       dry danse       GryBm FM SAND, tr cobbles         10       6       55       24"       20"       120"       14       17       wet       dense       GryBm FM SAND, tr cobbles         16       7       55       10"       8"       15"10"       28       50/4"       v dense       Gry FM SAND & SILT, tr cobbles         20       7       55       10"       8"       15"10"       28       50/4"       v dense       Gry FM SAND & SILT, it cobbles         20       6       85       24"       13"       220"       23       25       wet       dense       Gry FM SAND & SILT, it cobbles, boulders         20       8       55       24"       13"       220"       23       25       wet       dense       Gry FM SAND, sm silt, lit C sand, F gravel       Gry FM SAND, sm silt, lit C sand, F gravel       dense       wet       dense       32"       E.O.B. 320"       E.O.B. 32			4	- 55	24	20	00					•		DITT W SAND,	u siit	
6         ss         24"         20"         120"         14         17         wet dense         Gry FM SAND & SILT, it cobbles           15         - <td< td=""><td></td><td></td><td>5</td><td>SS</td><td>24"</td><td>16"</td><td>10'0"</td><td>28</td><td></td><td></td><td></td><td></td><td></td><td>Gry/Brn FM SA</td><td>ND, tr cobbles</td><td></td></td<>			5	SS	24"	16"	10'0"	28						Gry/Brn FM SA	ND, tr cobbles	
15       17       18       17       18       18       dense       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       10"       8"       15"10"       28       50/4"       v dense       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       wet       Gry SILT, sm F gravel         20       9       ss       24"       12"       270"       22       23       wet       dense         20       9       ss       24"       12"       270"       22       23       wet       dense       Gry FM SAND, sm silt, lit C sand, F gravel         25       9       ss       24"       10"       320"       20       18       wet       dense       320"       E.O.B. 320"         36       10       ss       24"       10"       320"       20       18       wet       320"       E.O.B. 320"         40       10       10       ss       24"       10"       32"       20       It is investigation represent conditions at specific locations and may not represent conditions at specific locations and may not represent conditions at specific locations and may not represent conditions at specific locations and may	10		6	66	24"	20"	12'0"							Gry EM SAND	& SILT trachblac	
7       ss       10"       8"       15"10"       28       50/4"       v dense       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       wet       hard         20       8       ss       24"       13"       220"       23       25       wet       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       Gry FM SAND & SILT, lit cobbles, boulders         25       9       ss       24"       12"       270"       22       23       Gry FM SAND, sm silt, lit C sand, F gravel         30       10       ss       24"       10"       320"       Wet       dense       320"         36       10       ss       24"       10"       320"       20       18       wet         40       20       18       wet       320"       E.O.B. 32"0"       E.O.B. 32"0"         36       10       10       10"       32"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10" </td <td></td> <td></td> <td></td> <td>33</td> <td>24</td> <td>20</td> <td>120</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>a oich, a cobbles</td> <td></td>				33	24	20	120								a oich, a cobbles	
7       ss       10"       8"       15"10"       28       50/4"       v dense       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       wet       hard         20       8       ss       24"       13"       220"       23       25       wet       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       Gry FM SAND & SILT, lit cobbles, boulders         25       9       ss       24"       12"       270"       22       23       Gry FM SAND, sm silt, lit C sand, F gravel         30       10       ss       24"       10"       320"       Wet       dense       320"         36       10       ss       24"       10"       320"       20       18       wet         40       20       18       wet       320"       E.O.B. 32"0"       E.O.B. 32"0"         36       10       10       10"       32"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10" </td <td></td>																
7       ss       10"       8"       15"10"       28       50/4"       v dense       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       wet       hard         20       8       ss       24"       13"       220"       23       25       wet       Gry FM SAND & SILT, lit cobbles, boulders         20       8       ss       24"       13"       220"       23       25       Gry FM SAND & SILT, lit cobbles, boulders         25       9       ss       24"       12"       270"       22       23       Gry FM SAND, sm silt, lit C sand, F gravel         30       10       ss       24"       10"       320"       Wet       dense       320"         36       10       ss       24"       10"       320"       20       18       wet         40       20       18       wet       320"       E.O.B. 32"0"       E.O.B. 32"0"         36       10       10       10"       32"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10"       10" </td <td>15</td> <td></td>	15															
8         ss         24"         13"         220"         23         25         wet hard         Gry SiLT, sm F gravel           25         28         26         28         26         44			7	SS	10"	8"	15'10"	28	50/4"			v dense		Gry FM SAND	& SILT, lit cobbles, boulders	
8         ss         24"         13"         220"         23         25         wet hard         Gry SiLT, sm F gravel           25         28         26         28         26         44																
8         ss         24"         13"         220"         23         25         wet hard         Gry SiLT, sm F gravel           25         28         26         28         26         44	l															
25       28       26       hard       hard         25       9       ss       24"       12"       270"       22       23       wet         30       10       25       25       10"       320"       Cry FM SAND, sm silt, lit C sand, F gravel         30       10       ss       24"       10"       320"       wet       dense         30       10       ss       24"       10"       320"       20       18       wet         31       10       ss       24"       10"       320"       20"       E.O.B. 320"         35       10       10       ss       24"       10"       320"       E.O.B. 320"         36       10       10       10       10"       10"       10"       10"         36       10       10"       10"       10"       10"       10"       10"         37       10"       10"       10"       10"       10"       10"       10"         36       10"       10"       10"       10"       10"       10"       10"         NOTE: Subsoil conditions revealed by this investigation represent conditions at other locations or times.       10"       10"	20															
25       9       ss       24"       12"       270"       22       23       wet         30       9       ss       24"       12"       270"       22       23       wet         30       10       ss       24"       10"       32'0"       20       18       wet         40       10       ss       24"       10"       32'0"       20       18       wet         30       10       ss       24"       10"       32'0"       20       18       wet         31       10       ss       24"       10"       32'0"       E.O.B. 32'0"       E.O.B. 32'0"         35       10			8	SS	24"	13"	22'0"							Gry SILT, sm F	gravel	
9       ss       24"       12"       27'0"       22       23       wet       dense       Gry FM SAND, sm silt, lit C sand, F gravel         30       10       ss       24"       10"       32'0"       wet       dense       32'0"         30       10       ss       24"       10"       32'0"       20       18       wet         31       24"       10"       32'0"       20       18       wet       dense       32'0"         35       24"       27       30       dense       32'0"       E.O.B. 32'0"         35       240       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"       24"       24"       24"         40       240       240       240       24"								28	20			nard				
9       ss       24"       12"       27'0"       22       23       wet       dense       Gry FM SAND, sm silt, lit C sand, F gravel         30       10       ss       24"       10"       32'0"       wet       dense       32'0"         30       10       ss       24"       10"       32'0"       20       18       wet         31       24"       10"       32'0"       20       18       wet       dense       32'0"         35       24"       27       30       dense       32'0"       E.O.B. 32'0"         35       240       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"       24"       24"         40       240       24"       24"       24"       24"       24"       24"       24"       24"       24"         40       240       240       240       24"																
30       10       320"       25       25       dense         30       10       35       24"       10"       32'0"       20       18       wet         35       1       27       30       dense       32'0"       E.O.B. 32'0"         35       1       1       1       1       1       1       1         40       1       1       1       1       1       1       1         NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at specific locations or times.       FT.       HOLE NO.       B-2         GROUND SURFACE TO       FT.       USED       CASING       THEN       CASING TO       FT.       HOLE NO.       B-2         A = AUGER       UP = UNDISTUREED PISTON       T = THINWALL       V = VANE TEST       V = VANE TEST       Casing THEN       CASING TO       FT.       HOLE NO.       B-2         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE       SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM	25		0		041	4.01	יימידמ	00	- 02			wat			am ailt lit C aand E araval	
30       10       ss       24"       10"       32'0"       20       18       wet         31       10       ss       24"       10"       32'0"       20       18       wet         32       10       ss       24"       10"       32'0"       E.O.B. 32'0"         35       10       10       10       10       10       10       10         36       10       10       10       10       10       10       10       10         36       10       10       10       10       10       10       10       10       10         37       10			9	55	24		270							GIY FW SAND,	shi shi, ili O sahu, r yi avei	
10       ss       24"       10"       32'0"       20       18       wet dense       32'0"         35       27       30       32'0"       E.O.B. 32'0"       E.O.B. 32'0"         35       20       20       18       20       10       10       10       10         36       20       20       27       30       20       10       10       10       10       10         35       20       20       20       20       20       20       20       10																
10       ss       24"       10"       32'0"       20       18       wet dense       32'0"         35       27       30       32'0"       E.O.B. 32'0"       E.O.B. 32'0"         35       20       20       18       20       10       10       10       10         36       20       20       27       30       20       10       10       10       10       10         35       20       20       20       20       20       20       20       10	20															
30       dense       32'0"         35       36       32'0"         40       36       32'0"         80       36       32'0"         80       36       32'0"         80       36	30		10	SS	24"	10"	32'0"	20	18			wet				
35       36       37 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>dense</td><td>32'0"</td><td></td><td></td><td></td></td<>												dense	32'0"			
40       40 <td< td=""><td></td><td></td><td></td><td>-</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>E.O.B. 32'0"</td><td></td></td<>				-											E.O.B. 32'0"	
40       40 <td< td=""><td>35</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>	35															
NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.         GROUND SURFACE TOFT.       USEDCASING THENCASING TOFT.       HOLE NO.       B-2         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM																
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conditions at other locations or times.         GROUND SURFACE TOFT.       USEDCASING THENCASING TOFT.       HOLE NO.       B-2         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM	NO															
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM		con	ditio	ns at	t oth	er lo	cations	s or tir			_					
WOR = WEIGHT OF RODSWOH = WEIGHT OF HAMMER & RODSC = COARSESS = SPLIT TUBE SAMPLERH.S.A. = HOLLOW STEM AUGERM = MEDIUM		DUND SU	RFAC	E TO		F	T. US	SED						SING TO	FT. HOLE NO.	B-2
	wo	R = WEIG	HT O	FRO	os											
	SS		UBES			NE ~ 0						20 - 35% 1	ND =35 - 50		M = MEDIUM = = FINE	

	SOI			NG, 'AN I		<b>)</b> .	CLIEN	IT:	Jaca	bacci (	Constructio	on Associ	ation Inc	SHEET_1_OF_ HOLE NO.	 
				CT 06			PRO.I	ECT N	<u>ີ</u>	G267	-0245-15				D-3
				62-9:				ECT N/		0207	-0240-10			BORING LOCATIONS	· · · · · · · · · · · · · · · · · · ·
		•	•	46-48				_0110		543-5	57 Ellswo	orth Stree	et	per Plan	
FO	REMAN -	DRILI	LER				LOCA	TION		Bridg	jeport, Cl	-			
	BD/ad														
INS	SPECTOR										CASING	SAMPLER		OFFSET	
							4	TYPE			HSA	SS	NWD4	4	1/8/16
	OUND W/					5		SIZE	I.D. /IER W	-	4 ¼"	<u>1 3/8"</u> 140#	2%" BIT	DATE FINISH SURFACE ELEV.	1/8/16
	FT AF				5				/IER VV		<u> </u>	30"	dia	GROUND WATER ELEV.	
	<u>г</u>			SAM						<u></u> T					
		<u> </u>	Ţ`			<u> </u>	-				DENSITY	STRATA	FIELD ID	ENTIFICATION OF SOIL F	REMARKS
E	CASING							WS PE		CORE	OR	CHANGE		OR, LOSS OF WASH WAT	
DEPTH	BLOWS PER	NO	Type	PEN	REC	DEPTH	(FOR	CE ON	TUBE)	PER	CONSIST	DEPTH		IN ROCK, ETC.	
	FOOT	ĺ		1		@ BOT	0-6	6 - 12	12- 18	FT (MIN)	MOIST	ELEV			
		1	ss	24"	8"	2'0"	1	1			moist		Brn TOPSOIL		
							2	1			v loose	2'0"			
		2	SS	24"	16"	4'0"	12 18	15 22		ļ	moist	3'0"		m silt ( poss fill )	
5		3	SS	24"	10"	6'0"	14	14	1		dense moist		BIN FING SAINL	D, sm cobbles, lit silt, gravel	
		Ť			10		18	19			dense				
		4	SS	24"	18"	8'0"	8	16			wet				
		~	ļ	0.4	4.0%	4.0101	23	20			dense				
10		5	SS	24"	13"	10'0"	20 18	22 18			wet dense	10'0"			
'`		6	ss	24"	14"	12'0"	25	27			wet	100	partially decom	posed BEDROCK	
							30	25			v dense			•	
15		1	С	60"	8"	19'0"	RQD =	0%		2			AUGER REFUS	SAL irtially decomposed / Schist )	
'				00	0	190		070		3			DEDROOK ( pa	initially decomposed / Schist /	
										2					
1										2					
20										2		19'0"		E.O.B. 19'0"	
20														E.O.D. 190	
25					· .										
25															
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30															
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35															
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40															
NO	TE: Sub con	osoil ditio	con ns a	ditio t spe	ns re	evealec locatio	d by th ons an	us inv Id ma	vestig: y not i	ation r repres	epresent ent				
	con	ditio	ns a	t oth	er lo	cations	s or tir	nes.							
									INWALI	CASING	; THEN _ V = VANE TI		SING TO	FT. HOLE NO.	B-3
	AUGER R = WEIG					PISTON WOH = \						_01	C	C = COARSE	
SS :	= SPLIT T	UBE S	SAMP	LER		H.S.A. =	HOLLO	OW STI	EM AUC	SER				M = MEDIUM	
PRC	PORTION	JS US	SED	TRAC	F = 0	- 10%	ITTLE :	= 10 - 2	20% SC	DME = 2	20 - 35% AN	ND =35 - 50	1% F	F = FINE	

	SOI		STI			<b>)</b> .	CLIE	NT:	Jaca	bacci	Constructi	on Associ	ation Inc	SHEET_1_0	
			RD, C				PRO.	ECT N	<u> </u>	G267	7-0245-15			HOLE NO.	B-4
			)3) 2(					ECT N/		0201	-02-40-10			BORING LOCATIONS	
	N	Y (91	14) 94								557 Ellswo		et	per Plan	
FO	REMAN -	DRIL	LER				LOCA	TION		Brid	geport, C1	Г			
	TP/ad SPECTOR						-				CASING	SAMPLER	CORE BAR	OFFSET	•
	LOTOK							TYPE			HSA	SAMFLER	CORE DAR	DATE START	1/4/16
GF		ATER	OBSE	ERVA	TION	S	1	SIZE			4 1/4"	1 3/8"		DATE FINISH	1/4/16
AT	<u>15</u> FT	AFTE	R <u>0</u>	HOUF	RS			HAMN	IER W	Г.		140#	BIT	SURFACE ELEV.	
AT.	FTAF	TER	но	URS			ļ	HAMN	IER FA	LL		30"		GROUND WATER ELEV.	
				SAM	PLE	1									
DEPTH	CASING BLOWS PER	NO	Туре	PEN	REC	DEPTH	ON (FOR	WS PE I SAMP CE ON	'LER TUBE)	CORE TIME PER FT	DENSITY OR CONSIST	STRATA CHANGE DEPTH		ENTIFICATION OF SOIL DR, LOSS OF WASH WA IN ROCK, ETC.	
	FOOT					@ BOT	0-0	6 - 12	12- 18	(MIN)	MOIST	ELEV			
		1	SS	24"	8"	2'0"	2	1			moist		1" ASPHALT		( ( 50)
		2	SS	24"	2"	4'0"	2				soft moist		Brn Sill, sm F	M sand, tr F gravel, ash, coal, r	oots ( poss fill )
							2	2			soft				
5		3	SS	24"	18"	6'0"	2	13	ļ		moist			M sand ( poss fill )	
		4	ss	24"	18"	8'0"	15 20	28 17			v stiff moist			: SAND, sm silt, lit F gravel, tr C ), sm silt, lit F gravel	gravel
							13	13			compact				
1		5	SS	24"	18"	10'0"	11	12			moist		SAME		
10		6	SS	24"	0"	12'0"	16 6	15 11			compact moist		no recovery		
						120	12	19			compact		ine receiving		
15															
		7	SS	24"	17"	17'0"	18	19			wet		Brn SILT & FMC	C SAND, lit F gravel, tr C gravel	
							28	23			dense				
20												20'0"			
		8	SS	11"	9"	20'11"	45	60/5"			v dense		Brn highly to pa	rtially weathered BEDROCK	
25						0 FI FI	00/=					0.51-11			
		9	SS	5"	5"	25'5"	60/5"				v dense	25'5"	SAME	E.O.B. 25'5"	
													·	0.0. 23 3	
30															
						-									
35															
40															
	cond	ditio	ns at	t spe	cific	locatio	ons ar	nd may			epresent ent	······································			
GR	CONC OUND SUF					cations		nes.			THEN	CAS		FT. HOLE NO.	B-4
A =	AUGER (	UP =		STUR	BED F	PISTON		T = THI	NWALL	. '	V = VANE TE				
	R = WEIGH = SPLIT TL					WOH = V H.S.A. =					S			COARSE = MEDIUM	
											20 - 35% AN	ND =35 - 50			

90 DONOVAN RD.         PROJECT NO.         G287-0245-15         HOLE NO.         B-5           CT (203) 265-9328         PROJECT NOME         543-557 Elleworth Street         ppr Plan         ppr Plan           EDB/as         IOCATION         Bridgeport, CT         Participation         ppr Plan         ppr Plan           INSPECTOR         TYPE         HSAS         DATE ETART         11/2/16           INSPECTOR         TYPE         HSAS         DATE ETART         11/2/16           ILI_T FT AFTER_ HOURS         HAMMER FALL         30°         DATE ETART         11/2/16           ATFT AFTER_ ROURS         HAMMER FALL         30°         DATE ETART         11/2/16           ILI_T FT AFTER_ HOURS         HAMMER FALL         OR         CORSIT         GROUND WATER ELEV.         30°           IN Type PEN REC         BLOWS PER 8 IN CORE         CORSIT         CORSIT         DEPTH         INR DCK, ETC.         INR DCK, ETC.           IN Type PEN REC         BLOWS PER 8 IN CORE         CORSIT         CORSIT         DEPTH         INR DCK, ETC.         INR DCK, ETC.           IN Type PEN REC         BLOWS PER 8 IN CORE         CORSIT         CORSIT         DEPTH SAND, TOCHAR, ESC.         INR DCK, ETC.           IN A SAS 57 07         SSS 5         S		SOI					<b>)</b> .	CLIEN	۹L:	Jaca	bacci	Constructi	on Associ	iation Inc	SHEET_1_0	
CT (203) 262-8028         PPOLICIT NAME         Softworth Street         BORING LOCATIONS           DD/gas         COCATION         Bridgeport, CT								PRO.J	ECT N	<u>ר</u>	G267	7-0245-15			HOLE NO.	B-5
NY (914) 946-8850         543-557 Elisevont Streat         per Plan           BD/das				•							0201	0240 10			BORING LOCATIONS	
BD/jas         CASING SAMPLER         CORE BAR         OFFSET           INSPECTOR         TYPE         CASING SAMPLER         CORE BAR         OFFSET           GRUNDOWITER OWSERVITIONS         AT_T ATTER_D_HOURS         HSA         SS         DATE START         11/2/15           AT_T ATTER_D_HOURS         HAMMER VIT.         13/8"         DATE START         DATE START         11/2/15           CASING SAMPLER         SAMPLER         MAMMER VIT.         13/8"         DATE START         START           COSING IN TYPE PONEC         SAMPLER         CONSIST         STRATA         FIELD IDENTIFICATION OF SOIL REMARKS           COSING IN TYPE PONEC         BE ST         COS ON TO ESP (IN CORE)         CORE CASING INCC.         COS ON COSING INCC.         COS ON COSING INCC.         DEFTH         INCOX FER (IN CORE)         CORE CASING INCC.         COS ON COSING INCC.         DEFTH         INCOX FER (IN CORE)         COSINGT         INCOX FER (IN CORE)			•											ət		
INSPECTOR     CASNO SAMPLER     CASNO SAMPLER     CORE START 17/12/15       GROUND WATER DISERVATIONS AT_DT /FT ATTER_DOURS     TYPE     HSA     SS     DATE START 17/12/15       AT_DT /FT ATTER_DOURS     HAMMER WIT.     140/6     BIT     SUPACE LEV.       AT_DT /FT ATTER_DOURS     FXAMPLE     FXAMPLE     DATE START 17/12/15       BLOWS NO     Type PEN NEC     FXAMPLER     FXAMPLER       BLOWS NO     Type PEN NEC     CONSTRUCTION OF SOIL REMARKS.       DENSITY     STRATA     FIELD IDENTIFICATION OF SOIL REMARKS.       DENSITY     STRATA     FIELD IDENTIFICATION OF SOIL REMARKS.       DENSITY     STRATA     FIELD IDENTIFICATION OF SOIL REMARKS.       DENSITY     TOP 0     0     1       SAMPLER     DENSITY     STRATA       DENSITY     TOP 0     0     1       SAMPLER     DENSITY     STRATA       DENSITY     TOP 0     0     0       SAMPLER     DENSITY     STRATA       DENSITY     STRATA     FIELD IDENTIFICATION OF SOIL REMARKS.       DENSITY     STRATA     DITESTART       SAMPLER     DENSITY     STRATA       SAMPLER     DITESTART     STRATA       SAMPLER     SAMPLER     DITESTART       SAMPLER     SAMPLER	FO		DRILI	LER				LOCA	TION		Brid	geport, C1	Г			
SROUND WATER OBSERVATIONS ATTT ATTERHOURS         TYPE SIZE LD SIZE LD S		Enner										CASING	SAMPLER		OFESET	
SRUEND WATER DRSERVATIONS AT_FT_AFTER_LHOURS         SIZE LD. HAMMER WT. HAMMER WT. HAMER WT. HAMER WT. HAMMER WT. HAMMER WT. HAMMER WT. HAMMER WT. HAMMER WT. HAMMER WT. HAMER WT.		Loron							TYPE					CORE BAR		1/12/16
AT_FT_ATTER_HOURS         HAMMER FALL         30"         GROUND WATER ELEV.           Image: CASING Solution of the second state	GF		ATER	OBS	ERVA	TION	S									
E         SAMPLE         BLOWS PER 0         CORE         DENSITY         STRATA OR CANNE         FIELD IDENTIFICATION OF SOIL REMARKS OR CONSIDE PER 1         DENSITY (CROCE ON TUBE) FER MOIST         STRATA CONSIDE CANNE         FIELD IDENTIFICATION OF SOIL REMARKS COUNT           0         1         5         2         8         20°         1         2         1         10°°         10°	1					RS			HAMN	IER WI	г.			BIT	SURFACE ELEV.	
E         CASING         True         ELOWS PER 6 IN ON SAMPLER (N SAMPLER (PORT)         STRATA OR (N SAMPLER (PORT)         FIELD IDENTIFICATION OF SOIL REMARKS (N SAWPLER, SEAMS (N STRATA (N SAWPLER)           1         8         24         20"         1         2         100"           1         8         24         20"         1         2         100"         100 ST           1         8         24         20"         1         1         2         100"         100 ST         100 ST           1         3         55         0"         40"         16         16         40" <td>AT.</td> <td>FT_AF</td> <td>TER_</td> <td>НО</td> <td>URS</td> <td></td> <td></td> <td></td> <td>HAMN</td> <td>IER FA</td> <td></td> <td></td> <td>30"</td> <td></td> <td>GROUND WATER ELEV.</td> <td></td>	AT.	FT_AF	TER_	НО	URS				HAMN	IER FA			30"		GROUND WATER ELEV.	
E         CASING BLOWS         VD         Type         REC         DEPTI (PCRE OF 1 2 12 18 (MS) (D 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 1 12 - 18 (MS) (D 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0				<u>ب</u>	SAM	PLE	1				+					
B         BLOWS         NO         Type (PEN)         REC         FORCE ON TUBE (PER)         PER         CORNIST         DEPTH         IN ROCK, ETC.           1         1         1         1         2         2         3         model         TOPSOL         20"         1         TOPSOL         TOPSOL         20"		CASING														
FOOT         I	EPT	BLOWS	NO	Туре	PEN	REC						1				,
I         Iss         24"         8"         20"         1         2         3         model         TOPSOL           2         3s         24"         12"         16         17         17         17         17         17         17         17         17         16         17         17         17         17         17         17         17         17         18         17         18         17         18         100"         10"         10"         10"         10"         10"         10"         10"         10"         10"         10"         10"         10"         10"         10"         1												MOIST	FLEV			
2         2         5         2         4         10         7         15         moist           5         3         3         5         0         4'5'         50/5'         4'5'         60/5'         4'5'         50/5'         4'5'         60/5'         4'5'         60/5'         4'5'         50/5'         4'5'         60/5'         4'5'         60/5'         4'5'         50/5'         4'5'         60/5'         4'5'         60/5'         4'5'         50/5'         4'5'         60/5'         4'5'         60/5'         4'5'         50/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         60/5'         4'5'         6'5'         4'5'         6'5'			1	SS	24"	8"			2		(			TOPSOIL		· , · · · · · · · · · · · · · · · · · ·
5         3         85         5'         0'         4'5'         500'         dense widense widense         dense widense widense         BOULDER & COBBLES from 4 - 7'           10         4         45         24'         10'         10         12         wolst         vidense           10         4         45         24'         10'         10         12         vident           10         5         55         24''         12'         120''         vident         vident           16         6         55         24''         12'         10''         vident         vident           20         7         55         24''         10''         10''         vident         wolt           20         7         55         24''         12''         28''         30''         wet         SAME           20         7         55         24''         2''         28''         30''         wet         SAME           30         8         8''         27''         30''         32''         wet         Nard         SAME           30         8         8''         10''         10''         10''''         10'''''					0.4"	(0)						-	2'0"			······································
5       3       85       5'       0'       4'5'       50/5'       moist         10       4       35       24'       10'       10       12       moist         10       5       55       24'       12'       120'       14       15         15       5       52       24'       12'       120'       15       17       visit         16       6       55       24'       18'       170''       20       18       moist         16       6       55       24''       18''       170''       20       18       moist         16       6       55       24''       12''       20       18       moist       moist         16       6       55       24''       4''       20'''       28       30       wet         20       7       55       24''       4''''       20'''''       30'''''       32''''''         20       7       55       24''''''''''''''''''''''''''''''''''''			2	SS	24"	12"	4'0"			<u> </u>		-		1		
4       ss       24"       10"       10       12       molet       v stiff         5       ss       24"       12"       11       12       v stiff       v stiff         15       5       ss       24"       12"       12"       14       15       molet       v stiff         16       6       ss       24"       18"       17"       v       v stiff         16       6       ss       24"       18"       17"       v       v stiff         16       6       ss       24"       18"       17"       v       v stiff         20       7       ss       24"       14"       20"       18       molet       stiff         20       7       ss       24"       4"       220"       28       30       wet       hard         21       6       ss       24"       3"       270"       30       32       wet         30       8       ss       32"       27"       nard       270"       staff         31       10       10       10       10       10       10       10       10         32       10 <td>5</td> <td></td> <td>3</td> <td>SS</td> <td>5"</td> <td>0"</td> <td>4'5"</td> <td></td> <td>10</td> <td></td> <td></td> <td></td> <td></td> <td>DOOLDEINGO</td> <td></td> <td></td>	5		3	SS	5"	0"	4'5"		10					DOOLDEINGO		
4       ss       24"       10"       10       12       molet       v stiff         5       ss       24"       12"       11       12       v stiff       v stiff         15       5       ss       24"       12"       12"       14       15       molet       v stiff         16       6       ss       24"       18"       17"       v       v stiff         16       6       ss       24"       18"       17"       v       v stiff         16       6       ss       24"       18"       17"       v       v stiff         20       7       ss       24"       14"       20"       18       molet       stiff         20       7       ss       24"       4"       220"       28       30       wet       hard         21       6       ss       24"       3"       270"       30       32       wet         30       8       ss       32"       27"       nard       270"       staff         31       10       10       10       10       10       10       10       10         32       10 <td></td> <td></td> <td>-</td> <td></td>			-													
4       ss       24"       10"       10       12       molet       v stiff         5       ss       24"       12"       11       12       v stiff       v stiff         15       5       ss       24"       12"       12"       14       15       molet       v stiff         16       6       ss       24"       18"       17"       v       v stiff         16       6       ss       24"       18"       17"       v       v stiff         16       6       ss       24"       18"       17"       v       v stiff         20       7       ss       24"       14"       20"       18       molet       stiff         20       7       ss       24"       4"       220"       28       30       wet       hard         21       6       ss       24"       3"       270"       30       32       wet         30       8       ss       32"       27"       nard       270"       staff         31       10       10       10       10       10       10       10       10         32       10 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td><u> </u></td> <td><u> </u></td> <td>-</td> <td>10'0"</td> <td></td> <td></td> <td></td>										<u> </u>	<u> </u>	-	10'0"			
10       5       ss       24"       12"       12"       12"       15"       moist       moist       moist       stiff       stiff <t< td=""><td></td><td colspan="12"></td><td>Gry SILT, sm F</td><td>M SAND, tr F gravel</td><td>• .</td></t<>														Gry SILT, sm F	M SAND, tr F gravel	• .
15       15       17       v stiff         15       6       ss       24"       18"       moist         16       6       ss       24"       18"       moist         20       7       ss       24"       4"       22"       23         20       7       ss       24"       4"       22"       23         20       7       ss       24"       4"       22"       23         20       7       ss       24"       4"       22"       28       30         21       1       35       39       wet       hard       SAME         25       28       27"       4       270"       SAME         36       38       32       wet       hard       270"         36       36       32       wet       hard       270"         36       36       36       36       36       36       36         36       36       36       36       36       36       36         36       36       36       36       36       36       36         37       38       39       16       16 </td <td>10</td> <td></td>	10															
15       6       ss       24"       18"       170"       20       18       10			5	SS	24"	12"	12'0"									
8       ss       24"       18"       moist hard         20       7       ss       24"       4"       220"       28       30       wet hard       SAME         20       7       ss       24"       4"       220"       28       30       wet hard       SAME         20       7       ss       24"       4"       220"       28       30       wet hard       SAME         25       2       28       27       hard       270"       E.O.B. 270"       SAME         30       28       27       14"								10	17			v Sun				
8       ss       24"       18"       moist hard         20       7       ss       24"       4"       220"       28       30       wet hard       SAME         20       7       ss       24"       4"       220"       28       30       wet hard       SAME         20       7       ss       24"       4"       220"       28       30       wet hard       SAME         25       2       28       27       hard       270"       E.O.B. 270"       SAME         30       28       27       14"																
20       7       ss       24"       4"       220"       28       30       wet hard       sAME         20       7       ss       24"       4"       220"       28       30       wet hard       sAME         20       8       ss       24"       4"       220"       28       30       wet hard       sAME         25       9       9       9       9       9       SAME       sAME         30       1       1       1       1       1       1       1       1         30       1 <t< td=""><td>15</td><td></td><td>6</td><td></td><td>24"</td><td>10"</td><td>17'0"</td><td>20</td><td>19</td><td></td><td></td><td>moint</td><td></td><td></td><td>Maand Elaraval liteshhles</td><td></td></t<>	15		6		24"	10"	17'0"	20	19			moint			Maand Elaraval liteshhles	
7       ss       24"       4"       220"       28       30       wet hard       SAME         25       8       ss       24"       30       9       SAME       SAME         25       8       ss       24"       30       9       SAME       SAME         30       28       27       9       8       Ss       24"       30       28       27       SAME         30       28       27       9       8       Ss       24"       30       28       27       SAME         30       28       27       9       8       Ss       24"       28       27       SAME         30       28       27       9       8       Ss       24"       28       27       SAME         30       28       27       9       8       SS       24"       10				- 55	24	10	170							Ory OLT, SHIF	ivi sanu, r gravel, ili cobbles	
7       ss       24"       4"       220"       28       30       wet hard       SAME         25       8       ss       24"       30       9       SAME       SAME         25       8       ss       24"       30       9       SAME       SAME         30       28       27       9       8       Ss       24"       30       28       27       SAME         30       28       27       9       8       Ss       24"       30       28       27       SAME         30       28       27       9       8       Ss       24"       28       27       SAME         30       28       27       9       8       Ss       24"       28       27       SAME         30       28       27       9       8       SS       24"       10																
7       ss       24"       4"       220"       28       30       wet hard       SAME         25       8       ss       24"       30       9       SAME       SAME         25       8       ss       24"       30       9       SAME       SAME         30       28       27       9       8       Ss       24"       30       28       27       SAME         30       28       27       9       8       Ss       24"       30       28       27       SAME         30       28       27       9       8       Ss       24"       28       27       SAME         30       28       27       9       8       Ss       24"       28       27       SAME         30       28       27       9       8       SS       24"       10	20															
25       36       39       hard       hard         25       28       27       hard       270"         30       28       27       hard       270"         31       28       27       hard       270"         32       28       27       100       100         33       28       27       100       100         34       28       27       100       100         35       28       100       100       100         40       100       100       100       100         35       100       100       100       100         36       100       100       100       100 <tr< td=""><td>20</td><td></td><td>7</td><td>SS</td><td>24"</td><td>4"</td><td>22'0"</td><td>28</td><td>30</td><td></td><td></td><td>wet</td><td></td><td>SAME</td><td></td><td></td></tr<>	20		7	SS	24"	4"	22'0"	28	30			wet		SAME		
8         ss         24"         3"         270"         30         32         wet hard         SAME           30         28         27         1         1         270"         E.O.B. 27'0"           30         30         32         1         1         1         1         1           30         30         32         1         1         1         1         1           30         1         1         1         1         1         1         1         1           30         1 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>35</td> <td></td> <td></td> <td></td> <td>hard</td> <td></td> <td></td> <td></td> <td></td>								35				hard				
8         ss         24"         3"         270"         30         32         wet hard         SAME           30         28         27         1         1         270"         E.O.B. 27'0"           30         30         32         1         1         1         1         1           30         30         32         1         1         1         1         1           30         1         1         1         1         1         1         1         1           30         1 </td <td></td> <td>•</td> <td></td> <td></td> <td></td> <td></td>												•				
30       28       27       hard       270"         30       8       8       8       9       1       <	25															
30       E.O.B. 27'0"         31       E.O.B. 27'0"         35       E.O.B. 27'0"         36       E.O.B. 27'0"         37       E.O.B. 27'0"         38       E.O.B. 27'0"         39       E.O.B. 27'0"         30       E.O.B. 27'0"         31       E.O.B. 27'0"         32       E.O.B. 27'0"         33       E.O.B. 27'0"         34       E.O.B. 27'0"         35       E.O.B. 27'0"         36       E.O.B. 27'0"         37       E.O.B. 27'0"         38       E.O.B. 27'0"         39       E.O.B. 27'0"         30       E.O.B. 27'0"         31       E.O.B. 27'0"         32       E.O.B. 27'0"         33       E.O.B. 27'0"         40       E.O.B. 27'0"         40       E.O.B. 20         NOTE: Subsoil conditions revealed by this investigation represent conditions at other locations or times.         GROUND SURFACE TO       FT.         USED       CASING THEN         CASING TO       FT.         HOLE NO.       B-5         A LIGER UP = UNDISTURBED PISTON       T = THINWALL         V = VANE TEST <t< td=""><td></td><td></td><td>8</td><td>SS</td><td>24"</td><td>3"</td><td>27'0"</td><td></td><td></td><td></td><td></td><td>wet</td><td></td><td>SAME</td><td></td><td></td></t<>			8	SS	24"	3"	27'0"					wet		SAME		
30       30 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>28</td><td>27</td><td></td><td></td><td>hard</td><td>27'0"</td><td></td><td></td><td></td></td<>								28	27			hard	27'0"			
35       36       37         40       40       40         NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at specific locations and may not represent conditions at other locations or times.       6         GROUND SURFACE TOFT.       USEDCASING THENCASING TOFT.       HOLE NO.         BROUND SURFACE TO FRODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM <td></td>																
40 NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM	30															
40 NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM			]					[								
40 NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM																
40 NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times. GROUND SURFACE TOFT. USEDCASING THENCASING TOFT. HOLE NO. B-5 A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM																
NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.         GROUND SURFACE TOFT.       FT.       USEDCASING THENCASING TOFT.       HOLE NO.       B-5         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM	35															
NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.         GROUND SURFACE TOFT.       FT.       USEDCASING THENCASING TOFT.       HOLE NO.       B-5         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM													-			
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NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent conditions at other locations or times.         GROUND SURFACE TOFT.       FT.       USEDCASING THENCASING TOFT.       HOLE NO.       B-5         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM				]						[						
conditions at specific locations and may not represent         conditions at other locations or times.         GROUND SURFACE TOFT.       USEDCASING       THENCASING TOFT.       HOLE NO.       B-5         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM		75. 0		00-	diti a	nc				oction	ation	oprocont	1			
conditions at other locations or times.         GROUND SURFACE TOFT.       USEDCASING THENCASING TOFT.       HOLE NO.       B-5         A = AUGER       UP = UNDISTURBED PISTON       T = THINWALL       V = VANE TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM	NU															
A = AUGER       UP = UNDISTURBED       PISTON       T = THINWALL       V = VANE       TEST         WOR = WEIGHT OF RODS       WOH = WEIGHT OF HAMMER & RODS       C = COARSE         SS = SPLIT TUBE SAMPLER       H.S.A. = HOLLOW STEM AUGER       M = MEDIUM	0.5	con	ditio	ns at	t oth	er lo	cations	s or tin		-	-					<b>B</b> 5
WOR = WEIGHT OF RODSWOH = WEIGHT OF HAMMER & RODSC = COARSESS = SPLIT TUBE SAMPLERH.S.A. = HOLLOW STEM AUGERM = MEDIUM									T = TH							
SS = SPLIT TUBE SAMPLER H.S.A. = HOLLOW STEM AUGER M = MEDIUM PROPORTIONS USED: TRACE = 0 - 10% LITTLE = 10 - 20% SOME = 20 - 35% AND =35 - 50% F = FINE	wo	R = WEIGI	HT OF	= ROE	S		WOH = V	VEIGHT	of H/	MMER	& ROD	S				
	SS = PRC	= SPLIT TI PORTION	IS US JRE S	SAMP	LER TRAC	E = 0						20 - 35% AI	ND =35 - 50			

	SOI					С.	CLIEN	IT:	Jaca	bacci (	Constructi	on Assoc	iation Inc	SHEET <u>1</u> HOLE NO,	
			RD, C				PROJ	ECT N	D.	G267	7-0245-15		<u> </u>	HOLE NO.	B-6
			) 3) 2					ECT N/				-		BORING LOCATIONS	
	- With od		14) 9	46-4	850						557 Eliswo		et	per Plan	
FC	REMAN - BD/ad/		LER				LOCA	TION		Bridg	geport, Cl	Г			
INS	SPECTOR						1				CASING	SAMPLER	CORE BAR	OFFSET	
								TYPE			HSA	SS	NWD4	DATE START	1/8/16
GF		ATER	OBS	ERVA	TION	S '	1	SIZE	I.D.		4 1⁄4"	1 3/8"	21⁄8"	DATE FINISH	1/12/16
	none_FT				URS			HAM	/IER W	г.		140#	BIT	SURFACE ELEV.	
AT.	FT_AF	TER_						HAMN	/IER FA	LL		30"	dia	GROUND WATER ELEV.	•
			т <u>;</u>	SAM		1	_								
DEPTH	CASING BLOWS PER		Туре	PEN	REC	DEPTH	ON (FOR			CORE TIME PER FT	DENSITY OR CONSIST	STRATA CHANGE DEPTH		ENTIFICATION OF SO DR, LOSS OF WASH W IN ROCK, ETC.	
	FOOT		ļ			@ BOT			12- 10	(MIN)	MOIST	ELEV			
		1	SS	24"	8"	2'0"	1	2			dry/moist soft	1'6"	TOPSOIL		
1		2	SS	24"	18"	4'0"	8	24			dry/moist	2'6"	Brn SILT, sm F	sand, lit cobbles ( poss fill )	
				1"	4.	1125	30	40			v dense			sm silt, sm cobbles	
5		3	SS C	60"	1" 24"	4'1" 12'0"	50/1" RQD =	17%			v dense	<u>5'0"</u> 7'0"	partially decom	posed BEDROCK	
					21	120	TIGE	17.70					BEDROCK ( So		-
10															
10															
												12'0"			
			<u> </u>					-						E.O.B. 12'0"	
15					<u></u>							i			
20															
25															
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40										ľ					
NO	cond	ditio	ns af	t spe	cific	locatio	ons an	d may			epresent ent	L			
GRC	UND SUF					cations	s or tin SED	ies.		ASING	THEN			FT. HOLE NO.	B-6
	AUGER (							r = THI	NWALL		/ = VANE TE				0-0
											S				
						H.S.A. =					0-35% AN	1D 25 50		1 = MEDIUM = FINE	

	SOI		STI	-	-	C.	CLIEN	NT:	Jaca	bacci (	Constructi	on Associ	ation Inc	SHEET_1_C HOLE NO.	
			RD, C				PROI	ECT N	0	G267	-0245-15			HOLL NO.	B-7
			(2), C (3) 2(					ECT N		0207	-02-40-10			BORING LOCATIONS	
		•	(4) 9/				F NOS	LOTIN		543-5	557 Ellswe	orth Stree	et	per Plan	
FO	REMAN -						LOCA	TION			geport, Cl				
	BD/jas														
INS	PECTOR										CASING	SAMPLER	CORE BAR	OFFSET	
								TYPE			HSA	SS		DATE START	1/13/16
						S		SIZE			4 ¼"	1 3/8"	and a local	DATE FINISH	1/13/16
	<u>18</u> FT				RS							140# 30"	BIT	SURFACE ELEV.	
<u> </u>	FTAF							HAW	IER FA		. · · · ·			GROUND WATER ELEV.	
		<u> </u>	1	SAM	PLE		-								
	CASING							WS PE		CORE	DENSITY OR	STRATA CHANGE	1	ENTIFICATION OF SOIL DR, LOSS OF WASH WA	
DEPTH	BLOWS		Туре	PEN	REC				'LER TUBE)	TIME PER	CONSIST	DEPTH		IN ROCK, ETC.	
ä	PER					DEPTH	0-6	6 - 12		FT					
	FOOT	1		24"	14"	@ BOT 2'0"	1	1 0	T	(MIN)	MOIST	ELEV	TOPPOIL		
		<u> </u>	SS	24	14	20	2	2			moist v loose	1'6"	TOPSOIL		
1		2	ss	24"	12"	4'0"	7	6	+		moist		Brn FM SAND,	lit silt, gravel ( poss fill )	
							5	7			compact	4'0"			
5		3	SS	24"	12"	6'0"	15	17			moist		Brn SILT & FM	SAND, sm cobbles, gravel	
		4	SS	24"	12"	8'0"	17 22	18 24			hard moist			M sand, F gravel	
		<del>4</del>	55	24	12	00	27	24			hard		DIII OLT, SHIF	w sand, F gravel	
		5	SS	24"	16"	10'0"	9	11			moist				
10							12	12			v stiff				
		6	SS	24"	14"	12'0"	14	12	ļ		moist		SAME		
							11	13			v stiff				
15															
		7	SS	24"	18"	17'0"	20	22			moist		Gry SILT, sm F	M sand, tr cobbles	
							24	24			hard				
20															
		8	SS	22"	18"	22'0"	12	15			wet		SAME		
							18	50/4"			hard				
25															
		9	SS	24"	18"	27'0"	11	16			wet				
							14	14			v stiff				
30															
		10	SS	24"	16"	32'0"	15	15			wet		SAME		
							17	15			hard	32'0"			
													l	E.O.B. 32'0"	
35													·		
35															
[															
												ĺ			
40				-1:4" -			J Barr 47			4:					
NO						evealec : locatio					epresent				
	con	ditio	ns at	t oth	er lo	cations			,	50103					
	UND SU	RFAC	E TO		F	T. US	SED			CASING			SING TO	FT. HOLE NO.	B-7
	AUGER R = WEIG					PISTON WOH = V					V = VANE TI S	EST	r	C = COARSE	
	s SPLIT TI					H.S.A. =					-			I = MEDIUM	
						- 10%	ITTI E :	= 10 - 2	0% S	DMF = 2	20 - 35% AN	ND =35 - 50	% F	= FINE	

	SOI					<b>)</b> .	CLIEN	NT:	Jaca	bacci (	Constructio	on Associ	iation Inc	SHEET 1_OF_1	
			NOV RD, C							<u> </u>	0045 45			HOLE NO.	B-8
			(D, C )3) 2(				-			G26/	-0245-15			BORING LOCATIONS	
		•	(4) 94				FICU		-	543-5	57 Ellswo	orth Stree	et	per Plan	
FC	REMAN -					-	LOCA	TION			jeport, Cl				
	BD/jas SPECTOR											0.1.101 50			
	SPECTOR							TYPE			CASING HSA	SAMPLER SS	CORE BAR	OFFSET DATE START 1/14	/16
GF		ATER	OBSE	ERVA	TION	S	1	SIZE			4 1/4"	1 3/8"		DATE FINISH 1/14	
	<u>18_</u> FT								IER W	г.		140#	BIT	SURFACE ELEV.	
AT	FTAF	TER_	_но	URS			<u> </u>	HAMN	IER FA	LL		30"	·	GROUND WATER ELEV.	
	1			SAM	PLE	1									
	CASING							WS PE		CORE	DENSITY OR	STRATA CHANGE		ENTIFICATION OF SOIL REMA DR, LOSS OF WASH WATER, S	
DEPTH	BLOWS	NO	Туре	PEN	REC			SAMP		TIME	CONSIST	DEPTH		IN ROCK, ETC.	
	PER FOOT					DEPTH @ BOT		6 - 12		FT (MIN)	MOIST	ELEV			
		1	SS	24"	6"	2'0"	2	3			moist	1'0"	TOPSOIL		-
						(10)	2	2			stiff		Red Brn SILT,	sm FM sand, tr F gravel ( subsoil )	
			SS	24"	14"	4'0"	7	9 18			moist v stiff	3'6"			
5		3	SS	24"	16"	6'0"	12	15			moist	5'0"	Brn FMC SANE	D, sm silt, F gravel	
					1.01		24	30			dense		Brn SILT, sm F	sand, tr C sand, F gravel	
		4	SS	24"	16"	8'0"	26 20	24 22			moist hard				
		5	SS	24"	16"	10'0"	15	14			moist				
10							14	12			v stiff				
		6	SS	24"	18"	12'0"	14 16	16 17			moist hard				
							10	17			naiu				
15		7		24"	16"	17'0"	14	12			moist			sand, tr F gravel, cobbles	
			SS	24	10	170	14	12			v stiff		GIY OLT, SHIF	sand, il Figlavel, cobbles	
1 - 1 - 3											1				
20															
20		8	SS	24"	14"	22'0"	25	26			wet		SAME		
							30	31			hard				
25										· · · · ·					
		9	SS	24"	12"	27'0"	27	28			wet		SAME; sm weat	hered bedrock frags	
							33	33			hard	27'0"		E.O.B. 27'0"	
													•		
30															
														·	
35															
									·						
40															
Į I	TE: Sub	soil	con	ditio	ns re	evealed	bv th	is inv	estia	tion r	epresent				—
	con	ditio	ns af	t spe	cific	locatio	ons an	d may							
GPO	CON DUND SUF					cations	ED			CASING	THEN	CA	SING TO	FT. HOLE NO. B.	-8
A =	AUGER	iP =		STUR	BED F	PISTON		T = THI	NWALL	N	/ = VANE TE				
WO	R = WEIGI = SPLIT TU			)S EP	1	WOH = V H.S.A. =					S			C = COARSE // = MEDIUM	
											0-35% AN	ND =35 - 50		F = FINE	

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	SOI		STI NOV		,	D.	CLIE	NT:	Jaca	bacci (	Constructi	on Associ	ation Inc	SHEET <u>1</u> 0 HOLE NO.	
			RD, C				PRO.	ECT N	0	G267	-0245-15	·····			B-9
			)3) 26					ECT N					·····	BORING LOCATIONS	
		•	4) 94								557 Ellswe		et	per Plan	
FO	REMAN -	DRIL	LER				LOCA	TION		Bridg	geport, Cl	F ·			
	TP/ad SPECTOR										CASING	SAMPLER	CORE BAR	OFFSET	
	DIEGION							TYPE			HSA	SAMPLER	CORE DAR	DATE START	1/15/16
GR		ATER	OBSE	ERVA	TION	S	1	SIZE			4 1/4"	1 3/8"		DATE FINISH	1/15/16
AT	<u>15_</u> FT	AFTE	R_0_I	HOUF	RS			HAM	IER W	г.		140#	BIT	SURFACE ELEV.	
AT.	<u>9</u> FT A	FTEF	<u>2</u> ⊦	IOUR	S		ļ	HAM	/IER FA	LL		30"	N.07	GROUND WATER ELEV.	
	<u> </u>			SAM	PLE								1	<del>///</del>	
DEPTH	CASING BLOWS PER	NO	Туре	PEN	REC	DEPTH	ON (FOR	WS PE I SAMF CE ON 6 - 12	LER TUBE)	CORE TIME PER FT	DENSITY OR CONSIST	STRATA CHANGE DEPTH		ENTIFICATION OF SOIL DR, LOSS OF WASH WA IN ROCK, ETC.	
	FOOT	1		24"	17"	@ BOT 2'0"	6	1 6	<u></u>	(MIN)	MOIST	ELEV			
		- 1 -	SS	24		20	6	4			dry stiff		1.5" ASPHALT Brn SILT, sm F	gravel, lit brick, ash, FM sand	(fill)
		2	SS	24"	19"	4'0"	2	3			dry			Γ, sm FM sand, tr F gravel, root	
5		3	SS	24"	12"	6'0"	4	6 6		<u> </u>	stiff	4'6"		C SAND, tr F gravel	
		5	55	24	12	00	9	11			dry stiff			C SAND, Ir F gravel	
		4	SS	24"	6"	8'0"	14	17			dry		SAME; tr C gra		
		-		0.4	4.01	4.0101	24	21			hard				
10		5	SS	24"	18"	10'0"	8	9 14			dry v stiff		BUN SILI & FMI	C SAND, lit F gravel	
		6	SS	24"	16"	12'0"	15	11			dry		SAME		
							13	14			v stiff				
15					•										
		7	SS	24"	22"	17'0"	15	12			moist		Brn SILT & VF-I	F to M SAND, tr F gravel	
							13	14			v stiff				
		••••••													
20				( 0		0014.0		0.014			wet	20'0"			
		8	SS	10"	10"	20'10"	55	60/4"			hard		highly to partial	y weathered BEDROCK	
												24'0"	AUGER REFUS		
25														E.O.B. 24'0"	
														1" SCH 40 PVC Observatior	
30													10' scr	een to 15'. Set curb box at s	urface.
30															
35							·								
40															
NO	con	ditio	ns at	t spe	cific	locatio	ons ar	id mag			epresent ent				
GRC	CON DUND SUF					cations	s or tir Sed	nes.		CASING	THEN_	CA		FT. HOLE NO.	B-9
A = ,	AUGER	JP =	UNDIS	STUR	BED F	PISTON			NWALL	- `	V = VANE TI				
	R = WEIGI • SPLIT TI					WOH = \ H.S.A. =					S			C = COARSE // = MEDIUM	
											20 - 35% AM	ND =35 - 50		= FINE	

	SOI				-	С.	CLIEN	NT:	Jaca	bacci (	Constructi	on Associ	iation Inc	SHEET_1_OI	
			NOV RD, C				PROI	ECT N		G267	-0245-15			HOLE NO.	B-10
			03) 20					ECT N		0207	-0245-15		·····	BORING LOCATIONS	
		•	14) 9					20110		543-	557 Ellswo	orth Stree	et	per Plan	
FC	REMAN -		LER				LOCA	TION		Bridg	geport, C	Г			· · · · · · · · · · · · · · · · · · ·
	BD/jas SPECTOR										0.4.6%10	0.110.50		055057	
	SPECIUR							TYPE			CASING HSA	SAMPLER SS	CORE BAR	OFFSET DATE START	1/14/16
GF		ATER	OBSI	ERVA		S	-	SIZE			4 1/4"	1 3/8"		DATE FINISH	1/14/16
ł	<u>20</u> FT								/IER W	г.		140#	BIT	SURFACE ELEV.	
AT	FT AF	TER	НО	URS			ļ	HAM	/IER FA	LL	•	30"		GROUND WATER ELEV.	
				SAM	PLE										
DEPTH	CASING	NO		DEN	REC	1	ON	WS PE	LER	CORE TIME	DENSITY OR	STRATA CHANGE		ENTIFICATION OF SOIL DR, LOSS OF WASH WA	
DEF	BLOWS PER FOOT		Type			DEPTH @ BOT	0-6	CE ON 6 - 12		PER FT (MIN)	MOIST	DEPTH ELEV		IN ROCK, ETC.	,
		1	SS	24"	14"	2'0"	3	2			moist	1'0"	TOPSOIL		
		<u> </u>		0.4	4.01	40	2	1			soft		Red Brn SILT,	lit FM sand ( subsoil )	
		2	SS	24"	16"	4'0"	2	2			moist soft	4'6"	1		
5		3	SS	24"	16"	6'0"	8	14			moist	40	Bm SILT, sm V	/F-F sand, tr cobbles, F gravel	
				0.41	4.45	0101	12	8			v stiff				
1		4	SS	24"	14"	8'0"	10 12	10 13			moist v stiff		ļ		
		5	SS	24"	14"	10'0"	12	14			moist		Brn SILT, sm F	sand	
10				40"	0	44100	16	16			hard				
		6	SS	12"	6"	11'0"	20	50			moist		SAME; lit cobbl	es, boulders from 11- 12'	
4-															
15		7	SS	24"	18"	17'0"	22	24			moist		Brn Sli Tem F	sand, tr cobbles	
		1	00	21		17.0	20	20			hard			Salid, il CODDIES	
20															
		8	SS	24"	18"	22'0"	12	17			wet		Brn SILT & FM S	SAND, tr cobbles	
							18	18			hard				
25												25'0"	AUGER REFUS	AL	
													ł	E.O.B. 25'0"	
30															
										-					
	-														
35															
				_											
40			-+												
	TE: Sub	soil	con	ditio	ns re	vealed	bv th	is inv	estia	tion r	epresent			- 1100 - 11 - 1	
	con	ditio	ns at	spe	cific	locatio	ons an	d may							
GRO	CONC COND SUF					cations	s or tin ED	nes.			THEN	CA4		FT. HOLE NO.	B-10
	AUGER		-					T = THI	NWALL		/ = VANE TE				01-0
						NOH = V					S				
	SPLIT TU					H.S.A. =					0-35% AN	ND =35 - 50'		1 = MEDIUM F = FINE	

	SOI		STI NOV			. م	CLIEN	NT:	Jaca	bacci (	Constructio	on Associ	ation Inc	SHEET <u>1</u> OF <u>1</u> HOLE NO. B-	
			RD, C				PRO	ECT N	D.	G267	-0245-15	******			11
			)3) 2(					ECT N/			0240 10			BORING LOCATIONS	
			4) 9							<u>54</u> 3-5	57 Ellswo	orth Stree	et	per Plan	
0	REMAN -	DRILI	LER				LOCA	TION		Bridg	geport, C1	-			
NIC	BD/jas										040000		0005 040	OFFORT	
NC	PECIOR							TYPE			CASING HSA	SAMPLER SS	CORE BAR	OFFSET DATE START 1/13/1	6
GR		ATER	OBSI	RVA	TION	s	-	SIZE			4 1/4"	1 3/8"		DATE FINISH 1/13/1	
	<u>8</u> FT A					-			IER W	Г.		140#	BIT	SURFACE ELEV.	Ĩ
<u>۲</u>	FTAF	TER_	но	URS			ļ	HAM	/IER FA	LL		30"		GROUND WATER ELEV.	
	[ .		(	SAM	PLE					1			1		
DEPTH	CASING BLOWS PER	NO	Туре	PEN	REC	DEPTH	ON (FOR	WS PE I SAMP CE ON 6 - 12	LER TUBE)	CORE TIME PER FT	DENSITY OR CONSIST	STRATA CHANGE DEPTH		ENTIFICATION OF SOIL REMAR DR, LOSS OF WASH WATER, SE IN ROCK, ETC.	
	FOOT			0.0	0	@ BOT			T	(MIN)	MOIST	ELEV	70000		
		1	SS	24"	6"	2'0"	2	2			moist stiff	1'6"	TOPSOIL Red Brn SILT	lit sand, gravel ( subsoil )	
		2	SS	24"	8"	4'0"	7	8			moist			in amin' Aintai Cannaoli I	
~				0.0	40"	0101	8	10			v stiff	3'6"		10 1 17	
5		3	SS	24"	12"	6'0"	10 9	12 12			moist v stiff		Brn SILT, sm M	IC sand, F gravel	
		4	SS	24"	14"	8'0"	13	10			moist		SAME		
		r		0.01	40"	4010	11	11			v stiff				
0		5	SS	24"	16"	10'0"	14 13	13 15			wet v stiff		BIN SIL I, SM F.	M sand, tr F gravel	
		6	SS	24"	18"	12'0"	16	16			wet				
							12	17			v stiff				
				,		1									
5															
		7	SS	24"	18"	17'0"	24	33			wet		SAME		
							34	50			hard				
1						·									
0				<u></u>	4.0"	0.010.1		00							
		8	SS	24"	16"	22'0"	20 24	22 33		·	wet hard		Brn SILT, lit cob	DIES	
•											nard				
												0.510"			
5		9	SS	1"	1"	25'1"	50/1"				hard		AUGER REFUS BEDROCK ( Sci		<u>.</u>
		U	00								, icit d		,		
											· ·				
0		1	с	60"	8"	30'0"	RQD =	7%				30'0"			
		-	<u> </u>	00	0			1 /0					I	E.O.B. 30'0"	
5															
ŀ															
οĮ											<u> </u>			· · · · · · · · · · · · · · · · · · ·	
Ő											epresent				
						location:			y not i	repres	ent				
	DUND SUF	RFAC	E TO		F	T. US	SED			CASING			SING TO	FT. HOLE NO. B-1	1
	AUGER R = WEIGI					PISTON WOH = \					V = VANE TI S	EST	ſ	C = COARSE	
	- SPLIT TU										U .			M = MEDIUM	
											20 - 35% Al	ND =35 - 50	% F	F = FINE	

	SOI		STI		-	С.	CLIEN	IT:	Jaca	bacci (	Constructi	on Associ	iation Inc	SHEET_1_OF HOLE NO.	
			RD, C				PROJ	ECT N	2	G267	-0245-15			HOLE NO.	B-12
			)3) 2(				<b></b>	ECT N/		0207	02.10.10			BORING LOCATIONS	
			4) 94								557 Ellswo		et	per Plan	
FC	REMAN -	DRIL	LER				LOCA	TION		Bridg	geport, Cl	Г			
	BD/jas										CASING	SAMPLER	CORE BAR	OFFSET	
								TYPE			HSA	SS		DATE START	1/13/16
GF	OUND W	ATER	OBSI	ERVA	TION	S	1	SIZE	I.D.		4 ¼"	1 3/8"		DATE FINISH	1/13/16
1 °	<u>18</u> FT				RS				/IER W			140#	BIT	SURFACE ELEV.	
AT	FT_AF	TER_					<u> </u>	HAMN	IER FA	LL		30"		GROUND WATER ELEV.	
		<u> </u>		SAM T	PLE T	1	4							ENTIFICATION OF SOIL F	
Ξ	CASING							NS PE		CORE	DENSITY OR	STRATA CHANGE	1	DR, LOSS OF WASH WAT	
DEPTH	BLOWS	NO	Туре	PEN	REC		1	SAMP CE ON	TUBE)	PER	CONSIST	DEPTH		IN ROCK, ETC.	
	PER FOOT					DEPTH @ BOT	0-6	6 - 12	12- 18	FT (MIN)	MOIST	ELEV			
		1	SS	24"	14"	2'0"	5	2		Ĺ	moist		Brn TOPSOIL		
		2		24"	12"	4'0"	3	3	ļ		stiff	2'0"			
		<u> </u>	SS	24	12	40	7	6			moist stiff		Brn SILT, sm F	gravei .	
5		3	SS	24"	10"	6'0"	7	8			moist		Brn SILT, sm F	MC sand, F gravel	
		4		24"	14"	8'0"	8 11	10		ļ	v stiff				
			SS	<u> </u>	14	00	9	11 12			moist v stiff				
		5	SS	24"	14"	10'0"	12	14			moist		SAME		
10		0		0.41	4.61	40101	14	17	ļ		v stiff				
		6	SS	24"	16"	12'0"	18 16	16 14			molst hard				
1=															
15		7	SS	24"	16"	17'0"	18	22			moist		SAME; lit cobble	es	
							23	20			hard				
20															
		8	SS	24"	18"	22'0"	19	19			wet		SAME		
							24	22			hard				
25															
		9	SS	24"	18"	27'0"	10	17			wet		Gry SILT, sm FN	I sand, F gravel, lit cobbles	
				-			25	34			hard				
30		10		0.41	10	20101							CAME		
		10	SS	24"	18"	32'0"	<u>28</u> 26	30 26			wet hard-	32'0"	SAME		
							_~	_~				52.0	E	E.O.B. 32'0"	
2-			·												
35															
	· · · · ·														
					]										
40			-+												
	TE: Sub	soil	con	ditio	ns re	evealed	by th	is inv	estiga	ation r	epresent				
	cone	ditio	ns af	spe	cific	locatio	ons an	d may							
GRO	CONC OUND SUF				er lo F	cations	e or tin	nes.			THEN	CAS		FT. HOLE NO.	B-12
A = .	AUGER I	JP = I	UNDIS	STUR	BED F	PISTON	-		NWALL	<u>،</u> ۱	V = VANE TE				
	R = WEIGI SPLIT TU					WOH = V H.S.A. =					S			C = COARSE / = MEDIUM	
											0-35% AN	ND =35 - 50		i = MEDIUM = FINE	

	SOI					С.	CLIEN	NT:	Jaca	bacci (	Constructi	on Associ	ation Inc	SHEET 1 0	
			NOV							C267	7-0245-15		<u></u>	HOLE NO.	B-13
			(D, C )3) 2(					ECT N		G26/	-0245-15				
			14) 94				FROJ	ECTIN		543-	557 Ellsw	orth Stree	et	BORING LOCATIONS per Plan	
FC	REMAN -						LOCA	TION			geport, C			porridit	
	BD/jas												•	· · · · · · · · · · · · · · · · · · ·	
	SPECTOR										CASING	SAMPLER	CORE BAR	OFFSET	
			OPE		TION	~	-	TYPE SIZE			HSA 4 ¼"			DATE START	1/15/16
	<u>10</u> FT					5			I.D. /IER WI	-	4 74	140#	BIT	DATE FINISH SURFACE ELEV.	1/15/16
	FT AF								/ER FA			30"		GROUND WATER ELEV.	
-	T			SAM	PLE					<u> </u>	1	I	1		· · · · · · · · · · · · · · · · · · ·
			Τ					WS PE	RAIN	CORE	DENSITY	STRATA		ENTIFICATION OF SOIL	
DEPTH	CASING BLOWS	NO	Type	PEN	REC		ON	SAMP	LER	TIME	OR	CHANGE	INCL. COLO	DR, LOSS OF WASH WA	TER, SEAMS.
	PER	NO	libbe			DEPTH	(FOR	CE ON 6 - 12	TUBE)	PER FT	CONSIST	DEPTH		IN ROCK, ETC.	
	FOOT			0.41		@ BOT		· · · · · ·	12-10	(MIN)	MOIST	ELEV	-		
		1	SS	24"	14"	2'0"	1 2	1			moist v loose	1'0"	TOPSOIL Brn VE E SANI	D & SILT ( poss fill )	
		2	SS	24"	14"	4'0"	4	5			moist				
							5	9			loose	4'0"			
5		3	SS	24"	14"	6'0"	12 15	10 15			moist compact		Brn FMC SANE	), sm silt, lit F gravel	
1		4	SS	24"	14"	8'0"	16	16			moist		-		
							14	17			compact				
10		5	SS	24"	10"	10'0"	16 20	27 15			moist		SAME		
		6	SS	.0"	0"	10'0"	20 50/0"	15			dense wet				
15													,		
'		7	SS	24"	10"	17'0"	18	16			wet				
							20	20			dense				
20												1			
		8	SS	24"	12"	22'0"	24	23			wet		SAME		
							22	18			dense				
												24'0"	AUGER RREFL	ISAL	
25														E.O.B. 24'0"	
										· · · · ·					
											[				
30						· ··· · ·									
							·								
0		]						]		]					
35															
40					-										
	TE: Sub	soil	cond	ditio	ns re	evealed	bv th	is inv	estia	tion r	epresent	1			
	cond	litio	ns at	spe	cific	locatio	ons an	d may							
GP	CONC OUND SUF					cations	ED	nes.		CASING	THEN	CA9		FT. HOLE NO.	B-13
	AUGER I				BED F	PISTON	-		NWALL	. N	V = VANE TE				
WO	R = WEIGH • SPLIT TU			S		WOH = V H.S.A. =					S			C = COARSE / = MEDIUM	
											.0 - 35% AN	ND =35 - 50		F = FINE	

	SOI 90		STI NOV			۰.	CLIEN	NT:	Jaca	bacci (	Constructio	on Assoc	ation Inc	SHEET <u>1</u> OF <u>1</u> HOLE NO. B-14
			RD, C				PROJ	ECT N	D.	G267	-0245-15			
			)3) 2(					ECT N/						BORING LOCATIONS
	N	Y (91	14) 9/	46-48	850						557 Ellswo		et	per Plan
=0	REMAN -	DRIL	LER				LOCA	TION		Bridg	jeport, Cl	Γ		
NS	BD/jas SPECTOR										CASING	SAMPLER	CORE BAR	OFFSET
								TYPE			HSA	SS	OONE DAW	DATE START 1/14/16
ЗR	OUND W	ATER	OBS	ERVA	TION	S	1	SIZE	I.D.		4 1⁄4"	1 3/8"		DATE FINISH 1/14/16
	<u>12_</u> FT				RS				IER W			140#	BIT	SURFACE ELEV.
λΤ <u>.</u>	FTAF	TER						HAM	/IER FA		-	30"		GROUND WATER ELEV.
				SAM T	PLE T	T	-							ENTIFICATION OF SOIL REMARKS
	PER	NO	Туре	PEN	REC	DEPTH	ON (FOR	WS PE I SAMP CE ON 6 - 12	LER TUBE)	CORE TIME PER FT	DENSITY OR CONSIST	CHANGE DEPTH		DR, LOSS OF WASH WATER, SEAM IN ROCK, ETC.
	FOOT	1	SS	24"	10"	@ BOT 2'0"	11	3	1	(MIN)	MOIST moist	ELEV		BUILDING RUBBLE, sm sand, silt ( fill )
		<u> </u>	- 35	1 <u>24</u>		20	2	2			loose	2'0"	DOLL DRICK &	
		2	SS	24"	12"	4'0"	7	8		Ĺ	moist		Brn FM SAND,	sm silt, tr gravel, C sand ( poss fill )
5		3	SS	24"	10"	6'0"	8	10 20	· ·		compact moist	4'0"	Brn FMC SAND	) sm F gravel
						1	22	24			dense	6'0"		
		4	SS	6"	4"	6'6*	50				moist			
		5	SS	24"	12"	10'0"	7	11			moist		Brn SII T sm F	sand, tr F gravel, cobbles
0					·		11	10			stiff		511 0121, 0111	
		6	SS	24"	14"	12'0"	12 12	10 12			moist			
								12			stiff			
15		7	SS	24"	14"	17'0"	15	17			wet			
		1	- 33	24		170	17	16			hard			
20														
		8	SS	24"	16"	22'0"	18	18			wet		SAME	
							20	17			hard			
5														
		9	SS	24"	14"	27'0"	<u>20</u> 26	<u>24</u> 30			wet hard			
							20	- 50			naru			
_														
80		10	SS	24"	14"	32'0"	27	25			wet		SAME	
ŀ		10	33	24	17	52.0	25	23			hard	32'0"		
ļ													E	E.O.B. 32'0"
5											ľ			
Ĭ														
F								]						
$\mathbf{F}$											·			
o[														
0	con	ditio	ns at	t spe	cific	evealed locations	ons an	nd mag			epresent ent			
	OUND SUF	RFAC	E TO		F	T. US	SED			CASING				FT. HOLE NO. B-14
	AUGER R = WEIGI					PISTON WOH = V					V = VANE TE S	EST	r	C = COARSE
5 =	SPLIT TU	JBE S	SAMPI	LER		H.S.A. =	HOLLO	OW STE	EM AUG	BER			Ν	I = MEDIUM
20	PORTION	is us	ED: -	TRAC	E = 0	-10% L	ITTLE :	= 10 - 2	0% SC	DME = 2	0-35% AN	VD =35 - 50	% F	= FINE

	SOI		STI NOV			C.	CLIEN	IT:	Jaca	bacci (	Constructio	on Associ	ation Inc	SHEET <u>1</u> 0 HOLE NO.	F <u>1</u> B-15
			RD, C				PROJ	ECT N	<u></u>	G267	-0245-15			HOLL NO.	D-10
			(2) (3) 20					ECT N/		0201	0240 10			BORING LOCATIONS	
			4) 9						=	543-5	557 Ellswo	orth Stree	et	per Plan	
FO	REMAN -	DRILI	LER				LOCA	TION		Bridg	geport, Cl				
	BD/ad						<b> </b>								
IINS	PECTOR										CASING HSA	SAMPLER	CORE BAR	OFFSET	1/8/16
			OPE					TYPE SIZE			<u> </u>	SS 1 3/8"		DATE START DATE FINISH	1/8/16
	<u>none</u> FT					5			I.D. /IER W	r.	4 74	140#	BIT	SURFACE ELEV.	1/0/10
1	<u>9</u> FT C								IER FA			30"		GROUND WATER ELEV.	
	Γ	<b></b>		SAM	PLE					T	1.			-	
			Τ	T	1	1		NS PE		CORE	DENSITY	STRATA	FIELD ID	ENTIFICATION OF SOIL	REMARKS
E	CASING		T					SAMP		TIME	OR	CHANGE	INCL. COLO	DR, LOSS OF WASH WA	TER, SEAMS
DEPTH	BLOWS PER	NU	Гуре	PEN	REC	DEPTH			TUBE)	PER FT	CONSIST	DEPTH		IN ROCK, ETC.	
	FOOT				·	@ BOT	0-6	6 - 12	12- 18	(MIN)	MOIST	ELEV		·	
		1	SS	24"	12"	2'0"	3	1	ļ	ļ	moist	0.01	6" ASPHALT		
		2	ss	24"	14"	4'0"	1 8	1 10			v loose moist	2'0"		, lit sand, silt ( fill ) D, lit silt ( poss fill )	
		-	- <sup>30</sup>	21	1-1	10	10	12			compact	3'6"			
5		3	SS	24"	18"	6'0"	12	14			moist		Brn SILT, lit FM	1 sand, tr gravel, cobbles	
		4		24"	18"	8'0"	16 15	16 15			v stiff moist				
ĺ			SS	24	10	00	17	14		<u> </u>	hard				
		5	SS	24"	20"	10'0"	20	22			moist				
10				0.11			23	20			hard				•
		6	SS	24"	24"	12'0"	18 14	18 23			moist hard				
							14	20			naru				
15		-7		0.4	0.41	4721011	10	10					0.4.1/5		
		7	SS	24"	24"	17'0"	12 15	12 14		-	moist v stiff		SAME		
											, oum		lit cobbles from	18'6" - 19'6"	
20		8	SS	24"	18"	22'0"	14	19			moist			M sand, tr FC gravel	
	-	0	- 55	24	10	22.0	20	21			hard			w saliu, (i i O gravei	
0.5				·											
25		9	SS	24"	12"	27'0"	25	30			moist		SAME		
			00	21		210	33	34			hard	27'0"	0,1112		
														E.O.B. 27'0"	
30											ĺ				
30	-														
35															
40															
	TE: Sub	soil	con	ditio	ons re	evealed	i by th	is inv	estig	ation r	epresent				
	con	ditio	ns a	t spe	ecific	locatio	ons an	d ma	y not	repres	ent				
GRO	CON DUND SUF					cations	s or tin SED	nes.		CASING	THEN_	CA	SING TO	FT. HOLE NO.	B-15
	AUGER				RBED I	PISTON			NWALI	- '	V = VANE TI				
	R = WEIG					WOH = V					S			C = COARSE M = MEDIUM	
	SPLIT TU					H.S.A. =					20 - 35% AI	ND =35 - 50		= FINE	

	SOI		STI	-		<b>C</b> .	CLIE	NT:	Jaca	bacci	Constructi	on Associ	iation Inc	SHEET <u>1</u> OF	
			RD, C				PRO	ECT N	0	G267	7-0245-15				B-16
			)3) 2					ECT N		0201	-02-5-15			BORING LOCATIONS	
			14) 9							543-	557 Ellswo	orth Stree	et	per Plan	
=0	REMAN -	DRIL	LER				LOCA	TION		Bridg	geport, Cl	Ē			
	TP/ad SPECTOR										CACINO		00055.04.5	OFFORT	
INC	PECIOR							TYPE			CASING HSA	SAMPLER SS	CORE BAR NWD4	OFFSET DATE START	1/15/16
- -		ATER	OBS	FRVA	TION	<u></u>	-	SIZE			4 1/4"	1 3/8"	21/8"	4	1/15/16
	<u>11_</u> FT					•			MER W	Г.		140#	BIT	SURFACE ELEV.	11 10/10
T	FTAF	TER_	<u> </u>	URS				HAM	MER FA	LL		30"	dia	GROUND WATER ELEV.	
				SAM	PLE						1				
	CASING BLOWS PER FOOT	NO	Туре	PEN	REC	DEPTH	ON (FOR	WS PE I SAMF CE ON 6 - 12	'LER TUBE)	CORE TIME PER FT	DENSITY OR CONSIST			ENTIFICATION OF SOIL RE DR, LOSS OF WASH WATE IN ROCK, ETC.	
	FUUT	1	SS	24"	10"	@ BOT 2'0"	5	8	1	(MIN)	MOIST dry	ELEV	2" ASPHALT / I	Brn VF-F to M SAND, sm F gravel,	lit slit
							6	7			compact	2'0"	Blk ASH & F G	RAVEL ( fill )	·
		2	SS	24"	2"	4'0"	8	10 20			dry		Brn VF-F to M	SAND, sm F gravel, lit silt, ash ( fill	)
5		3	SS	24"	18"	6'0"	12	20			compact dry	4'6"			
							27	29			v dense			C SAND, lit F gravel, tr silt	
		4	SS	24"	18"	8'0"	36	32			dry V danaa		Brn VF-FM to C	SAND, lit F gravel, silt	
		5	SS	24"	18"	10'0"	24 16	25 12			v dense moist		Brn FMC SAND	), sm silt, lit F gravel	
0							12 22	13			compact			, on one are grater	
		6 ss 24" 17" -						20			wet		SAME		
			<u> </u>	<u> </u>			17	13			dense				
5				0.4		47108	10								
		7	SS	24"	20"	17'0"	13 19	14 13			wet dense		Brn FMC SAND	, sm silt, lit F gravel	
							10	10	<u> </u>		dense				
20		8	SS	15"	15"	21'3"	20	38			wet	20'6"	SAME		
			- 33	10	10	215	60/3"	30			v dense			rtially decomposed BEDROCK	
													0 7 1		
5															
5		9	SS	5"	5"	25'5"	60/5"				v dense		SAME		
					_										
0												30'0"	AUGER REFUS	AL	
		1	С	60"	46"	35'0"	RQD =	30%		1.0	F		BEDROCK ( Sch		
										1.0					
										<u>1.0</u> 1.5					
5										1.5		35'0"			
													E	E.O.B. 35'0"	
				_											
o														•	
0	cone	ditio	ns at	t spe	cific	locatio	ons ar	nd mag			epresent ent				
20	CONC DUND SUF					cations	<u>s or tir</u> SED	nes.	· (		THEN_	CAS		FT. HOLE NO.	B-16
	AUGER I							T = TH	INWALL		V = VANE TE				
						WOH = \					S			C = COARSE 1 = MEDIUM	
	SPLIT TU					H.S.A. = - 10% l					20 - 35% AN	ND =35 - 50		i = FINE	

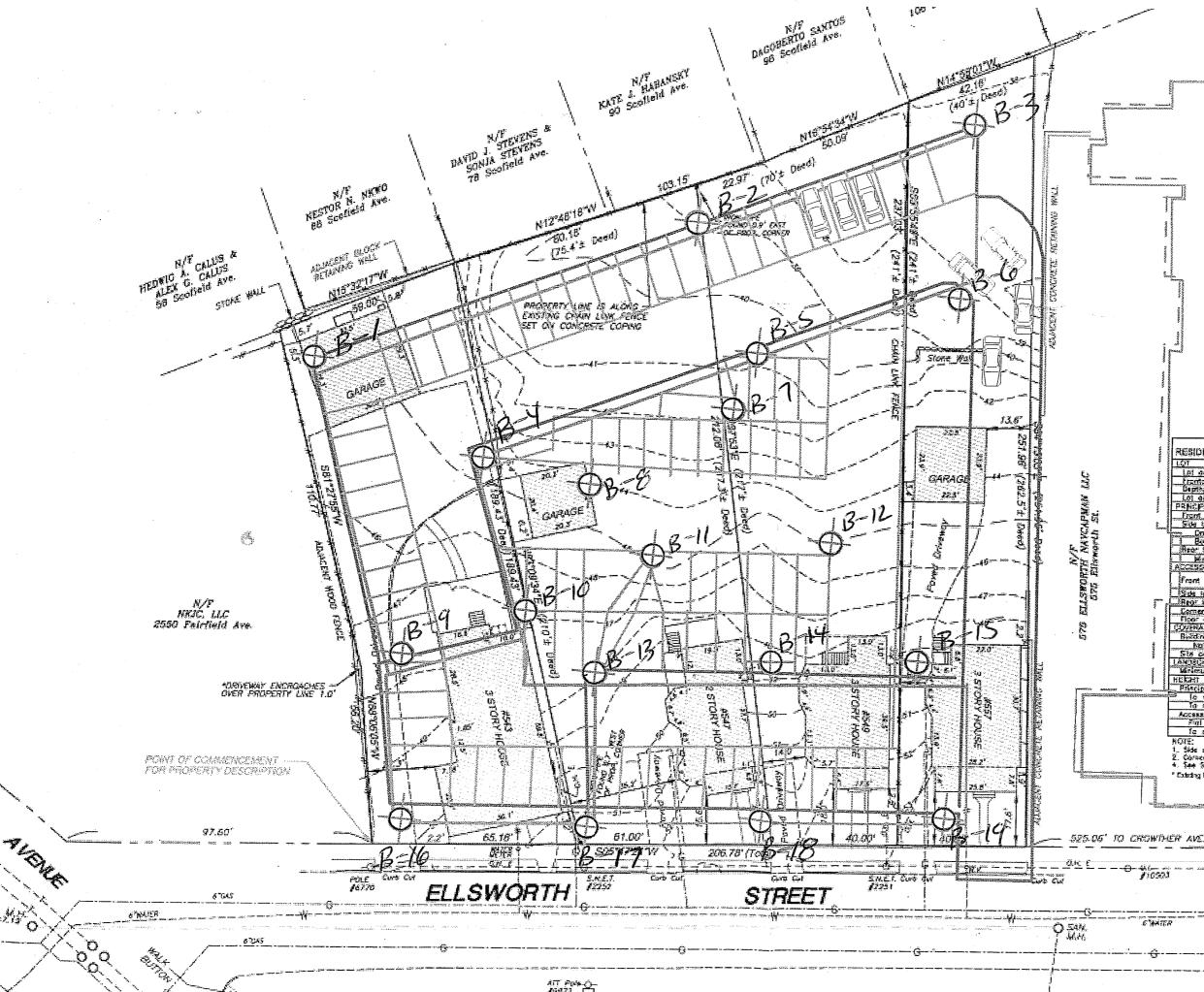
90 DONOVAN RD.         HOLE           OXFORD, CT 06478         PROJECT NO.         G267-0245-15           CT (203) 262-9328         PROJECT NAME         BORING LOCAT	NO. B-17
	er Plan
FOREMAN - DRILLER LOCATION Bridgeport, CT	
TP/ad	
INSPECTOR CASING SAMPLER CORE BAR OFFSET	1/13/16
TYPEHSASSDATE STARTGROUND WATER OBSERVATIONSSIZE I.D.4 ¼"1 3/8"DATE FINISH	1/13/16
AT_none_FT_AFTER_0_HOURS HAMMER WT. 140# BIT_SURFACE ELEV	
AT_10_FT_ON_1-15-16	R ELEV.
SAMPLE	
BLOWS PER 6 IN CORE DENSITY STRATA FIELD IDENTIFICATION	
FOOT @ BOT (MIN) MOIST ELEV	,
1         ss         24"         10"         2'0"         5         3         dry/moist         0'6"         TOPSOIL           Image: Comparison of the state of	s ( poss fill )
2 ss 24" 13" 4'0" 2 2 dry/moist SAME	- ( ,
5 30 loose 3'6"	
5         3         ss         24"         22"         6'0"         27         34         dry/moist v dense         Lt Brn/Tan VF-F to M SAND, sm s	llt, lit F gravel, tr C gravel
4 ss 24" 17" 8'0" 38 47 dry/moist	
40 31 v dense 8'0"	
5         ss         24"         17"         10'0"         16         22         dry/moist         Brn SILT & FMC SAND, lit F grave           10         25         30         hard         Brn SILT & FMC SAND, lit F grave	1
6 ss 14" 14" 11'2" 24 25 dry/moist SAME; lit cobbles	
60/2" hard	
7 ss 11" 11" 15'11" 33 60/5" dry Brn SILT & FMC SAND, lit F grave	I
20	
8 ss 10" 10" 20'10" 30 60/4" hard SAME; lit highly weathered bedroc	k
	i.
25 25'0"	······
9 ss 1" 1" 25'1" 60/1" dry 25'1" partially weathered BEDROCK	
E.O.B. 25'1"	i
Installed 1" SCH 40 PVC ob	
30 screen to 15' depth. Set of	curb box at surface
40 NOTE: Subsoil conditions revealed by this investigation represent	
NOTE: Subsoil conditions revealed by this investigation represent conditions at specific locations and may not represent	
conditions at other locations or times.         GROUND SURFACE TO       FT.       USED       CASING       THEN       CASING TO       FT.       HO	LE NO. B-17
A = AUGER UP = UNDISTURBED PISTON T = THINWALL V = VANE TEST	
WOR = WEIGHT OF RODS WOH = WEIGHT OF HAMMER & RODS C = COARSE	
SS = SPLIT TUBE SAMPLER         H.S.A. = HOLLOW STEM AUGER         M = MEDIUM           PROPORTIONS USED:         TRACE = 0 - 10%         LITTLE = 10 - 20%         SOME = 20 - 35%         AND = 35 - 50%         F = FINE	

	SO			-		).	CLIEN	IT:	Jaca	bacci (	Constructi	on Associ	ation Inc	SHEET_1_0	
										0007	0045 45			HOLE NO.	B-18
			RD, C 03) 2(							G26/	-0245-15				
			(3) 20 (4) 94				PRUJ	ECT N/		543-5	557 Ellswe	orth Stree	et	BORING LOCATIONS per Plan	
FO	REMAN -		****				LOCA	TION			jeport, Cl				
L	TP/ad														
IINS	SPECTOR							TYPE			CASING HSA	SAMPLER SS	CORE BAR	OFFSET DATE START	1/14/16
GR	OUND W	ATER	OBSE		TION	3		SIZE	D			1 3/8"		DATE FINISH	1/14/16
	_25_FT								IER WI	г.		140#	BIT	SURFACE ELEV.	1714710
AT.	FT AF	TER_	но	URS				HAMN	IER FA	LL	· · ·	30"		GROUND WATER ELEV.	
			ę	SAM	PLE								<u> </u>		
							BLO	WS PE	R 6 IN	CORE	DENSITY	STRATA		ENTIFICATION OF SOIL DR, LOSS OF WASH WA	
DEPTH	CASING BLOWS	NO	Туре	PEN	REC	1		SAMP CE ON		TIME PER	OR CONSIST	CHANGE DEPTH		IN ROCK, ETC.	IER, SEANIS
۳ ۳	PER					DEPTH	0-6	6 - 12		FT	MOIOT				
	FOOT	1	SS	24"	10"	@ BOT 2'0"	4	3	1	(MIN)	MOIST dry	ELEV	Drk Brn SILT, li	it FM sand, tr F gravel, roots ( t	opsoil )
							2	4			stiff				, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
		2	SS	24"	6"	4'0"	2	3			dry	3'6"	SAME		
5		3	SS	24"	16"	6'0"	5	15 15			stiff dry			lit silt, tr F gravel SAND, sm F gravel, tr silt	
ľ			00			1	20	28			dense			sind, ontri gravor, a one	
		4	SS	24"	18"	8'0"	27	37			dry	0101		), sm silt, lit F gravel	
		5	ss	24"	18"	10'0"	38 20	50 19			v dense dry		lit cobbles from Brn SILT & FM	7 - 8 C SAND, lit FC gravel	
10							18	21			hard				
		6	SS	24"	21"	12'0"	21	30			dry		SAME		
							34	38			hard				
15					4.0%	17101		- 10							
		7	SS	24"	. 18"	17'0"	<u>20</u> 41	<u>40</u> 37			dry hard		SAME		
							-11	- 07			nara				
20		8	SS	24"	22"	22'0"	18	22			dry	•	Grv SILT & FM0	C SAND, lit FC gravel, cobbles	
			- 00	21			43	31			hard				
25															
<b> </b> _		9	SS	24"	0"	27'0"	23	27			wet		no recovery		
							39	37			hard				
													SAME		
30															
		10	SS	24"	19"	32'0"	31	36			wet		Grn highly to pa	rtially weathered BEDROCK	
							42	50			hard	32'0"		E.O.B. 32'0"	
													•		
35															
	·														.
40 NO	TE. 01		0.07	di4i -	ne -		اله يرط ا			ation -	oprocent				· · · · · · · · · · · · · · · · · · ·
						locatio					epresent ent				
	con	ditio	ns a	<u>t oth</u>	er lo	cations	s or tir		_						
	DUND SÜ AUGER						SED	T = TH	NWALI	CASING	THÊN _ V = VANE T		SING TO	FT. HOLE NO.	B-18
wo	R = WEIG	НТ ОІ	F ROI	os		WOH = V	VEIGHT	OF H	AMMER	& ROD				C = COARSE	
	= SPLIT T					H.S.A. = - 10% L					20 - 35% AI	ND =35 - 50		/I = MEDIUM F = FINE	

:	SOII 90			NG, 'AN F		<b>.</b>	CLIEN	NT:	Jaca	bacci (	Constructi	on Associ	iation Inc	SHEET_1_OF_2 HOLE NO. B-19
				T 06			PRO-	ECT N	0.	G267	-0245-15			B-19
			-	62-93				ECT N						BORING LOCATIONS
				46-48						543-5	557 Ellswo	orth Stree	et	per Plan
	/AN - I	DRILI	ER				LOCA	TION		Bridg	geport, Cl	Γ		
	2/ad										0.00000			
NOPEL	JIOR							TYPE			CASING HSA	SAMPLER SS	CORE BAR	OFFSET DATE START 1/13/16
ROUN		TER	OBS	RVA	TION	<u></u>	1	SIZE			3 ¾"	1 3/8"	·····	DATE FINISH 1/13/16
	FT /					-			IER W	г.		140#	BIT	SURFACE ELEV.
.TF1	Γ AF	TER_	HO	URS			I	HAM	IER FA	LL		30"		GROUND WATER ELEV.
				ŞAMI	PLE									
		NO	Туре	PEN	REC	DEPTH @ BOT	ON (FOR	WS PE I SAMF CE ON 6 - 12	'LER TUBE)	CORE TIME PER FT (MIN)	DENSITY OR CONSIST MOIST	STRATA CHANGE DEPTH ELEV		ENTIFICATION OF SOIL REMARKS DR, LOSS OF WASH WATER, SEAN IN ROCK, ETC.
		.1	SS	24"	13"	2'0"	2	2			dry/moist		Drk Brn SILT (	topsoil ), lit FM sand, tr F gravel, roots
<u> </u>	]	0		0.4"	0"	4101	2	3			soft	016"	CAME	
$\vdash$		2	SS	24"	8"	4'0"	4	4			dry stiff	3'6"	SAME Brn VF-F to M S	SAND & SILT, tr F gravel
5		3	SS	24"	14"	6'0"	5	14	ļ		dry			F to M SAND, sm F gravel, lit silt
$\vdash$		4		24"	16"	8'0"	21 23	20 26	1		dense dry		SAME	
$\vdash$		4	SS	<u> 24</u>	10	00	30	20			dry v dense		1	), sm silt, lit F gravel, tr cobbles
	5 ss 17" 14" s						16	28			dry		SAME	-
0							60/5" 16	15			v dense dry		lit cobbles from	
-			- 55	24	17	12'0"	14	18			compact			SAND, sm silt, lit F gravel
5												15'0"		
۳ <u>–</u>		7	SS	24"	13"	17'0"	14	12			dгу		Brn SILT & FMC	C SAND, lit F gravel
							16	17			v stiff			
₀ ├──														
		8	SS	24"	16"	22'0"	15	57			wet		SAME; tr cobble	25
							22	25			hard			
5							-							
		9	SS	24"	13"	27'0"	9 10	10 13			wet v stiff		Brn SILT & FMC	CSAND, lit F gravel
							10	10			v Suit			•
י		10		24"	16"	32'0"	7	11			wat		SAME: tr C are:	
	-+	10	SS	24	10	JZU	16	20			wet v stiff		SAME; tr C grav	
<u> </u>														
5		11	SS	23"	22"	36'5"	16	28			wet		Brn highly to par	tially weathered BEDROCK
							47	60/5"			hard			,
<u> </u>									·					
┉	-+													
						evealed locatio					epresent ent			
	cond	itio	ns at	t oth	er lo	cations	or tir		-	-				FT. HOLE NO. B-19
	DSUR SER L					T. US PISTON	ED	T = TH	INWALI	CASING	THEN		SING TO	FT. HOLE NO. B-19
) DR = \	WEIGH	IT OF	ROL	s		WOH = V	VEIGH	ΓOF Η/	AMMER	& ROD				C = COARSE
= SP	LIT TU	JBE S	SAMPI	LER		H.S.A. =	HOLLO	DW STR	EM AUC	SER			N	A = MEDIUM

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	SOI				•	С.	CLIEN	ŧΤ:	Jaca	bacci (	Constructi	on Assoc	iation Inc	SHEET_2_OF	2
			NOV											HOLE NO.	B-19
			RD, C					ECT N		G267	-0245-15		- 147.82		•
			)3) 2  4) 9				PROJ	ECT N	AME	543-	557 Ellswo	orth Stre	of	BORING LOCATIONS per Plan	
FO	REMAN -		-				LOCA	TION			geport, Cl			perrian	
	TP/ad														
INS	SPECTOR										CASING	SAMPLER	CORE BAR	OFFSET	
			OPS			<u> </u>	4	TYPE			HSA	SS			1/13/16
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						cations		nes.		CASING	THEN_			FT. HOLE NO.	B-19
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#### PATRICIA C. SULLIVAN

Please Reply To Bridgeport Writer's Direct Dial: (203) 337-41247 E-Mail: psullivan@cohenandwolf.com

October 27, 2021

Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, Connecticut 06604

#### Re: 547 North Avenue, Bridgeport

Dear Mr. Buckley:

Enclosed please find an Application filed by 547 N Ave Bridgeport Realty, LLC ("Applicant") under the Bridgeport Zoning Regulations ("Regulations") for a Special Permit and Site Plan Review ("Application") for property owned by the Applicant located at 547 North Avenue ("Site") in Bridgeport, Connecticut. The Site is in an I-L zone and is improved with a motor vehicle gas station, including an existing building supporting that use. The present Application proposes to use 850 square feet of the existing building as a convenience store, selling items typically found in a motor vehicle gasoline station convenience store.

The Site is located at the intersection of North Avenue and Housatonic Avenue and the gas station use is long existing. The site can be accessed from both Housatonic and North Avenues. There will be no changes to the existing site, the proposal is just to add the sale of convenience store items to the existing structure on the Site.

The Applicant respectfully requests that the Commission approve its request for a convenience store on this site.

Patre C. S.C.

Patricia C. Sullivan

PCS:rpr

1115 BROAD STREET P.O. BOX 1821 BRIDGEPORT, CT 06601-1821 TEL: (203) 368-0211 FAX: (203) 394-9901 158 DEER HILL AVENUE Danbury, CT 06810 Tel: (203) 792-2771 Fax: (203) 791-8149 320 Post Road West Westport, CT 06880 Tel: (203) 222-1034 Fax: (203) 227-1373

	D BUILDER COL	Clī	TY OF BRIDGEP	ORT	File No	
A LEV	5		APPLICATIO	N		
1.	NAME OF APPL	ICANT: 547 N AV	ve Bridgepor	t Realty L	LC	
	Is the Applicant's	name Trustee of Reco	ord? Yes	1	No <u>×</u>	
		the sector decision of the	Repoficiary shall ar	company this a	application upon f	iling.
3.	Address of Prope	erty: 547 North A	avenue, briu	gepon, o	00000	(zip code)
		(number)	(street) 53/1514		(state) Lot No. <u>1</u>	(2)) 0000)
4.	Assessor's Map	Information: Block No.		a	Section:	
5.						
	(Attach copies	of Amendment) roperty (Metes & Bound	225.24' x	15.00' x 2	17.22' x 12	3.28'
6.	Description of Pr	operty (Metes & Bound	bs): <u></u>			
7.	Evicting Zone C	lassification: I-L				
- 05	Zone Classificat	ion requested: n/a				
9. 9			operty: Petitioner pro	poses to create	approximately 850 \$	SF retail convenience store
780	WILLI ALL CAR	sung building do	arraceeeee		and the second s	cle service facility
		Jested: Special P	ermit and Si	te Plan Re	eview	
	Apploval(3) requ			0		
		-DA-C	-21-t	N	Date:	
	Signature:	Patricia C. Sulliva	n. Attorney for t	he Applicant		
	Print Name:		•		_	
	If signed by Age	ent, state capacity (Law	vyer, Developer, etc	:.) Signature:	Patricia C. Sulliva	n, Attorney for the Applicant
				man be a bill and a second	et Bridgen	ort_CT 06604
	Mailing Address	c/o Cohen &	203 4	14 6455	Fax:	203-337-5524
	Phone: 203-	<u>337-4124</u> s: psullivan@co	Cell: <u>203</u> 2	com	- Fax.	
	E-mail Address	s: psunivan@cc	Jilenanowon			
			Deter		Clerk:	
	\$	_Fee received	Date:			
	THIS	APPLICATION MUST	BE SUBMITTED IN	PERSON AN	D WITH COMPLE	ETED CHECKLIST
		& Signed Application F		D A-2 Site Si		Building Floor Plans
		Site / Landscape Plan		Drainage F	Plan	Building Elevations
		itement of Developmer		D Property C	)wner's List	🗆 Fee
	Written Sta     Cart of Inc	corporation & Organizat	tion and First Repo	rt (Corporations	s & LLC's)	
	Cert. of Inc		Noti Burg t Hot topp			
		PROPI	ERTY OWNER'S E	NDORSEMEN	T OF APPLICAT	ION intrilar
	547 N Ave Brid	Igeport Realty LLC	Vie i.L	Belou	At the	Data
	Print C	)wner's Name	Owner'	s Signatŭre		Date
		Owner's Name	Owner	s Signature		Date
	Print C	WINE S INGINE	0.1.101	<b>U</b> 1		

Rev. 6/18/2016

PROPERTIES WITHIN 100' OF 547 NORTH AVENUE

PROPERTY ADDRESS	OWNERS NAME	MAILING ADDRESS	CITY	STATE	STATE ZIP CODE
596 NORTH AV	MCKENZIE DORETH	747 LAUREL AVE	BRIDGEPORT	ст	06604
635 NORTH AV	EZ REALTY LLC	643 NORTH AVE	BRIDGEPORT	CT	06606
625 NORTH AV	BRACAGLIA PAOLO	291 TOLL HOUSE LN	FAIRFIELD	СТ	06825
580 NORTH AV #582	580 NORTH AVE LLC	580-582 NORTH AVE	BRIDGEPORT	СТ	06604
547 NORTH AV	547 N AVENUE BRIDGEPORT REALTY LLC 555 S COLUMBUS AVE	555 S COLUMBUS AVE	MOUNT VERNON	NΥ	10550
608 NORTH AV #630	MCKENZIE DORETH	747 LAUREL AVE	BRIDGEPORT	СT	06604
529 NORTH AV	MTM FAMILY LIMITED PARTNERSHIP	<b>1137 SEAVIEW AVE</b>	BRIDGEPORT	СТ	06607
615 NORTH AV	615 NORTH AVE LLC	580 NORTH AVE	BRIDGEPORT	ст	06606
584 NORTH AV #588	MCCARTHY WILLIAM C	<b>134 SUNRISE HILL CIR</b>	ORANGE	CT	06477

1

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BUSINESS DETAILS

# 547 N AVE BRIDGEPORT REALTY LLC ACTIVE

555 S COLUMBUS AVE. SUITE 201, MOUNT VERNON, NY, 10550, United States

#### **Business Details**

**General Information** 

Business Name 547 N AVE BRIDGEPORT REALTY LLC

Business status ACTIVE

Citizenship/place of formation Foreign/NY

Business address 555 S COLUMBUS AVE. SUITE 201, MOUNT VERNON, NY, 10550, United States

Annual report due 3/31/2022

NAICS code Lessors of Nonresidential Buildings (except Miniwarehouses) (531120)

Business ALEI 1189005

Date formed 10/26/2015

Business type LLC

Mailing address 555 S COLUMBUS AVE. SUITE 201, MOUNT VERNON, NY, 10550, United States

Last report filed 2021

NAICS sub code

#### **Principal Details**

Principal Name TUMAY BASARANLAR

Principal Title MANAGER

https://service.ct.gov/business/s/onlinebusinesssearch?businessName=547 n ave bridgeport realty llc

## 10/27/21, 3:45 PM

 $\sim$ 

Principal Business address 555 S COLUMBUS AVE., SUITE 201, MOUNT VERNON, NY, 10550, United States

Principal Residence address 161 DUANE STREET, NEW YORK, NY, 10007, United States

#### BUSINESS DETAILS

JIMMY KOCHISARLI

Principal Title MANAGER

Principal Business address 555 SOUTH COLUMBUS AVENUE, SUITE 201, MT. VERNON, NY, 10550, United States

Principal Residence address 3 CROSSBOW LANE, WOODBURY, NY, 11797, United States

Principal Name JOSE MONTERO

Principal Title MANAGER

Principal Business address 555 SOUTH COLUMBUS AVE, SUITE 201, MT. VERNON, NY, 10550, United States

Principal Residence address 199 PINESBRIDGE ROAD, OSSINING, NY, 10562, United States

#### Agent details

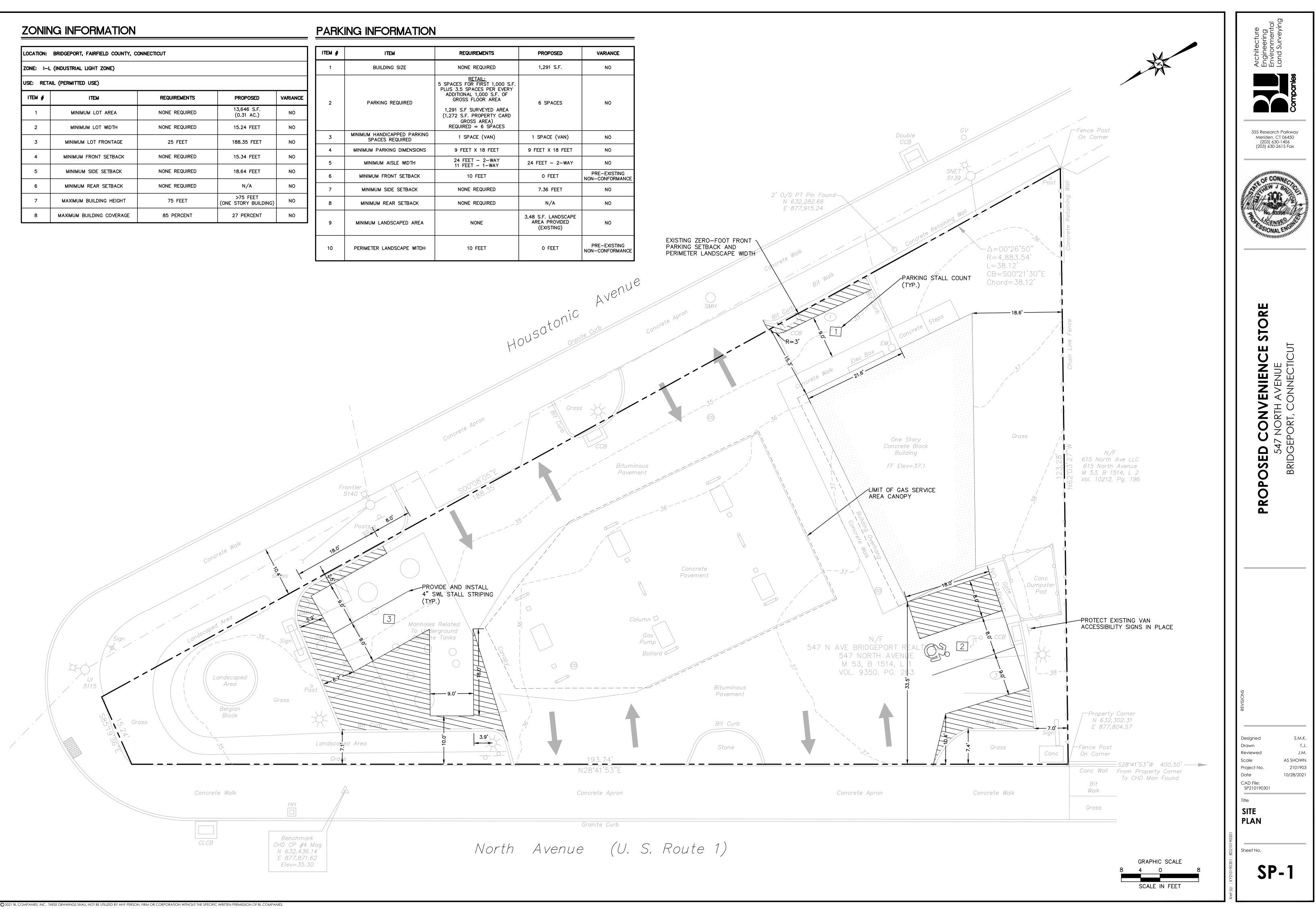
Agent name UNITED CORPORATE SERVICES, INC.

Agent Business address 66 CEDAR STREET, NEWINGTON, CT, 06111, United States

Agent Mailing address 66 CEDAR STREET, NEWINGTON, CT, 06111, United States

ZONE: I—L	. (INDUSTRIAL LIGHT ZONE)										
USE: RETAIL (PERMITTED USE)											
ITEM #	ITEM	REQUIREMENTS	PROPOSED	VARIANCE							
1	MINIMUM LOT AREA	NONE REQUIRED	13,646 S.F. (0.31 AC.)	NO							
2	MINIMUM LOT WIDTH	NONE REQUIRED	15.24 FEET	NO							
3	MINIMUM LOT FRONTAGE	25 FEET	188.35 FEET	NO							
4	MINIMUM FRONT SETBACK	NONE REQUIRED	15.34 FEET	NO							
5	MINIMUM SIDE SETBACK	NONE REQUIRED	18.64 FEET	NO							
6	MINIMUM REAR SETBACK	NONE REQUIRED	N/A	NO							
7	MAXIMUM BUILDING HEIGHT	75 FEET	>75 FEET (ONE STORY BUILDING)	NO							
8	MAXIMUM BUILDING COVERAGE	85 PERCENT	27 PERCENT	NO							

ITEM #	ITEM
1	BUILDING SIZE
2	PARKING REQUIRED
3	MINIMUM HANDICAPPED PARKIN SPACES REQUIRED
4	MINIMUM PARKING DIMENSIONS
5	MINIMUM AISLE WIDTH
6	MINIMUM FRONT SETBACK
7	MINIMUM SIDE SETBACK
8	MINIMUM REAR SETBACK
9	MINIMUM LANDSCAPED AREA
10	PERIMETER LANDSCAPE WITDH



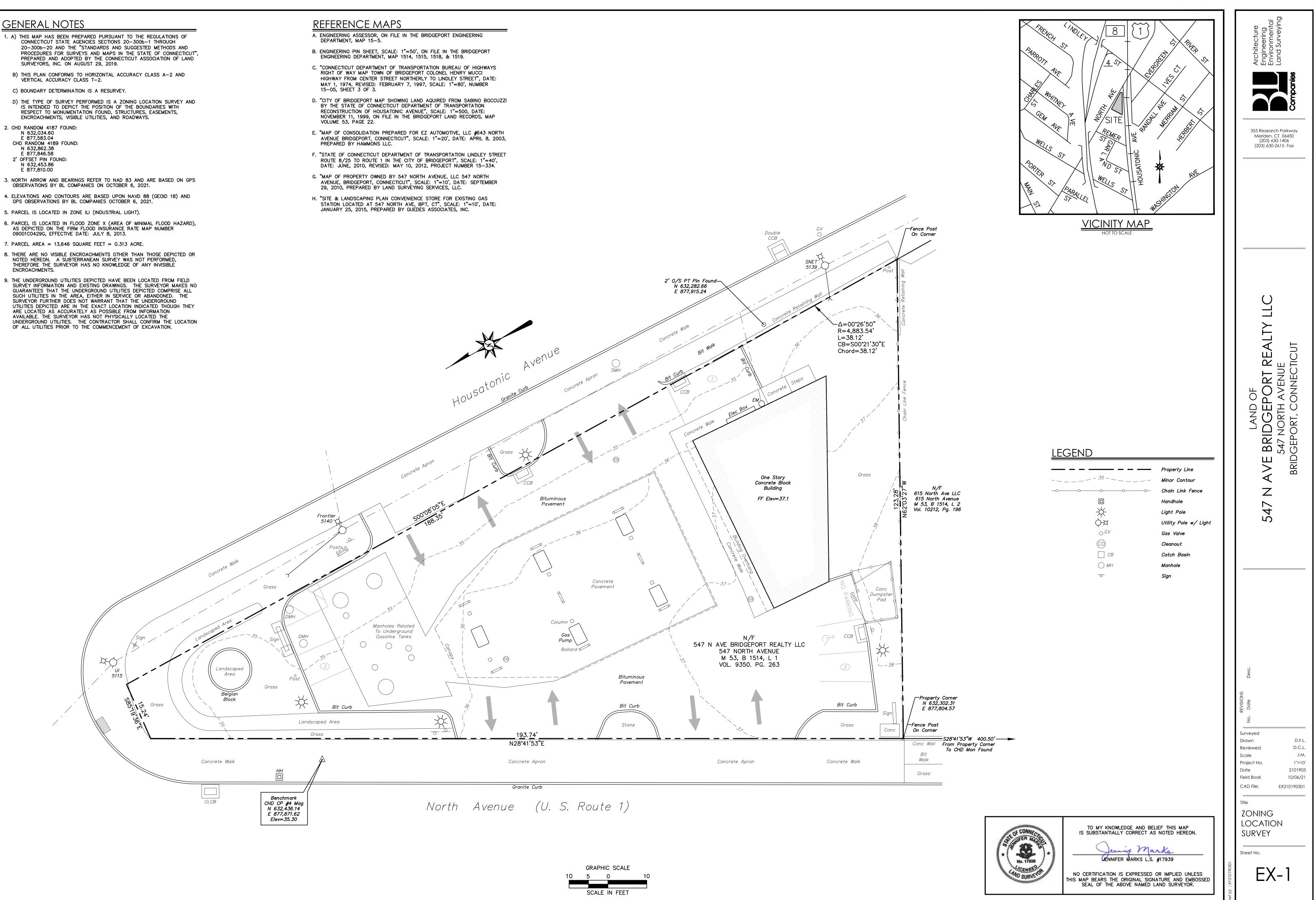


# GENERAL NOTES

- 1. A) THIS MAP HAS BEEN PREPARED PURSUANT TO THE REGULATIONS OF CONNECTICUT STATE AGENCIES SECTIONS 20-300b-1 THROUGH 20-300b-20 AND THE "STANDARDS AND SUGGESTED METHODS AND PROCEDURES FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT", PREPARED AND ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC. ON AUGUST 29, 2019.
- B) THIS PLAN CONFORMS TO HORIZONTAL ACCURACY CLASS A-2 AND VERTICAL ACCURACY CLASS T-2.
- C) BOUNDARY DETERMINATION IS A RESURVEY.
- IS INTENDED TO DEPICT THE POSITION OF THE BOUNDARIES WITH RESPECT TO MONUMENTATION FOUND, STRUCTURES, EASEMENTS, ENCROACHMENTS, VISIBLE UTILITIES, AND ROADWAYS.
- N 632,034.60
- E 877,583.04 CHD RANDOM 4189 FOUND:
- N 632,862.38 E 877,846.58
- 2' OFFSET PIN FOUND: N 632,453.86
- E 877,810.00
- 3. NORTH ARROW AND BEARINGS REFER TO NAD 83 AND ARE BASED ON GPS OBSERVATIONS BY BL COMPANIES ON OCTOBER 6, 2021.
- 4. ELEVATIONS AND CONTOURS ARE BASED UPON NAVD 88 (GEOID 18) AND
- 5. PARCEL IS LOCATED IN ZONE ILI (INDUSTRIAL LIGHT).
- 6. PARCEL IS LOCATED IN FLOOD ZONE X (AREA OF MINIMAL FLOOD HAZARD), AS DEPICTED ON THE FIRM FLOOD INSURANCE RATE MAP NUMBER
- 8. THERE ARE NO VISIBLE ENCROACHMENTS OTHER THAN THOSE DEPICTED OR NOTED HEREON. A SUBTERRANEAN SURVEY WAS NOT PERFORMED, THEREFORE THE SURVEYOR HAS NO KNOWLEDGE OF ANY INVISIBLE
- 9. THE UNDERGROUND UTILITIES DEPICTED HAVE BEEN LOCATED FROM FIELD SURVEY INFORMATION AND EXISTING DRAWINGS. THE SURVEYOR MAKES NO GUARANTEES THAT THE UNDERGROUND UTILITIES DEPICTED COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED. THE SURVEYOR FURTHER DOES NOT WARRANT THAT THE UNDERGROUND UTILITIES DEPICTED ARE IN THE EXACT LOCATION INDICATED THOUGH THEY ARE LOCATED AS ACCURATELY AS POSSIBLE FROM INFORMATION AVAILABLE. THE SURVEYOR HAS NOT PHYSICALLY LOCATED THE UNDERGROUND UTILITIES. THE CONTRACTOR SHALL CONFIRM THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF EXCAVATION.

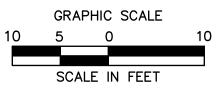
2021 BL COMPANIES, INC. THESE DRAWINGS SHALL NOT BE UTILIZED BY ANY PERSON, FIRM OR CORPORATION WITHOUT THE SPECIFIC WRITTEN PERMISSION OF BL COMPANIES.

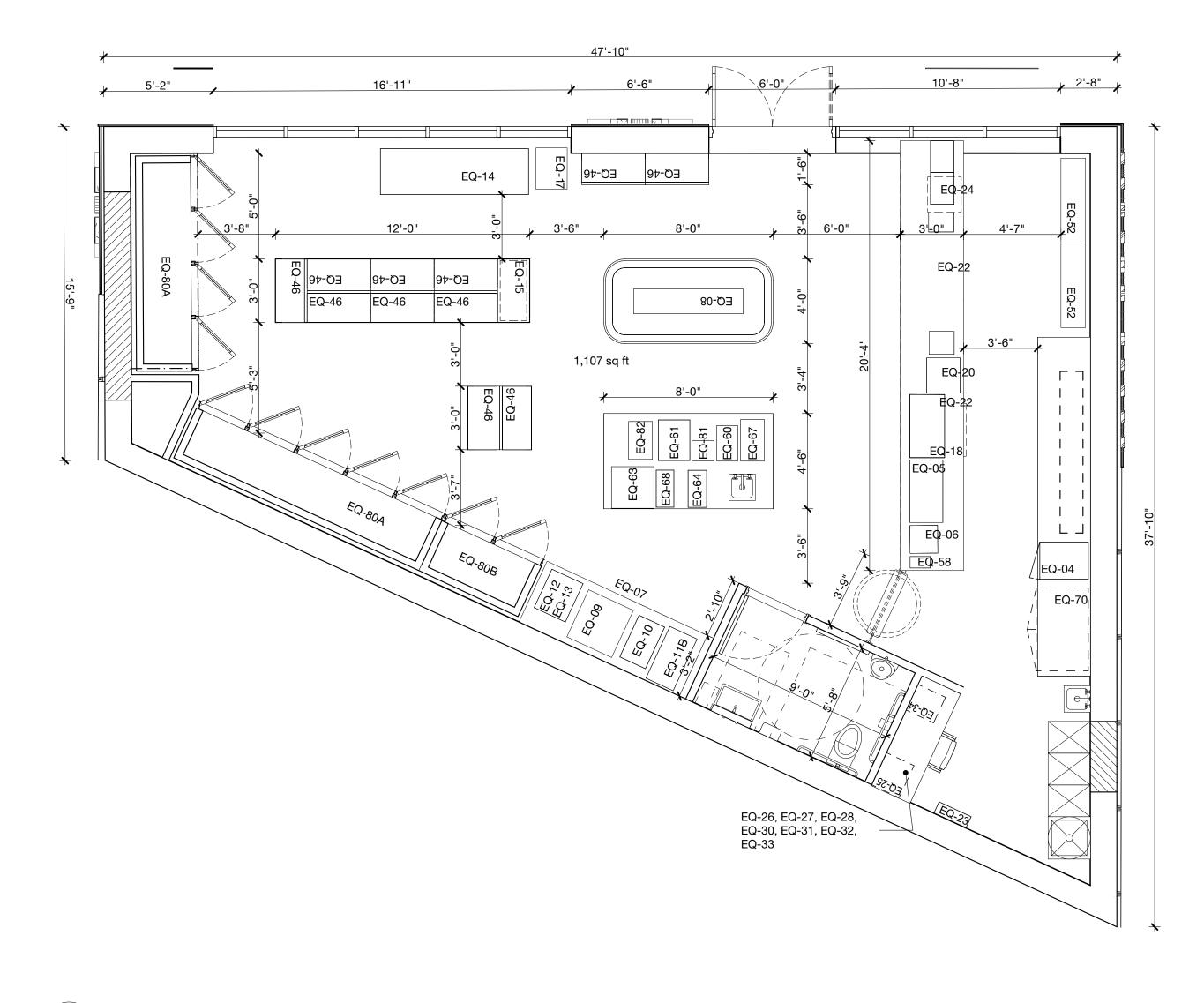
- 15-05, SHEET 3 OF 3.
- VOLUME 53, PAGE 22.
- PREPARED BY HAMMONS LLC.



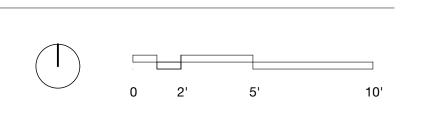
EX-202 28,

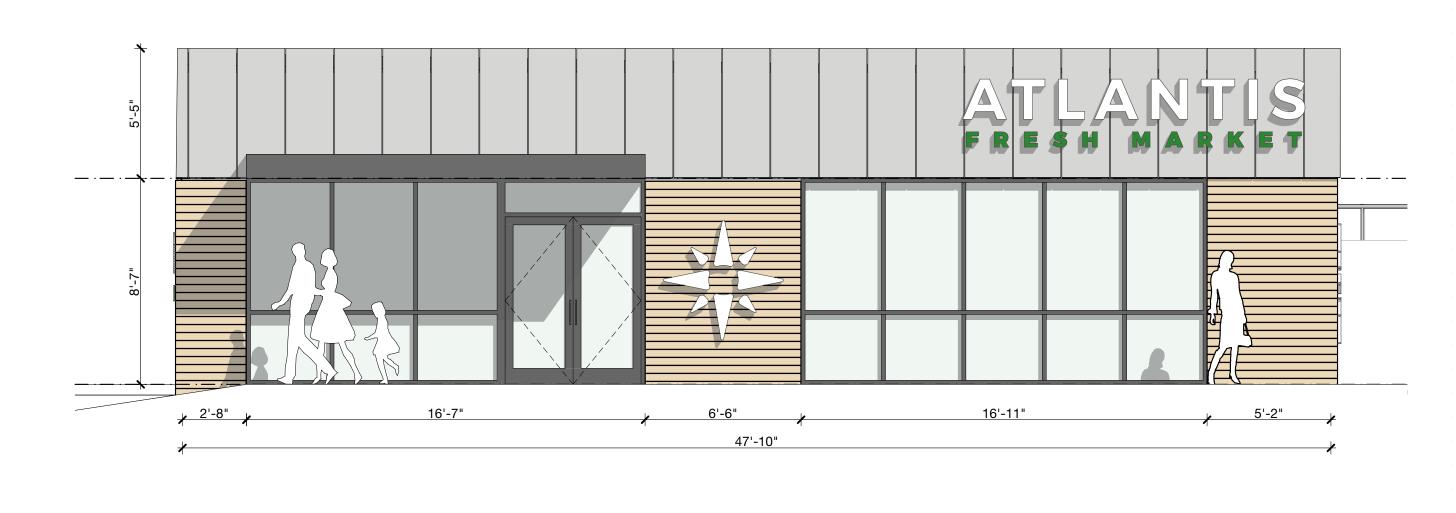


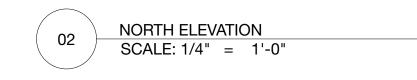




01 PROPOSED ONVENIENCE STORE LAYOUT SCALE: 1/4" = 1'-0"



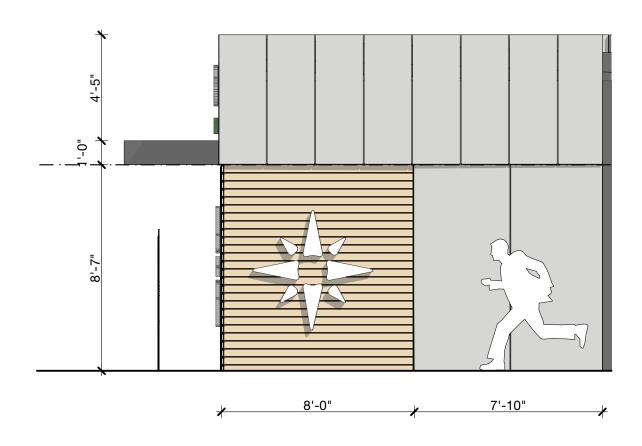








EAST ELEVATION SCALE: 1/4" = 1'-0"



04 WEST ELEVATION SCALE: 1/4" = 1'-0"

# ATLANTIS FRESH MARKET CONVENIENCE STORE RENOVATION

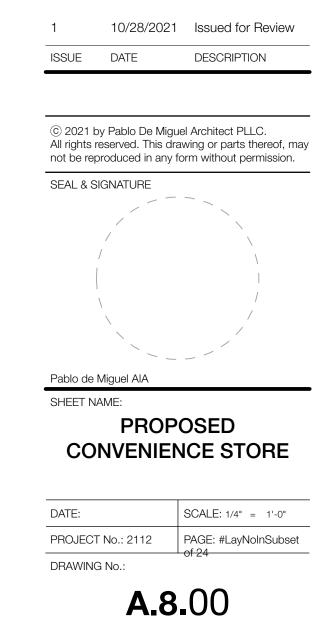
#### **547 North Avenue** Bridgeport, NY 06606 #Site ID

# OWNER:

Atlantis Management Group 555 S Columbus Ave #201 Mt. Vernon NY 10550 contact: Jose Montero T. 914-699-9500 E. Josem@atlantismgmt.com

# ARCHITECT:

Pablo De Miguel Architect PLLC
162 14th Street
Brooklyn, NY 11215
contact: Pablo de Miguel AIA
T. 646 265 0338 E. pablo@pablodemiguel.com
W. www.pablodemiguel.com

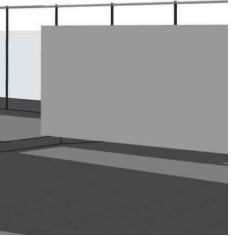




#### Pablo de Miguel Architect PLLC

162 14th street Brooklyn, NY 11215

hola@pablodemiguel.com www.pablodemiguel.com



#### ATLANTIS FRESH MARKET

547 North Avenue Bridgeport CT 06606

## VIEW 1

SCALE: **1:133.78** DATE: **10/20/21** 





#### Pablo de Miguel Architect PLLC

162 14th street Brooklyn, NY 11215

hola@pablodemiguel.com www.pablodemiguel.com

#### ATLANTIS FRESH MARKET

547 North Avenue Bridgeport CT 06606

## VIEW 2

SCALE: **1:163.59** DATE: **10/20/21** 





#### Pablo de Miguel Architect PLLC

162 14th street Brooklyn, NY 11215

hola@pablodemiguel.com www.pablodemiguel.com

#### ATLANTIS FRESH MARKET

547 North Avenue Bridgeport CT 06606

## VIEW 3

SCALE: **1:2.10** DATE: **10/20/21** 



Contra La	OF BRIDGEPOR	ITY OF BRIDGEP	ORT		
T IO TWAS	OPPORATED 1849	& ZONING ( APPLICATIO			0
1.	NAME OF APPLICANT: SVMC Hold	dings, Inc.			
2.	Is the Applicant's name Trustee of Rec	cord? Yes	No	<u>х</u>	<u>.</u>
	If yes, a sworn statement disclosing the		ccompany this ap	oplication upo	on filing.
3. /	Address of Property: 2800 Main Stree	et			
	(number)	(street)	(s	tate)	(zip code)
4. <i>A</i>	Assessor's Map Information: Block No	59/2120	Lo	ot No. <u>1/X</u>	
5. A	Amendments to Zoning Regulations: (i	indicate) Article: <u>N/A</u>	λ	Sectio	n:
(	Attach copies of Amendment)				
6. E	Description of Property (Metes & Boun	nds): <u>504.31' x 967.</u>	50' x 510.12' x 9	975.37'	2
_					
7. E	Existing Zone Classification: P2				
	Zone Classification requested: <u>N/A</u>				
). C	Describe Proposed Development of Pr	operty: Installation	of a roof sign or	n the existing	g hospital building
-	Approval(s) requested: Special Permi				
s	Signature:			Date?	0/127/2022
P	Print Name:			AL'	V/
IT	f signed by Agent, state capacity (Law		- 1/		L
	lailing Address: c/o Chris Russo, Ru		Print Name: 🏒	ad Eairfield	CT 06824
	-				
	Phone: 203-255-9928	Cell: 203-520-	4003	Fax:	203-255-6618
E	-mail Address: Chris@russorizio.co				
\$	Fee received	Date:	Cl	erk:	
	THIS APPLICATION MUST B				
	Completed & Signed Application Fo	orm 🗆	A-2 Site Surve	у	Building Floor Plans
	Completed Site / Landscape Plan		Drainage Plan		Building Elevations
	Written Statement of Development	and Use 🛛 🚦	Property Owne	r's List	🗆 Fee
	Cert. of Incorporation & Organizatio	on and First Report (	Corporations & L	LC's)	
			m		
	PROPER	TY OWNER'S END	ORSEMENT OF	APPLICATI	ION
S	WMC Holdings, Inc.		V		01/27/2022
_	Print Owner's Name	Owner's Si	ignature	×	Date
_					
	Print Owner's Name	Owner's Si	gnature		Date

Lisa S. Broder\* LBroder@russorizio.com

Colin B. Connor Colin@russorizio.com

Robert G. Golger Bob@russorizio.com

David K. Kurata DKurata@russorizio.com

Stanton H. Lesser+ Stanton@russorizio.com

Katherine M. Macol Kathy@russorizio.com

Victoria L. Miller<sup>\*</sup> Victoria@russorizio.com

Anthony J. Novella\* Anovella@russorizio.com



10 Sasco Hill Road, Fairfield, CT 06824 Tel 203-254-7579 or 203-255-9928 Fax 203-576-6626

5 Brook St., Suite 2B, Darien, CT 06820 Tel 203-309-5500

299 Broadway, Suite 708, New York, NY 10007 Tel 646-357-3527

www.russorizio.com

January 27, 2022

Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, CT 06604 **HAND-DELIVERED**  Leah M. Parisi Leah@russorizio.com

William M. Petroccio\* WPetro@russorizio.com

> Raymond Rizio\* Ray@russorizio.com

Christopher B. Russo Chris@russorizio.com

Rob@russorizio.com

John J. Ryan+ John@russorizio.com

Jane Ford Shaw Jane@russorizio.com

Vanessa R. Wambolt Vanessa@russorizio.com

\* Also Admitted in NY Also Admitted in VT + Of Counsel

#### Re: Petition for Special Permit and Site Plan Review - 2800 Main Street

Dear Mr. Buckley:

Please accept, on behalf of my client, SVMC Holdings, Inc. (the "Petitioner"), the following narrative and enclosed application materials as part of an application for the property located at 2800 Main Street (the "Site") for Special Permit and Site Plan Review approval to install a roof sign on an existing hospital building.

#### **Narrative**

The Petitioner requests a Special Permit and Site Plan Review under Sections 11.50 and 11.70 of the Zoning Regulations of the City of Bridgeport (the "Regulations") to install a roof sign on the existing hospital building. The Site occupies an entire block that is bounded by Main Street, Hunting Street, Gurdon Street and Hawley Avenue. The Site is located in the P2 Zone. The lot area of the Site is approximately 7.84 acres.

The Petitioner proposes to install a roof sign on the existing hospital building, so the sign will display on the southern frontage of the Site. The sign is proposed to be located set back from the edge of the roof, which will reduce visibility of the sign from neighboring properties below. Submitted plans show the proposed visibility of the sign from different surrounding locations. They depict that the sign will have little to no visibility from surrounding neighbors. The sign will be visible at a greater distance from commuters on the nearby highway system.

The sign structure is proposed to be 72' in length, which will not extend over or beyond the façade of the existing building in accordance with the Regulations. The sign structure will be almost 17' in height, which is several feet below the permitted height under the Regulations. The actual

signage lettering and hospital logo will occupy a much smaller area on this sign structure. The sign support frame will be painted to match the building color to hide the structure and promote the lettering. The sign will be illuminated for better visibility at night.

Roof signs are permitted within the P Zone under Sec. 9.50.6 of the Regulations. The proposed sign does not exceed width of the building and the proposed height is below the permitted standard by several feet. The non-display side of the sign will be shielded as a setback portion of the existing building will be located directly behind the proposed sign and the sign structure will be painted in the same color as the existing building. The sign will not cover windows or any significant architectural feature.

Hospitals, for better or worse, are a central location for every person. Whether for work, a routine visit, or an emergency, at some point, everyone will need to easily locate a hospital. This not only includes area residents familiar with the location of a hospital, but also persons with no knowledge of this area, such as an out-of-state family member visiting their sick relative. The Regulations recognize hospitals as one of the tallest, most prominent buildings of the cityscape and, again, as a central feature of any community. The Plan of Conservation and Development ("POCD") recognized that hospital buildings are anchor institutions within the City. They are an incredible service to our community and they represent a growing and significant workforce center. The POCD set a goal to work with these institutions to promote their connection to the City. This Petition will help support the vital role this anchor institution holds in the City.

#### Special Permit and Site Plan Review

The Petition satisfies all Special Permit and Site Plan Review standards under Sections 11.50 and 11.70 of the Regulations. The Petitioner has proposed a signage plan which is in conformity with the Regulations. The proposed sign location will not eliminate or reduce by more than 25% the view of significant natural or local features as shown on the submitted plans in accordance with Sec. 9.50.6.D(3). As stated above, the Petition satisfies the objectives and policies of the POCD. The proposed use is permitted in the Zone under the Regulations. The proposed sign will not impair future development of the surrounding area, but it will support one of the greatest drivers of development in the City. It will not be detrimental to the nearby surrounding area as the sign is barely visible due to its location set back from the existing hospital building and due to the height of the building itself. The sign does not extend beyond the façade at any location, which will also protect nearby properties. The sign will obviously not have any impact on the Long Island Sound.

For the reasons stated above, the Petitioner respectfully requests approval of the application for Special Permit and Site Plan Review.

Sincerely, Ray Rizio

PROPERTIES WITHIN 100' OF 2800 MAIN ST

LOCATION	OWNER'S NAME	MAILING ADDRESS	CITY	STAT	STATE ZIP CODE
<b>2900 MAIN ST</b>	2900 MAIN STREET LLC	2839 MAIN STREET, 2ND FL	BRIDGFPORT	t	
280 GURDON ST	STOKES MICHAEL J SR & DEBORAH D STROKES	280 GURDON ST	BRIDGEPORT	55	06606
2800 MAIN ST	SVMC HOLDINGS INC	ONE STATE STREET SUITE 19	HARTFORD	С	06103
236 GURDON ST #238	HAWLEY AVENUE HOLDINGS LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	ե	06606
40 HUNIING ST	PADILLA AIESHA	184 MARTIN TER	BRIDGEPORT	С	06606
191 HAWLEY AV #193	AMARAL ERNEST & DOROTHY	636 TAFT AVE	BRIDGEPORT	CT	06604
2819 MAIN ST #2821	ST VINCENTS MEDICAL CENTER FOUNDATION	2979 MAIN ST	BRIDGEPORT	С	06606
199 HAWLEY AV #201	CASABLANCA PAUL M	199 HAWLEY AVE	BRIDGEPORT	С	06606
141 HAWLEY AV	HAWLEY AVENUE HOLDINGS LLC	2800 MAIN STREET, 2ND FL	BRIDGEPORT	с	06606
60 HUNTING ST	MAIN STREET PROP LLC	2800 MAIN ST	BRIDGEPORT	С	06606
263 HAWLEY AV #265	JEANCALIXTE ALFRED ET AL	265 HAWLEY AVE	BRIDGEPORT	С	06606
183 HAWLEY AV #185		125 POWELL PLACE	BRIDGEPORT	Ե	06604
270 GURDON ST	LYDDY CHRISTOPHER J & BARBARA	45 JOHNSON PLACE	MONROE	С	06468
216 GURDON ST #218	SVMS HOLDINGS INC	ONE STATE STREET SUITE 19	HARTFORD	С	06103
2857 MAIN ST #2859	CRESPO JOAQUIN	2857 MAIN ST	BRIDGEPORT	С	06606
242 GURDON ST #246	ST VINCENTS DEVELOPMENT INC	101 S HANLEY ROAD SUITE 450	ST LOUIS	MO	63105
255 HAWLEY AV #257	HAWLEY AVENUE HOLDINGS LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	С	06606
223 HAWLEY AV #225	SIMPSON ANN-MARIE T & PRINCE CLARKE	223 HAWLEY AVE #225	BRIDGEPORT	сŢ	06606
	NORTHBRIDGE LANDLORD LLC C/O KEYBANK	ATTN: ESCROW ADMINISTRATION,			
<b>2875 MAIN ST</b>	REAL ESTATE CAPITAL	11501 OVERLOOK ST	OVERLAND PARK	KS	66211
230 GURDON ST	HAWLEY AVENUE HOLDINGS LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	ե	06606
66 HUNTING ST	COLLAZO MARIA	66 HUNTING ST	BRIDGEPORT	С	06606
74 HUNTING ST #76	MAIN STREET PROPERTIES LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	Ե	06606
2837 MAIN ST #2839	MAIN STREET PROPERTIES LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	C	06606
136 HAWLEY AV	CIRILO DOLORES ESTATE OF & MARIA CIRILO	136 HAWLEY AVE	BRIDGEPORT	С	06606
84 HUNTING ST #86	FEOLA GENE N & EVELYN	49 LAUREL ST	TRUMBULL	C	06611
146 HUNTING ST	SVMC HOLDINGS INC	ONE STATE STREET SUITE 19	HARTFORD	CT	06103
88 HUNTING ST	MAIN STREET PROPERTIES LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	СТ	06606
290 GURDON ST #292	PROVENZANO LOUIS	309 HOUSATONIC AVE	STRATFORD	сŢ	06615
98 HUNTING ST #100	MAIN STREET PROPERTIES LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	С	06606
104 HUNTING ST	MAIN STREET PROPERTIES LLC	2839 MAIN STREET, 2ND FL	BRIDGEPORT	CT	06606

PINHEIRO TIAGO VIRGILE FRANCOISE	BMXS BPT LLC NORTHBRIDGE LANDLORD LLC	ANTON FREDERICK W III ET AL	OTERO RAFAEL & IVONNE PEREZ	AFM ENTERPRISES INC	HESKE RONALD W & NANCY A HESKE	FLORES BIBIANO	SVMC HOLDINGS INC	HAWLEY AVENUE HOLDINGS LLC	DOS SANTOS DANIEL A	HAWLEY AVENUE HOLDINGS LLC	ABD LLC C/O PETER DINARDO ENTERPRISES
	140 HAWLEY AV #144 2861 MAIN ST #2863		167 HAWLEY AV #169	2829 MAIN ST #2831	175 HAWLEY AV #177	205 HAWLEY AV #211	2771 MAIN ST #2817	235 HAWLEY AV #237	245 HAWLEY AV #247	2754 MAIN ST #2756	2741 MAIN ST #2755

260 GURDON ST	В
248 GURDON ST	В
140-144 HAWLEY AVE	В
135 SOUTH ROAD	щ
159 HAWLEY AVE, #161	В
169 HAWLEY AVENUE	В
67 FOLINO DR	В
177 HAWLEY AVE	В
157 DEN ROAD	Ś
<b>ONE STATE STREET SUITE 19</b>	Т
2839 MAIN STREET, 2ND FL	В
245 HAWLEY AVE #247	В
2839 MAIN STREET, 2ND FL	В
323 NORTH AVENUE	В

00990	00600	00990	06032	06610	00600	00990	06606	06903	06103	00990	06606	06606	00990
	RT CT												
BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	FARMINGTON	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	STAMFORD	HARTFORD	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT

#### FILING #0006251242 PG 03 OF 05 VOL E-02578 FILED 09/25/2018 10:00 AM PAGE 01716 SECRETARY OF THE STATE CONNECTICUT SECRETARY OF THE STATE

#### CERTIFICATE OF INCORPORATION

#### OF

#### SVMC HOLDINGS, INC.

#### 1. The name of the corporation is SVMC Holdings, Inc. (the "Corporation").

2. The Corporation is organized and shall be operated exclusively for charitable, scientific, literary or educational purposes within the meaning of Section 501(c)(3) of the Internal Revenue Code of 1986, as amended (the "Code"). The Corporation shall be operated as a component part of the integrated health care delivery system of which the parent is Hartford HealthCare Corporation (the "System"). The nature and activities to be conducted, or the purposes to be promoted or carried out by the Corporation, are as follows:

2.1. establishing and maintaining one or more hospitals or other health care facilities in the City of Bridgeport, Connecticut and in additional communities served by the Corporation;

2.2. providing health and wellness services and promoting and improving the general health and welfare of the communities served by the Corporation;

2.3. engaging in medical and scientific research, and in educational and other activities to promote and improve the general health and welfare of the communities served by the Corporation;

2.4. making grants to organizations within the System recognized as exempt from federal income tax under Section 501(c)(3) of the Code;

2.5. conducting activities, either directly or through related organizations recognized as exempt from federal income tax under Section 501(c)(3) of the Code, to raise funds in furtherance of the foregoing purposes of the Corporation, subject, however, to all limitations on the nature or extent of such activities applicable to organizations recognized as exempt from federal income tax under Section 501(c)(3) of the Code; and

2.6. in furtherance of the foregoing, engaging in any lawful act or activity for which corporations may be formed under the Revised Nonstock Corporation Act of the State of Connecticut (the "Act") as the same may be amended from time to time.

3. The Corporation is nonprofit and shall not have or issue shares of stock or make distributions or pay dividends.

4. The Corporation shall have a single member, namely, Hartford HealthCare Corporation, a Connecticut nonstock corporation (the "Member"). The Member shall have the exclusive power to elect directors of the Corporation ("Directors") and to remove Directors with or without cause, shall have the exclusive power to adopt, amend, and repeal the Bylaws of the Corporation (the "Bylaws"), and shall have such other rights, powers, and responsibilities as are accorded to members under the Act, this Certificate of Incorporation, or the Bylaws.

5. Notwithstanding any other provision of this Certificate of Incorporation to the contrary, the Corporation shall not carry on any activities not permitted to be carried on: (a) by

an organization exempt from federal income tax under Section 501(a) of the Code as an organization described in Section 501(c)(3) of the Code; or (b) by an organization, contributions to which are deductible under Section 170(c)(2) of the Code.

6. The net earnings of the Corporation or any part thereof may not be distributed to or inure to the benefit of any private individual or a Director or officer of the Corporation. However, nothing herein shall restrict the right of the Corporation to reasonably compensate any officer, Director or other individual for services rendered to the Corporation or to reimburse any officer, Director or other individual for expenses, disbursements or liabilities properly made or incurred, on account of that individual's service to the Corporation.

7. A substantial part of the activities of the Corporation shall not consist of the carrying on of propaganda or attempting to influence legislation except to the extent permitted by Section 501(h) of the Code. The Corporation may not participate in or intervene in (including the publication or distribution of statements) any political campaign on behalf of (or in opposition to) any candidate for public office.

8. Upon dissolution of the Corporation, the Board shall dispose of and distribute the assets remaining, after payment of all liabilities, exclusively for the purposes of the Corporation, to the Member exclusively for its charitable, scientific, literary or educational purposes, provided the Member shall be then exempt from federal taxation as an organization described in Section 501(c)(3) of the Code. If the Member shall not be so qualified as an organization described in Section 501(c)(3) of the Code, then the Board shall dispose of and distribute the assets remaining, after payment of all liabilities, exclusively for the charitable, scientific, literary or educational purposes of the Corporation, to one or more organizations as shall be then exempt from federal taxation as an organization 501(c)(3) of the Corporation, to one or more organizations as shall be then exempt from federal taxation as an organization or organizations described in Section 501(c)(3) of the Corporation or organizations described in Section 501(c)(3) of the Corporation or organizations described in Section 501(c)(3) of the corporation or organizations described in Section 501(c)(3) of the exempt from federal taxation as an organization or organizations described in Section 501(c)(3) of the corporation or organizations described in Section 501(c)(3) of the benefit of any private individual.

9. The personal liability of a Director of the Corporation to the Corporation for monetary damages for breach of duty as a Director of the Corporation shall be limited to the fullest extent permitted by the Act or any other applicable laws presently or hereafter in effect. Without limiting the effect of the preceding sentence, no Director of the Corporation shall be personally liable to the Corporation for monetary damages for breach of duty as a Director of the Corporation in an amount greater than the compensation received by the Director for serving the Corporation during the year of the violation if such breach did not: (i) involve a knowing and culpable violation of law by the Director; (ii) enable the Director, or an associate, as defined in Section 33-840 of the Connecticut General Statutes, to receive an improper personal economic gain; (iii) show a lack of good faith and a conscious disregard for the duty of the Director to the Corporation under circumstances in which the Director was aware that his or her conduct or omission created an unjustifiable risk of serious injury to the Corporation; or (iv) constitute a sustained and unexcused pattern of inattention that amounted to an abdication of the Director's duty to the Corporation. No amendment to, or modification or repeal of, this Article 9 shall adversely affect any right or protection of a Director of the Corporation existing hereunder with respect to any act or omission occurring prior to such amendment, modification or repeal. Nothing contained in this Article 9 shall be construed to deny to the Directors of the Corporation the benefit of Section 52-557m of the Connecticut General Statutes as in effect at the time of the violation.

10. The Corporation shall, to the fullest extent permitted by law, indemnify any Director, officer, or committee member of the Corporation (and, to the extent provided in a resolution of the Member's Board of Directors or by contract, may indemnify any employee, agent, or volunteer of the Corporation) (collectively, the "Agents") who was or is a party to or threatened to be made a party to any threatened, pending, or completed action, suit, or proceeding by reason of the fact that the person is or was an Agent, or is or was serving at the request of the Corporation as an Agent of another corporation, partnership, joint venture, trust, or other enterprise, whether for-profit or not-for-profit, against expenses, including attorney's fees (other than taxes, penalties, or expenses of correction), judgments, penalties, fines, and amounts paid in settlement actually and reasonably incurred by the Agent in connection with the action, suit, or proceeding if the Agent acted in good faith and in a manner that the Agent reasonably believed to be in or not opposed to the best interests of the Corporation, and with respect to any criminal proceeding, if the Agent had no reasonable cause to believe his or her conduct was unlawful.

11. The name and address of the initial registered agent of the Corporation is Hartford HealthCare Corporation, 85 Jefferson Street, Legal Department, Hartford, CT 06106.

12. References in this Certificate of Incorporation to the Act shall be deemed to include amendments adopted from time to time to such Act, and references to a Section of the Code shall be construed to refer both to such Section and to the regulations promulgated thereunder, as they now exist or as the same may hereafter be amended from time to time (or the corresponding provision of any future United States Internal Revenue Law).



#### SECRETARY OF THE STATE OF CONNECTICUT

MAILING ADDRESS: COMMERCIAL RECORDING DIVISION, CONNECTICUT SECRETARY OF THE STATE, P.O. BOX 150470, HARTFORD, CT 05115-0470 DELIVERY ADDRESS: COMMERCIAL RECORDING DIVISION, CONNECTICUT SECRETARY OF THE STATE, 30 TRINITY STREET, HARTFORD, CT 06106 PHONE: 860-509-6003

WEBSITE:WWW.concord-sols.ct.gov

## **CERTIFICATE OF INCORPORATION** NONSTOCK CORPORATION

	TE ALL SECTIONS. PRINT OR TYPE. ATTACH 81/2.	CONNECT	251242 PG 01 OF 05 VOL E-02578 09/25/2018 10:00 AM PAGE 01714 SECRETARY OF THE STATE TCUT SECRETARY OF THE STATE
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ADDRESS:	80 Seymour Street		
	Legal Department		
CITY:	Hartford		
STATE:	CT ZIP: 06102		
SIAL,			
1. NAME OF C	ORPORATION:		
SVMC Holdi	ngs, Inc		
THE CORPORA DISTRIBUTIONS	TION IS NONPROFIT AND SHALL NOT H. S.	AVE OR ISSUE SHAI	RES OF STOCK OR MAKE
2. PLACE A CH	ECK NEXT TO THE APPROPRIATE STA	TEMENT:	
	PORATION SHALL NOT HAVE MEMBER	S.	
B. THE COR	PORATION SHALL ONLY HAVE MEMBE	RS, WHICH ARE NO	T ENTITLED TO VOTE.
C. THE COR	PORATION SHALL HAVE ONE CLASS O	F MEMBERS.	
- D. THE COR	PORATION SHALL HAVE MULTIPLE CLA	ASSES OF MEMBER	S WHICH CLASSES ARE
DESIGNA	TED AS FOLLOWS:	•	
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	NT OF REGISTERED AGENT: (PLEASE SEL)	ECT DIVLY ONE A. URB.)	,
	S AGENT NAME:		
BUSINESS ADI	DRESS: (P.O.BOX UNACCEPTABLE)	RESIDENCE ADD	RESS: (P.O.BOX UNACCEPTABLE)
ADDRESS:		ADDRESS:	
CITY:		CITY:	
STATE:	ZIP;	STATE:	ZIP:
B. BUSINESS E	NTITY AGENT NAME: HARTFORD HEALT	HCARE CORPORATIO	N
ADDRESS: (P.O.	BOX UNACCEPTABLE)		
ADDRESS:	85 JEFFERSON ST.		
	LEGAL DEPARTMENT		
CITY:	HARTFORD		
STATE:	CT ZIP: 06106	e.	

ACCEPTANCE OF APPOINT		1				
	Y		= Da	vid Ha	ACH VPI	egal
	SIGNAT	TURE OF AGENT				
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CORPORATION:						
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	- 1					
5. OTHER INFORMATION:						
SEE ATTACHMENT SHEET						
6. CORPORATION EMAIL ADI	RESS - REO	UIRED: (IF NON	E. MUST STAT	E "NONE ")		
	-,					
NONE						
			and the second			
7. EXECUTION: CERTIFICATE M	UST BE SIGNE	D BY EACH INCOR	PORATOR			
7. EXECUTION: CERTIFICATE MI DATED THIS 25 <sup>th</sup> DAY OF	UST BE SIGNE	D BY EACH INCOR September		, 20 <sup>18</sup>		
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#### STATE OF CONNECTICUT Department of Public Health

In accordance with the provisions of the General Statutes of Connecticut Section 19a-493 the following license to maintain and operate a

**General Hospital** 

LICENSE NO:

77

has been granted to SVMC HOLDINGS, INC.

d/b/a ST. VINCENT'S MEDICAL CENTER

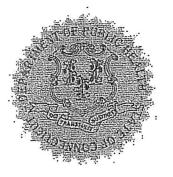
#### Located at 2800 MAIN ST BRIDGEPORT, CT 06606-4201

For the period from

10/01/2019 to 09/30/2021 .

During the license period and in accordance with the Regulations of the Connecticut State Agencies, changes to any of the following must be submitted to the Department of Public Health and are subject to the approval of the Department of Public Health: Maximum number of Beds:

Hospital beds: 473
Bassinets: 47



Appendix A: Satellite Locations

ilchell

Renée D. Coleman-Mitchell, MPH Commissioner

#### STATE OF CONNECTICUT Department of Public Health LICENSE APPENDIX A: SATELLITE LOCATIONS

#### General Hospital LICENSE NO: 77 SVMC HOLDINGS, INC.

# For the period from 10/01/2019 to 09/30/2021.

ST. VINCENT'S BEHAVIORAL HEALTH CENTER - WESTPORT - 47 LONG LOTS RD, WESTPORT, CT 06880-3828, ST. VINCENT'S CENTER FOR WOULD HEALING - STRATFORD - 3272 MAIN ST, STRATFORD, CT 06614-4819, ST. VINCENT'S CENTER FOR WOUND HEALING - TRUMBULL - 115 TECHNOLOGY DR, TRUMBULL, CT 06611-6337, ST. VINCENT'S OUTPATIENT BEHAVIORAL HEALTH - BRIDGEPORT - 2400 MAIN ST, BRIDGEPORT, CT 06606-5323, ST. VINCENT'S OUTPATIENT BEHAVIORAL HEALTH - NORWALK - 1 LOIS ST, NORWALK, CT 06851-4404

# SVMC HOLDINGS, INC. ACTIVE

2800 MAIN ST., BRIDGEPORT, CT, 06606, United States

BUSINESS DETAILS			~	
Business Details				
General Information				_
Business Name SVMC HOLDINGS, INC. Business status				
ACTIVE				
Citizenship/place of form Domestic/Connecticut	nation			
Business address 2800 MAIN ST., BRIDGEPO	PRT, CT, 06606, United Sta	ates		
Annual report due 1/13/2023				
NAICS code General Medical and Sur	gical Hospitals (622110)			
Business ALEI 1285886				
Date formed 9/25/2018				
Business type Non-Stock				
Mailing address 2800 MAIN ST., BRIDGEPO	RT, CT, 06606, United Sta	ates		
Last report filed 2022				
NAICS sub code 622110				
Principal Details				-
Principal Name CAROL D. BIRKS				
Principal Title				
Director Principal Business addres	3S			
ONE STATE STREET, SUITE		United States		

#### 1/27/22, 5:25 AM

onlineBusinessSearch

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name JAMES CARDON M.D.

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name PETER CIMINO M.D.

Principal Title Ex-Officio Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name GEORGE ESTRADA

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name HELENE GLOTZER

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name BARBARA MILLER

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name CYNTHIA PUGLIESE

Principal Title Director

#### 1/27/22, 5:25 AM

onlineBusinessSearch

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name DARA THOMAS RICHARDS M.D.

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name LUCY TEIXEIRA

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name PATRICK TOOLE

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name STRICK WOODS M.D.

Principal Title Director

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name DAVID MACK

Principal Title Secretary

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name JOHN PETILLO

#### 1/27/22, 5:25 AM

onlineBusinessSearch

Principal Title Director & Chair

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name ALFRED PAVLIS

Principal Title Director & Vice Chair

Principal Business address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Residence address ONE STATE STREET, SUITE 19, HARTFORD, CT, 06103, United States

Principal Name William Jennings

Principal Title Ex-Officio Director & President

Principal Business address One State Street, 19th Floor, Hartford, CT, 06103, United States

Principal Residence address One State Street, 19th Floor, Hartford, CT, 06103, United States

Principal Name William Jennings

Principal Title Ex-Officio Director & President

Principal Business address One State Street, 19th Floor, Hartford, CT, 06103, United States

Principal Residence address One State Street, 19th Floor, Hartford, CT, 06103, United States

#### Agent details

Agent name CORPORATION SERVICE COMPANY

Agent Business address 100 PEARL STREET, 17TH FLOOR, MC-CSC1, HARTFORD, CT, 06103, United States

F	iling History		~
	Business Formation - Certificate of Incorporation OOO6251242 Filing date: 9/25/2018	Filing time:	
	Volume Type B		

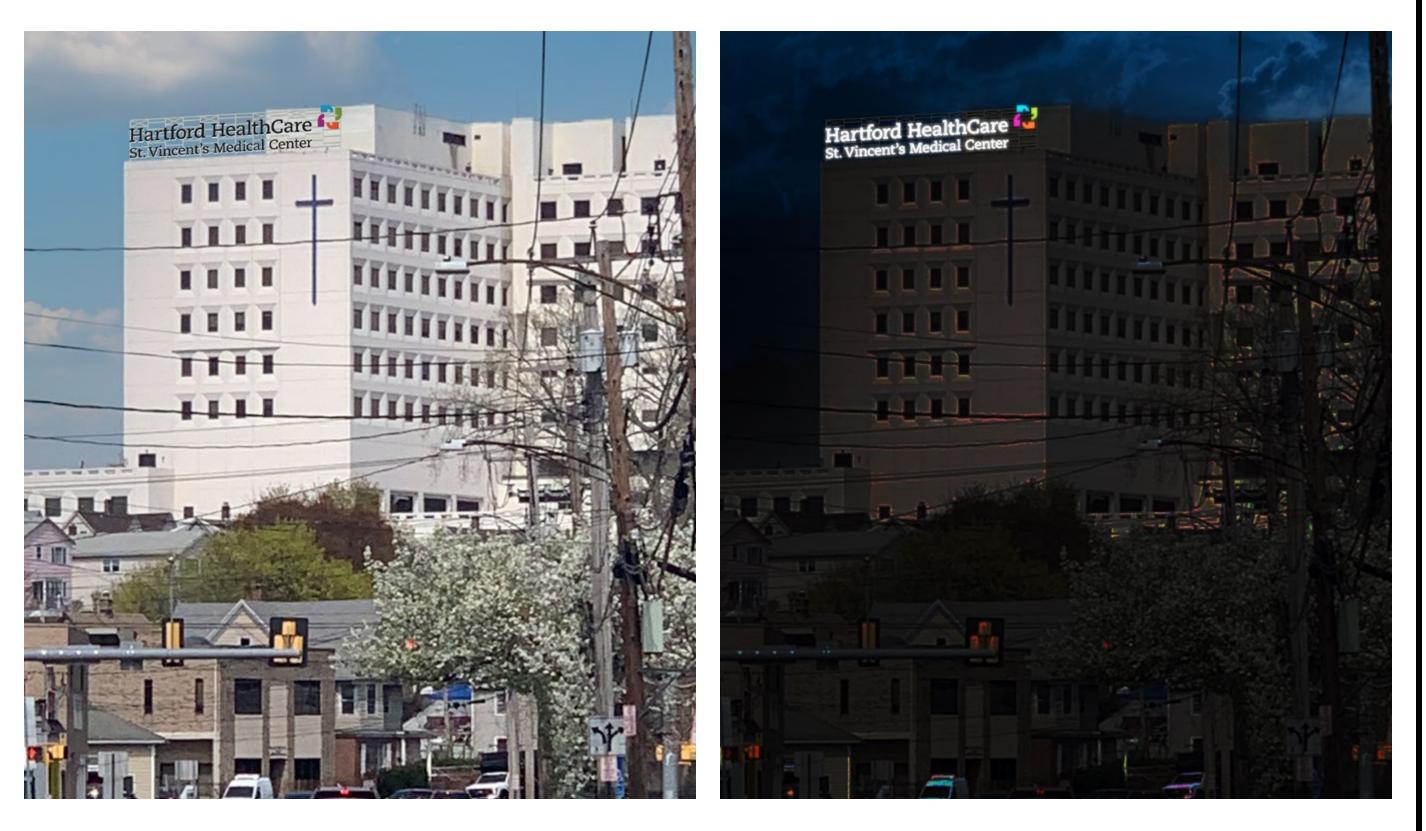
# Hartford HealthCare

HHC | 2800 Main St. BRIDGEPORT, CT | St Vincent's Medical Center Terrace Letters • Version 10 • Job# 57524 • January 21, 2022



60 Westfield Drive Plantsville, CT 06479 860.229.1812









Project Address: HHC | St Vincent's 2800 Main St. BRIDGEPORT, CT

 SPI WO #:
 57524

 Issue Date:
 3/18/2021

Salesperson:Pete RappoccioDesigner:Gigi

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#### **Revisions:**

RV1:	GD	3/26/2021
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RV3:	GD	4/26/2021
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RV7:	OK	11/18/21
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RV9:	OK	01/21/22



PMS 313c PMS 377c PMS 173c PMS 2405c

# **SIGN TYPE**

CHANNEL LETTERS (TERRACE) PAGE **2 of 8** 







Frame Mounted Channel Letters.

sign support frame

perforated black day / white night

Find to match building color Find to match building color Find the alth Care Find the alth Care St. Vincent's Medical Center

72' - 0"

Sign support frame location approx. 10' [V.I.F.] from edge of building.





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#### 60 Westfield Drive Plantsville, CT 06479 860.229.1812



SIGN TYPE CHANNEL LETTERS (TERRACE) PAGE **3 of 8** 



Hawley St. - SVMC Lot Entrance



Hawley Ave. - Gurdon St. - On Site





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57524 3/18/2021

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# SIGN TYPE

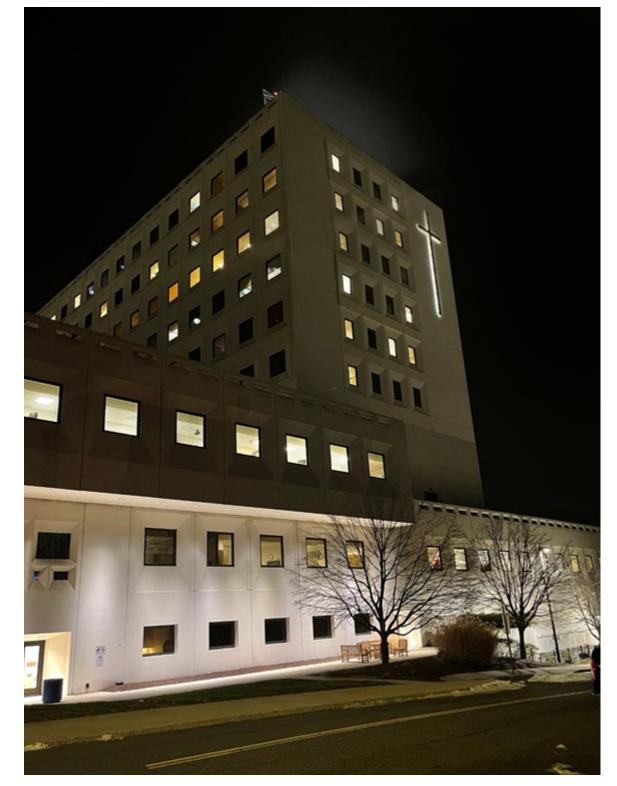
CHANNEL LETTERS (TERRACE) PAGE **4 of 8** 





60 Westfield Drive

Plantsville, CT 06479 860.229.1812





Anson St. - Hawley St.

Hawley Ave. - Gurdon St.









Project Address: HHC | St Vincent's 2800 Main St. BRIDGEPORT, CT

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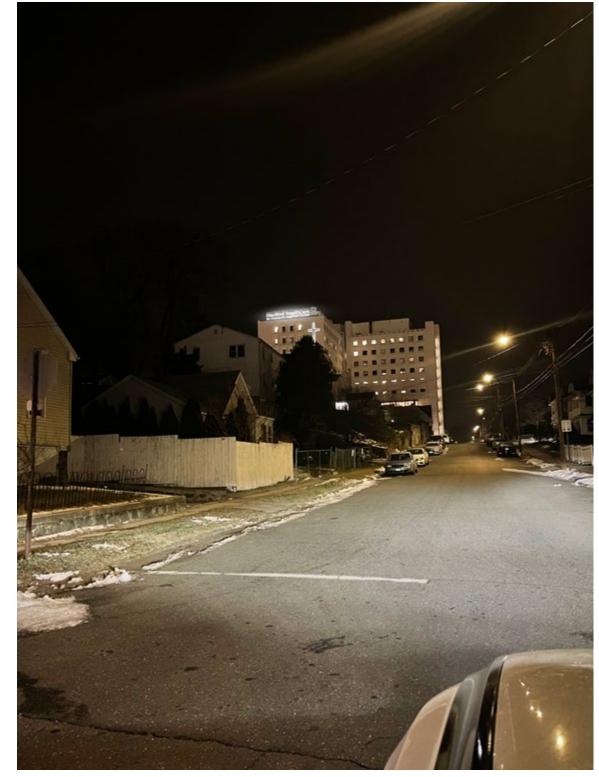
# **SIGN TYPE**

CHANNEL LETTERS (TERRACE) PAGE **5 of 8** 

60 Westfield Drive Plantsville, CT 06479 860.229.1812







Salem St. - Gurdon St.



Gurdon St. - French St.





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SPI WO #: Issue Date:

57524 3/18/2021

Salesperson:Pete RappoccioDesigner:Gigi

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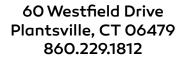
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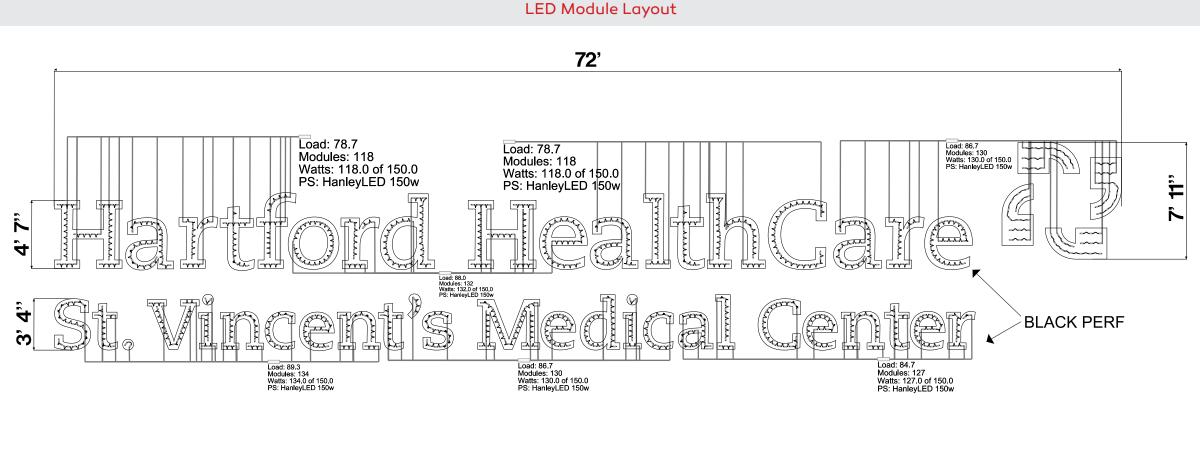
# SIGN TYPE

CHANNEL LETTERS (TERRACE) PAGE 6 of 8









#### O R D H E A L H C A 23 mods 15 mods 30 mods 39 mods 25 mods 27 mods 17 mods 16 mods 32 mods 26 mods 27 mods 23.0 w 15.0 w 30 mods 39 mods 25 mods 27 mods 17 mods 16 mods 32 mods 26 mods 27 mods 23.0 w 15.0 w 30.0 w 39.0 w 25.0 w 27 0 w 17 0 w 16.0 w 32.0 w 26.0 w 27.0 w 3.7 mod/ft<sup>2</sup> 3.4 mod/ft<sup>2</sup> 3.4 mod/ft<sup>2</sup> 3.9 mod/ft<sup>2</sup> 4.4 mod/ft<sup>2</sup> 3.6 mod/ft<sup>2</sup> 3.4 mod/ft<sup>2</sup> 3.4 mod/ft<sup>2</sup> 4.4 mod/ft<sup>2</sup> 4.75 sq ft 4.450 sq ft 6.450 sq ft 6.177 sq ft 4.755 sq ft 4.669 sq ft 6.73 sq ft 7.662 sq ft 6.177 sq ft 39 mods 39 0 w 3.7 mod/ft<sup>2</sup> A C 27 mods 15 mods 27.0 w 15.0 w 4.4 mod/ft<sup>2</sup> 3.4 mod/ft<sup>2</sup> 6.177 so ft 4.393 so ft 16 mods 16.0 w 3.4 mod/ft<sup>2</sup> 4.669 sq.ft 21 mods 21.0 w 3.6 mod/ft<sup>2</sup> 5.860 sq ft 15 mods 15.0 w 3.4 mod/ft<sup>2</sup> 24 mods 24.0 w 3.7 mod/ft<sup>2</sup> 6.447 so ft 64 mods 64.0 w 2.3 mod/ft<sup>2</sup> 5 19 mods 19.0 w 4.2 mod/ft<sup>2</sup> 4.526 sq ft v 21 mods 21.0 w 4.9 mod/ft<sup>2</sup> 4 311 so ft 12 mods 12.0 w 4.7 mod/ft<sup>2</sup> 2.541 sq ft 4 mods 4.0 w 4.4 mod/ft<sup>2</sup> 5 16 mods 16 0 w 4.8 mod/ft<sup>2</sup> 3.357 sq ft L 17 mods 17.0 w 4.9 mod/ft<sup>2</sup> 3.495 sq ft 21 mods 21.0 w 4.4 mod/ft<sup>2</sup> 4.747 sq ft 11 mods 11.0 w 5.2 mod/ft<sup>2</sup> 2.113 sq ft A 19 mods 19.0 w 5.8 mod/ft<sup>2</sup> 3.270 sq ft 19 mods 19.0 w 4.6 mod/ft<sup>2</sup> 4.155 sq ft 17 mods 17.0 w 4.9 mod/ft<sup>2</sup> 3.495 sq ft 20 mods 20.0 w 4.9 mod/ft<sup>2</sup> 4.042 sq ft 11 mods 11.0 w 5.2 mod/ft<sup>2</sup> 12 mods 12.0 w 5.0 mod/ft<sup>2</sup> 17 mods 17.0 w 4.9 mod/ft 20 mods 20.0 w 4.9 mod/ft<sup>2</sup> 37 mods 37.0 w 4.8 mod/ft<sup>2</sup> 7.670 sg ft 12 mods 12.0 w 5.0 mod/ft<sup>2</sup> 2.411 sq ft 12 mods 12.0 w 4.7 mod/ft<sup>2</sup> 2.576 sq ft 12 mods 12.0 w 4.7 mod/ft<sup>2</sup> 2.541 sq ft 12 mods 12.0 w 4.7 mod/ft<sup>2</sup> 2 mods 2.0 w 4.8 mod/ft<sup>2</sup> 0.418 sq.ft 20 mods 20.0 w 4.9 mod/ft<sup>2</sup>

Dimmers can be used on all of the modules and power supplies recommended dimmer: G2G manual dimmer adjustment knob.

889 Modules: Hanley LED - Phoenix NRG PE-3 - HLED-PE3 889.0 Watts 7 Power Supplies: Hanley LED - HanleyLED 150w 226.374 Total sq ft, 760.062 perimeter ft



**5" DEEP** ≈ 8" BETWEEN MODULES

Power Supply positioning in layout is for reference only. Please note # of power supply outputs; do not exceed power supply output limitations



17 mods 17.0 w 4.9 mod/ft<sup>2</sup>

11 mods 11.0 w 4.6 mod/ft<sup>2</sup>

≈ 5" BETWEEN MODULES @ PERF

60 Westfield Drive Plantsville, CT 06479 860.229.1812





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# **SIGN TYPE**

CHANNEL LETTERS (TERRACE) PAGE 7 of 8

# LED SPECIFICATIONS

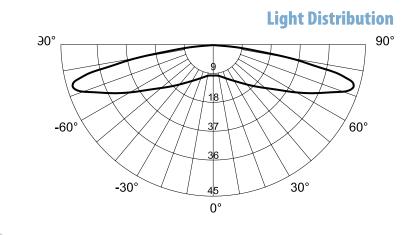
# **Specifications**

Viewing Angle	170° Phoenix Wing Optics	
Input Voltage	12vDC	
Watts	1w/mod (1.5w/ft.)	
Luminous Efficacy	170 (lm/W)	
Modules/Foot	1.5/ft. fully stretched	
Protection Grade	IP65 water proof	
Packaging	Anti-static bag, 75 modules (50 ft)/bag	
	5 bags/inner carton   10 bags/outer carton	
Warranty	10 Year (Product) / 5 Year (Labor)	
Operating Temp.	$-40^{\circ}$ ~ $+60^{\circ}$ C / $-40^{\circ}$ ~ $+140^{\circ}$ F	
Storage Temp.	$-40^{\circ}$ ~ $-70~^{\circ}$ C / $-40^{\circ}$ ~ $+158~^{\circ}$ F	
Cascade	25mods single-ended power feed 50mods double-ended power feed	

Color F		Color Temp	Lumens
Pure White	ILED-PE-3	7000K 17	70 lm/mod (255 lm/ft.)

\*Additional color temps available upon special order (MOQ's Apply)\*

		MAX POWER SUPPLY LOAD				
MODEL	35W	60W	100W	120W	150W	240W
PE-3	35mods	60mods	100mods	120mods	150mods	240mods



# POWER SUPPLY SPECIFICATIONS

# Warranty

Product

5 years

Safety Certification C RUL US (5/2) Easona US (5/2) C 262904 C Strike US (1) C 262904 C Strike US (1) C 262904 C Strike US (1) C Strike US (1)
---

1 year labor with any other qualified LED
2 years labor if paired with HanleyLEDs

Labor

# Product Parameters

Short-Circuit Protection

**Over-Voltage Protection** 

**Over-Temperature Protection** 

Output Characteristics	
Rated Output Voltage	12V
Rated Output Current	4.16A*3 CHANNELS
Rated Output Power	150W
Output Voltage Accuracy	±5%
Output ripple & noise	≤150mV
Input Characteristics	
Input Voltage Range	90 ~ 305Vac
Input Frequency Range	47HZ ~ 63HZ
Input Current	1.75A/115Vac .75A/277Vac
Inrush Current (cold start)	$\leq$ 80A
Efficiency	≥ 93% (230Vac)
PF	≥ .95(230Vac)
Power Input	2.1A Max
Protective Characteristics	
<b>Over-Current Protection</b>	

#### **Environmental Characteristics** Working Temperature $-40^{\circ} \sim +60^{\circ}$ C Working Humidity 20 ~ 95% RH (non-condensir -40° ~ +80°C Storage Temperature Storage Humidity 10~95% RH **IP** Rating IP68 Vibration 10 ~ 500HZ, 5G 30 minutes Safety and EMC P68, Class 2 Safety Rating

Dielectric	Strength (Hi-Pot)	I/P-0/P	3.75KVac/10mA/3S		
		I/P-Case	1.8KVac/10mA/3S		
Insulation	n Resistance	100M0hr	n Min/500Vdc/3S		
Groundin	g Resistance	100m0hm			
EMC		FCC part 7	FCC part 15classB		
Other Chara	octeristics				
MTBF		>50,000Khrs. MIL-HDBK-217F			
Size		202*66*34.5mm (L*W*H)			
Weight		1KG			

Customer Approval: APPROVED	APPROVED AS NOTED REVISE AND RESUBM	т
PRINT	SIGNDATE	-
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 57524

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Salesperson:Pete RappoccioDesigner:Gigi

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	GD GD SB OK OK OK



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# **SIGN TYPE**

CHANNEL LETTERS (TERRACE) PAGE **8 of 8** 

qualified LEDs
nsing)
tes (for X, Y, Z each axis)
/35
35
17F (25°C)

60 Westfield Drive Plantsville, CT 06479 860.229.1812



Cl.gov



ø	SOT BRIDGEROR
	CITY OF BRIDGEPORT File No
SENL O	PLANNING & ZONING COMMISSION
	APPLICATION
1.	NAME OF APPLICANT: 3115 Fairfield Ave LLC
2.	Is the Applicant's name Trustee of Record? Yes No X
	If yes, a sworn statement disclosing the Beneficiary shall accompany this application upon filing.
3.	Address of Property: 3115, 3129 and 3135 Fairfield Ave., 704 Courtland Ave. and 30 Clarkson St. / CT / 06605
	(number) (street) (state) (zip code)
4.	Assessor's Map Information: Block No. 8/107 Lot No. 1/A, 2, 24, 25 & 26
5.	Amendments to Zoning Regulations: (indicate) Article: <u>N/A</u> Section:
	(Attach copies of Amendment)
6.	Description of Property (Metes & Bounds): See submitted survey; 213.04' x 170.00' x 104.50' x 43.61' x 101.00' x 175.00'
7.	Existing Zone Classification: O-R and R-B
8.	Zone Classification requested: N/A
9.	Describe Proposed Development of Property: Proposed construction of a mixed-use building with ground floor
	retail use and residential multi-family apartment to contain 52 dwelling units and associated Site improvements.
	Approval(s) requested: Coastal Site Plan Review and Site Plan Review
	14.1
	Signature: Date: 12/23/2021
	Signature: Date: 12/23/2021
	If signed by Agent, state capacity (Lawyer, Developer, etc.) Signature:
	Drint Names
	Print Name:
	Mailing Address: _/o Chris Russo, Russo & Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06824
	Mailing Address:       c/o Chris Russo, Russo & Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06824         Phone:       203-528-0590         Cell:       203-520-4603
	Mailing Address: _/o Chris Russo, Russo & Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06824
	Mailing Address:       c/o Chris Russo, Russo & Rizio, LLC, 10 Sasco Hill Road, Fairfield, CT 06824         Phone:       203-528-0590       Cell:       203-520-4603       Fax:         E-mail Address:       Chris@russorizio.com       Fax:       Cell:       Cell:
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Tel 203-309-5500

www.russorizio.com

December 23, 2021

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\* Also Admitted in NY \* Also Admitted in VT + Of Counsel

Dennis Buckley Zoning Administrator Zoning Department 45 Lyon Terrace Bridgeport, CT 06604

Re: Petition for Site Plan Review and Coastal Site Plan Review – 3115, 3129 & 3135 Fairfield Ave., 704 Courtland Ave & 30 Clarkson Street

Dear Mr. Buckley:

Please accept this Petition to the Bridgeport Planning and Zoning Commission for Site Plan Review and Coastal Site Plan Review on behalf of my client, 3115 Fairfield Ave LLC, for the properties located at 3115, 3129 & 3135 Fairfield Ave., 704 Courtland Ave & 30 Clarkson Street (the "Site") in the O-R and R-B Zones.

#### Proposed Development & Use

The Petitioner proposes to construct a single mixed-use retail and residential multifamily apartment dwelling on the Site with associated Site improvements. The proposed building for the Site is located entirely within the O-R Zone and a portion of the proposed parking area is located in the R-B Zone. The entire Site is located within the coastal boundary with Ash Creek as the nearest coastal resource over Six hundred feet (600'+) away from the Site. Finally, the Site is also located within the Commercial Village Overlay District ("CVOD"). The Site has frontage on Fairfield Avenue, Courtland Avenue and Clarkson Street. The Site is predominantly vacant except for an existing two-family dwelling and accessory structures. The Petitioner proposes to demolish all these structures. Restaurants, a vehicle repair shop, an office building, a large apartment building, another mixed-use building and several two-family structures currently surround the Site. Retail uses with under 10,000 SF of floor area and multi-family dwellings are a permitted use within the O-R Zone. The Petitioner proposes to construct a Five-story mixed use building with Three thousand one hundred and seventy square feet (3,170 SF) of ground floor retail use and a multi-family residential apartment use containing Fifty-two (52) residential dwelling units.

The Site will be accessed via entrance/exit driveways on Courtland Avenue and Clarkson Street. The Petition proposes a parking area located behind the proposed building for a total of Fifty-two (52) off-street parking spaces. As the proposed building and use is located within the O-R Zone, there is no parking requirement for the proposed use, so the Petition is well in compliance with the Regulations. The proposed retail use will occupy Three thousand one hundred and seventy square feet (3,170 SF) of ground floor area. It is proposed to be separated into Two (2) separate units facing and with sole access to Fairfield Avenue. In compliance with Section 9-5-5.5 of the Regulations, the retail use is dominated by window area on its façade to meet the standards of the CVOD.

The balance of the building will contain a multi-family apartment use. A number of amenities are proposed for the Site, including a lobby, tenant lounge, mail area, fitness center, office space, roof deck and a green roof. The residential floors will be accessed on the ground floor room multiple points, including from the parking area and sidewalk on Fairfield Avenue. The upper residential floors will be accessed via Two (2) stairwells and an elevator. The proposed building will contain Four (4) studio, Twenty-five (25) one-bedroom and Twenty-three (23) two-bedroom dwelling units. A typical studio dwelling unit will range in size from 460 SF to 532 SF and contain a full kitchen, living/dining room and open bedroom area, walk-in/storage closet, washer/dryer, roof deck and full bath. A typical one-bedroom dwelling unit will range in size from 593 SF to 690 SF and will contain a private bedroom in addition to the studio unit features. The two-bedroom dwelling units will range in size from 974 SF to 1028 SF and will feature an additional bedroom and full bath.

The submitted elevations show a variety of materials, colors and depths consistent with apartment design found in new construction throughout the City and surrounding area. The Site will be connected via public sidewalks to the convenient Fairfield Avenue corridor. A significant amount of landscaping will be added to the Site with plantings along the rear property line as a buffer to residential properties and street trees along the street frontages in accordance with the CVOD Regulations. Existing structures along the rear property line will also be removed. The Petition will be a tremendous improvement to the Site and neighborhood to provide new construction housing to Bridgeport residents.

#### Site Plan Review

The Petition satisfies the Section 14-2-5 Site Plan Review standards of the Regulations. The design of the proposed buildings and landscaping create a harmonious building-street interaction providing a tremendous improvement to the existing streetscape. The scale and proportion of the buildings conform to the O-R Zone Development Standards and the CVOD as it is fully compliant with the Regulations. The Petition proposes significant landscaping along the rear property line and street frontages. The proposed retail and multi-family residential dwelling uses and its density are permitted in the O-R Zone. The

proposed uses and building replace a dated dwelling and vacant overgrown land on an underutilized Site. The Site is in close proximity to another high-density apartment building, so the proposed use will be in conformity with the area.

As stated above, the proposed design of the building and its location on the Fairfield Avenue corridor will be a great asset for residents of the neighborhood. The Petition proposes more adequate off-street parking and accessible spaces than required under the Regulations. The Petition conforms to the permitted standards under the Regulations.

#### Coastal Site Plan Review

The Petition also complies with Section 14-3 of the Regulations regarding coastal site plan review. While the Site is located within the coastal boundary, it is over Six hundred feet (600'+) from Ash Creek, which is the nearest coastal resource. Dozens of buildings and multiple streets and blocks exist between the coastal resource and the Site. It has no connection to the coastal resource but for being included within its boundary. There are no natural features associated with the coastal resource on the Site. As stated above, the Petition fully complies with the site plan review standards of the Regulations. The Petition poses no danger or threat to coastal resources and it has no potential adverse impacts. The proposed building and Site improvements will all be constructed in accordance with current codes and regulations, including appropriate stormwater drainage systems. Appropriate sediment and erosion controls, such as silt fencing and anti-tracking aprons, will be utilized during construction and stockpiles will be located at the rear of the Site.

For these reasons, we respectfully request approval of the Petition to construct a single mixed-use retail and residential multi-family apartment dwelling containing Fifty-two (52) dwelling units on the Site with associated Site improvements.

Sincerely,



# 54CITY OF BRIDGEPORT

Application Form **Municipal Coastal Site Plan Review** For Projects Located Fully or Partially Within the Coastal Boundary

Please complete this form in accordance with the attached instructions (CSPR-INST-11/99) and submit it with the appropriate plans to the Zoning office.

#### Section I: Applicant Identification

Applicant:3115 Fairfield Ave LLCDate:12/23/2021Address:C/O Russo & Rizio, LLC, 10 Sasco Hill Rd, Fairfield, CTPhone:203-528-0590
Project Address or <sub>Location:</sub> 3115, 3129 & 3135 Fairfield Ave, 704 Courtland Ave and 30 Clarkson St, Bridgeport, CT
Interest in Property:
List primary contact for correspondence if other than applicant: Name: Chris Russo, Russo & Rizio, LLC Address: 10 Sasco Hill Road
City/Town: <u>Fairfield</u> State: <u>CT</u> Zip Code: 06824
Business Phone: 203-528-0590
e-mail: Chris@russorizio.com

#### Section II: Project Site Plans

Please provide project site plans that clearly and accurately depict the following information, and check the appropriate boxes to indicate that the plans are included in this application:

RProject location

KExisting and proposed conditions, including buildings and grading

KCoastal resources on and contiguous to the site

 $\Gamma$  High tide line [as defined in CGS Section 22a-359(c)] and mean high water mark elevation

contours (for parcels abutting coastal waters and/or tidal wetlands only)

XSoil erosion and sediment controls

K Stormwater treatment practices

K Ownership and type of use on adjacent properties

Reference datum (i.e., National Geodetic Vertical Datum, Mean Sea Level, etc.)

#### Section III: Written Project Information

Please check the appropriate box to identify the plan or application that has resulted in this Coastal Site Plan Review:

▶ Site Plan for Zoning Compliance

Γ Subdivision or Resubdivision

 $\Gamma$  Special Permit or Special Exception

Γ Variance

Γ Municipal Project (CGS Section 8-24)

#### Part I: Site Information

1.	Street Address or Geographical Description:			
	3115, 3129 & 3135 Fairfield Ave, 704 Courtland Ave and 30 Clarkson St			
	City or Town: Bridgeport			
2.	Is project or activity proposed at a waterfront site (includes tidal wetlands frontage)? ΓYES 🕱ΝΟ			
3.	Name of on-site, adjacent or downstream coastal, tidal or navigable waters, if applicable: Ash Creek is located over 600' from the Site. There is no adjacent water.			
4.	structures,	d describe the existing land use on and adjacent to the site. Include any existing municipal zoning classification, significant features of the project site: ntly contains mostly vacant land, a two-family dwelling and accessory structures. The Site is located in the O-R and R-B Zones.		
	The Site i	s surrounded by a mix of commercial uses, including restaurants, a vehicle repair		
	shop, an	d office, and multi-family residential dwellings, including an apartment		
	building.			
5.	Indicate the	e area of the project site: <u>35,704</u> acres or square feet (circle one)		
6.	Check the	appropriate box below to indicate total land area of disturbance of the project or activity		
	(please als	o see Part II.B. regarding proposed stormwater best management practices):		
	Г	Project or activity will disturb 5 or more total acres of land area on the site. It may be		
		eligible for registration for the Department of Environmental Protection's (DEP) General		
		Permit for the Discharge of Stormwater and Dewatering Wastewaters Associated with		
		Construction Activities		
	Г	Project or activity will disturb one or more total acres but less than 5 total acres of land		
		area. A soil erosion and sedimentation control plan must be submitted to the municipal		
		land use agency reviewing this application.		
	~			

- Project or activity will not disturb 1 acre total of land area. Stormwater management controls may be required as part of the coastal site plan review.

#### Part II.A.: Description of Proposed Project or Activity

Describe the proposed project or activity including its purpose and related activities such as site clearing, grading, demolition, and other site preparations; percentage of increase or decrease in impervious cover over existing conditions resulting from the project; phasing, timing and method of proposed construction; and new uses and changes from existing uses (attach additional pages if necessary):

The Petitioner proposes to demolish the existing buildings on the Site and construct a five-story mixed-use building with ground floor retail space and Fifty-two (52) residential dwelling units. The Petitioner will construct a street level parking area to provide sufficient parking for the development. The proposed grading is shown on the submitted plan. The proposed building and site coverage is below the maximum standards of the zone under the Zoning Regulations. The development will be completed in one phase in an anticipated Eighteen (18) months of construction.

#### Part II.B.: Description of Proposed Stormwater Best Management Practices

Describe the stormwater best management practices that will be utilized to ensure that the volume of runoff generated by the first inch of rainfall is retained on-site, especially if the site or stormwater discharge is adjacent to tidal wetlands. If runoff cannot be retained on-site, describe the site limitations that prevent such retention and identify how stormwater will be treated before it is discharged from the site. Also demonstrate that the loadings of total suspended solids from the site will be reduced by 80 percent on an average annual basis, and that post-development stormwater runoff rates and volumes will not exceed pre-development runoff rates and volumes (attach additional pages if necessary):

Storm water run-off from the building and the driveway and parking areas will be treated with a subsurface system. The primary stormwater treatment will be implemented as to Stormwater Best Management Practice.

10

#### Part III: Identification of Applicable Coastal Resources and Coastal Resource Policies

Identify the coastal resources and associated policies that apply to the project by placing a check mark in the appropriate box(es) in the following table.

Coastal Resources	On-site	Adjacent	Off-site but within the influence of project	Not Applicable
General Coastal Resources* - Definition: CGS Section 22a-93(7); Policy: CGS Section 22a-92(a)(2)	X	×	$\times$	
Beaches & Dunes - Definition: CGS Section 22a-93(7)(C); Policies: CGS Sections 22a-92-(b)(2)(C) and 22a-92(c)(1)(K)				X
Bluffs & Escarpments - Definition: CGS Section 22a-93(7)(A); Policy: CGS Section 22a-92(b)(2)(A)				X
Coastal Hazard Area - Definition: CGS Section 22a-93(7)(H); Policies: CGS Sections 22a-92(a)(2), 22a-92(a)(5), 22a-92(b)(2)(F), 22a- 92(b)(2)(J), and 22a-92(c)(2)(B)				×
Coastal Waters, Estuarine Embayments, Nearshore Waters, Offshore Waters - Definition: CGS Sections 22a-93(5), 22a-93(7)(G), and 22a- 93(7)(K), and 22a-93(7)(L) respectively; Policies: CGS Sections 22a-92(a)(2) and 22a-92(c)(2)(A)				X
Developed Shorefront - Definition: CGS Section 22a-93(7)(I); Policy: 22a-92(b)(2)(G)				×
Freshwater Wetlands and Watercourses - Definition: CGS Section 22a-93(7)(F); Policy: CGS Section 22a-92(a)(2)				X
Intertidal Flats - Definition: CGS Section 22a-93(7)(D); Policies: 22a-92(b)(2)(D) and 22a-92(c)(1)(K)				X
Islands - Definition: CGS Section 22a-93(7)(J); Policy: CGS Section 22a-92(b)(2)(H)				X
Rocky Shorefront - Definition: CGS Section 22a-93(7)(B); Policy: CGS Section 22a-92(b)(2)(B)				X
Shellfish Concentration Areas - Definition: CGS Section 22a-93(7)(N); Policy: CGS Section 22a-92(c)(1)(I)			0	X
Shorelands - Definition: CGS Section 22a-93(7)(M); Policy: CGS Section 22a-92(b)(2)(I)				×
Tidal Wetlands - Definition: CGS Section 22a-93(7)(E); Policies: CGS Sections 22a-92(a)(2), 22a-92(b)(2)(E), and 22a- 92(c)(1)(B)				×

\* General Coastal Resource policy is applicable to all proposed activities

#### Part IV: Consistency with Applicable Coastal Resource Policies and Standards

Describe the location and condition of the coastal resources identified in Part III above and explain how the proposed project or activity is consistent with all of the applicable coastal resource policies and standards; also see adverse impacts assessment in Part VII.A below (attach additional pages if necessary): Ash Creek, which is the closest coastal resource to the Site, is located over 600' from the Site. The proposed project complies with CGS Sec. 22a-92(a)(1) "...by promoting economic growth without significantly disrupting the environment...", with CGS Sec. 22a-92(b)(2)(F) "...manage coastal hazard areas to minimize hazards to property..." and with CGS Sec. 22a-92(c)(2)(B) "...maintain patterns of water circulation in the placement of drainage control structures..."

#### Part V: Identification of Applicable Coastal Use and Activity Policies and Standards

Identify all coastal policies and standards in or referenced by CGS Section 22a-92 applicable to the proposed project or activity:

- $\times$  General Development\* CGS Sections 22a-92(a)(1), 22a-92(a)(2), and 22a-92(a)(9)
- Water-Dependent Uses\*\* CGS Sections 22a-92(a)(3) and 22a-92(b)(1)(A);
   Definition CGS Section 22a-93(16)
- 9 Ports and Harbors CGS Section 22a-92(b)(1)(C)
- 9 Coastal Structures and Filling CGS Section 22a-92(b)(1)(D)
- 9 Dredging and Navigation CGS Sections 22a-92(c)(1)(C) and 22a-92(c)(1)(D)
- 9 Boating CGS Section 22a-92(b)(1)(G)
- 9 Fisheries CGS Section 22a-92(c)(1)(I)
- 9 Coastal Recreation and Access CGS Sections 22a-92(a)(6), 22a-92(C)(1)(j) and 22a-92(c)(1)(K)
- 9 Sewer and Water Lines CGS Section 22a-92(b)(1)(B)
- 9 Fuel, Chemicals and Hazardous Materials CGS Sections 22a-92(b)(1)(C), 22a-92(b)(1)(E) and 22a-92(c)(1)(A)
- 9 Transportation CGS Sections 22a-92(b)(1)(F), 22a-92(c)(1)(F), 22a-92(c)(1)(G), and 22a-92(c)(1)(H)
- 9 Solid Waste CGS Section 22a-92(a)(2)
- 9 Dams, Dikes and Reservoirs CGS Section 22a-92(a)(2)
- 9 Cultural Resources CGS Section 22a-92(b)(1)(J)
- 9 Open Space and Agricultural Lands CGS Section 22a-92(a)(2)

<sup>\*</sup> General Development policies are applicable to all proposed activities

<sup>\*\*</sup> Water-dependent Use policies are applicable to all activities proposed at waterfront sites, including those with tidal wetlands frontage.

#### Part VI: Consistency With Applicable Coastal Use Policies And Standards

Explain how the proposed activity or use is consistent with all of the applicable coastal use and activity policies and standards identified in Part V. For projects proposed at waterfront sites (including those with tidal wetlands frontage), particular emphasis should be placed on the evaluation of the project's consistency with the water-dependent use policies and standards contained in CGS Sections 22a-92(a)(3) and 22a-92(b)(1)(A) -- also see adverse impacts assessment in Part VII.B below (attach additional pages if necessary):

No adverse impacts were determined on adjacent coastal resources. Stormwater treatment is proposed which will help reduce

erosion impacts as well as provide water infiltration. This project will be limited to the confines of the Site and will be completed within

Eighteen (18) months. All disturbed pervious areas will be loamed, seeded and planted upon completion of construction.

#### Part VII.A.: Identification of Potential Adverse Impacts on Coastal Resources

Please complete this section for all projects.

Identify the adverse impact categories below that apply to the proposed project or activity. The Aapplicable≅ column **must** be checked if the proposed activity has the **potential** to generate any adverse impacts as defined in CGS Section 22a-93(15). If an adverse impact may result from the proposed project or activity, please use Part VIII to describe what project design features may be used to eliminate, minimize, or mitigate the potential for adverse impacts.

Potential Adverse Impacts on Coastal Resources	Applicable	Not Applicable
Degrading tidal wetlands, beaches and dunes, rocky shorefronts, and bluffs and escarpments through significant alteration of their natural characteristics or functions - CGS Section 22a-93(15)(H)		×
Increasing the hazard of coastal flooding through significant alteration of shoreline configurations or bathymetry, particularly within high velocity flood zones - CGS Section 22a-93(15)(E)		×
Degrading existing circulation patterns of coastal water through the significant alteration of patterns of tidal exchange or flushing rates, freshwater input, or existing basin characteristics and channel contours - CGS Section 22a-93(15)(B)		×
Degracing watter with the open of the significant alteration of groundwater flow and recharge and volume of runoff - CGS Section 22a-93(15)(D)		×
Degrading natural erosion patterns through the significant alteration of littoral transport of sediments in terms of deposition or source reduction - CGS Section 22a-93(15)(C)		×
Degrading visual quality through significant alteration of the natural features of vistas and view points - CGS Section 22a-93(15)(F)	X	×
Degrading water quality through the significant introduction into either coastal waters or groundwater supplies of suspended solids, nutrients, toxics, heavy metals or pathogens, or through the significant alteration of temperature, pH, dissolved oxygen or salinity - CGS Section 22a-93(15)(A)		X
Degrading or destroying essential wildlife, finfish, or shellfish habitat through significant alteration of the composition, migration patterns, distribution, breeding or other population characteristics of the natural species or significant alterations of the natural components of the habitat - CGS Section 22a-93(15)(G)		×

#### Part VII.B.: Identification of Potential Adverse Impacts on Water-dependent Uses

Please complete the following two sections only if the project or activity is proposed at a waterfront site:

 Identify the adverse impact categories below that apply to the proposed project or activity. The Aapplicable≅ column must be checked if the proposed activity has the potential to generate any adverse impacts as defined in CGS Section 22a-93(17). If an adverse impact may result from the proposed project or activity, use Part VIII to describe what project design features may be used to eliminate, minimize, or mitigate the potential for adverse impacts.

Potential Adverse Impacts on Future Water-dependent Development Opportunities and Activities	Applicable	Not Applicable
Locating a non-water-dependent use at a site physically suited for or planned for location of a water-dependent use - CGS Section 22a-93(17)		×
Replacing an existing water-dependent use with a non-water- dependent use - CGS Section 22a-93(17)		×
Siting a non-water-dependent use which would substantially reduce or inhibit existing public access to marine or tidal waters - CGS Section 22a-93(17)		×

2. Identification of existing and/or proposed Water-dependent Uses

Describe the features or characteristics of the proposed activity or project that qualify as water-
dependent uses as defined in CGS Section 22a-93(16). If general public access to coastal waters is
provided, please identify the legal mechanisms used to ensure public access in perpetuity, and describe
any provisions for parking or other access to the site and proposed amenities associated with the access
(e.g., boardwalk, benches, trash receptacles, interpretative signage, etc.)*:
There is no proposed activity that will qualify as a water-dependent use as there is no
adjacent water within 600' of the Site.
If there are no water dependent use components, describe how the project site is not expressive for the

\*If there are no water-dependent use components, describe how the project site is not appropriate for the development of a water-dependent use.

#### Part VIII: Mitigation of Potential Adverse Impacts

Explain how all potential adverse impacts on coastal resources and/or future water-dependent development opportunities and activities identified in Part VII have been avoided, eliminated, or minimized (attach additional pages if necessary):

No adverse impacts were determined on adjacent coastal resources. Stormwater treatment

is proposed which will help reduce erosion impacts as well as provide water infiltration. New

lawn areas will also reduce erosion and provide storm water infiltration.

#### Part IX: Remaining Adverse Impacts

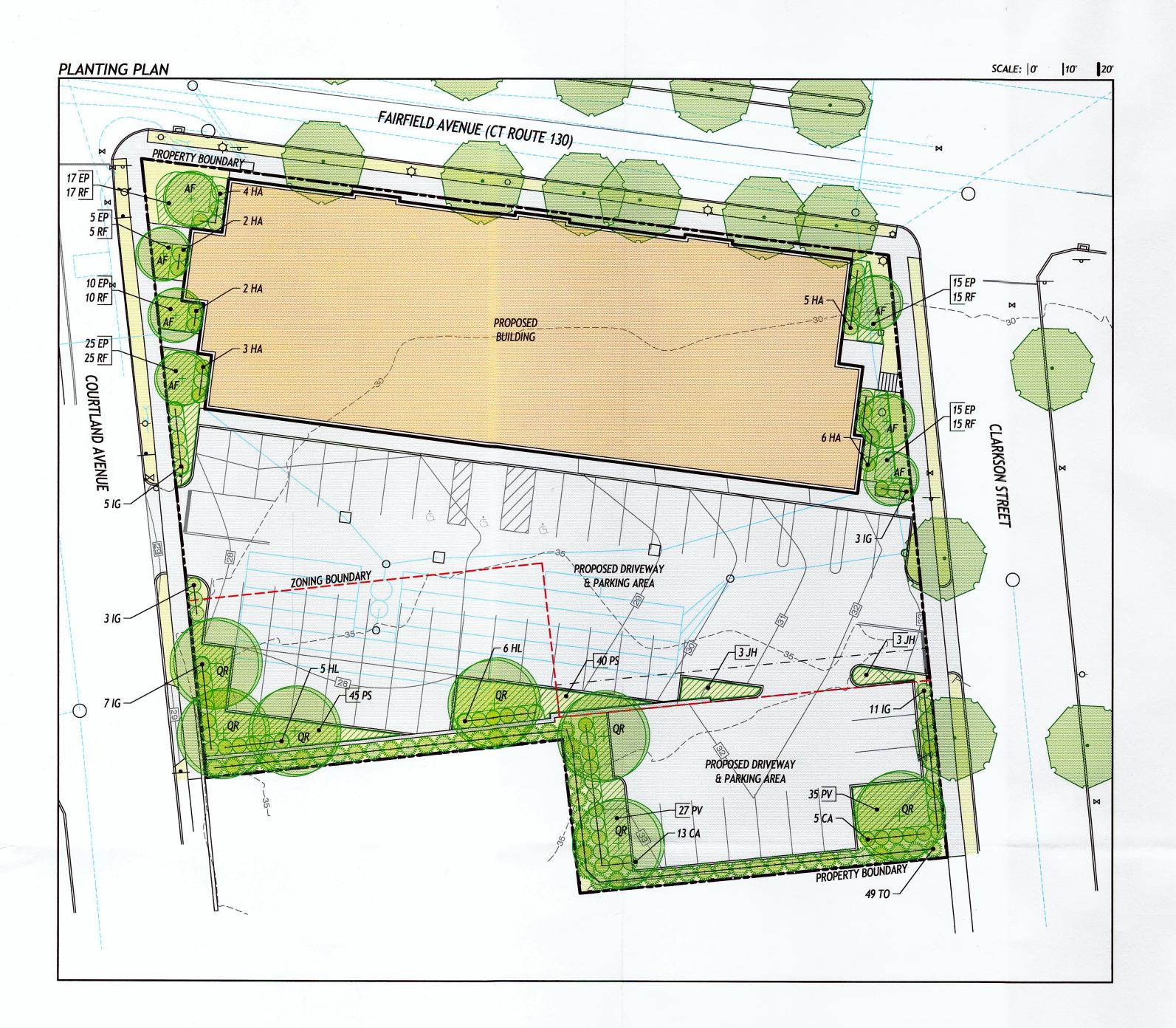
Explain why any remaining adverse impacts resulting from the proposed activity or use have not been mitigated and why the project as proposed is consistent with the Connecticut Coastal Management Act

(attach additional pages if necessary):

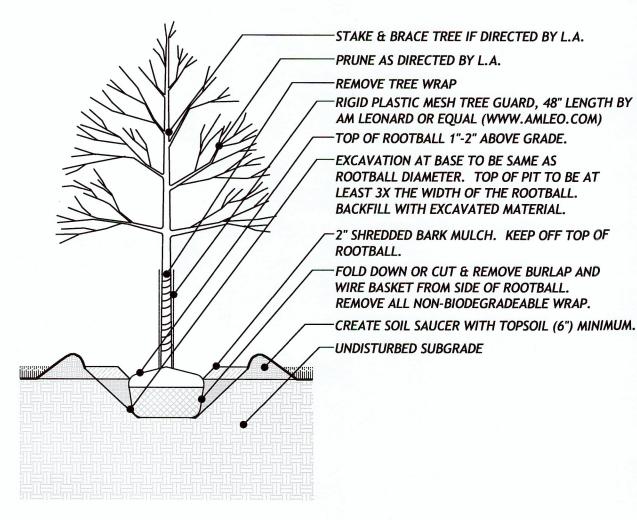
There will be no remaining adverse impacts resulting from the proposed activity.

LIST
of Pro
ROPERTY
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LIST OF PROPERTY OWNERS WITHIN 100' OF 3115, 3129 & 3135 FAIRFIELD AVE, 704 COURTLAND
5, 3129
& 3135
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50 CLARKSON ST 42 CLARKSON ST	686 COURTLAND AV 674 COURTLAND AV #676	16 MONTGOMERY ST 675 COURTLAND AV #679	41 CLARKSON ST	3083 FAIRFIELD AV #3085 29 CLARKSON ST	3171 FAIRFIELD AV	<b>3135 FAIRFIELD AV</b>	<b>3129 FAIRFIELD AV</b>	<b>3115 FAIRFIELD AV</b>	689 COURTLAND AV	704 COURTLAND AV	<b>30 CLARKSON ST</b>	<b>3104 FAIRFIELD AV</b>	<b>3150 FAIRFIELD AV</b>	<b>3126 FAIRFIELD AV</b>	<b>3120 FAIRFIELD AV</b>	<b>3142 FAIRFIELD AV</b>	40 CLARKSON ST	694 COURTLAND AV	8 MONTGOMERY ST #10	LOCATION
CORREA JEAN E MARGUERITE FRATARCANGELI REVOCABLE TRUST	WASHBURN THOMAS L ET AL PAVEL PEARL	AQUILA PROPERTIES LLC VITORINO JAMES	MCCARTHY MOLLY & STEPHEN MCCABE III	DEPARLE JUDITH & RICHARD	3171 FAIRFIELD AVENUE LLC	3115 FAIRFIELD AVE LLC	3115 FAIRFIELD AVE LLC	3115 FAIRFIELD AVE LLC	PEKAR MARGE	3115 FAIRFIELD AVE LLC	3115 FAIRFIELD AVE LLC	NRK LLC	FORMATO JOSEPH	LORA KARSYS VENTURA	<b>KERSTETTER GERALDINE &amp; RICHARD</b>	THRESHER HUGH G	MARGUERITE FRATARCANGELI REVOCABLE TRUST	BASJAH JOHN & FORTUNATA	ETIENNE DEAN & JEAN	OWNER
50 CLARKSON ST 42 CLARKSON ST	686 COURTLAND AV 152 WAKEMAN LN	32 SUGAR PLUM LN 675 COURTLAND AV #679	41 CLARKSON ST	3083 FAIRFIELD AVE	3255 FAIRFIELD AVE	15 AMERIC AVE, STE 110	15 AMERIC AVE, STE 110	15 AMERIC AVE, STE 110	000679 COURTLAND AVE	15 AMERIC AVE, STE 110	15 AMERIC AVE, STE 110	<b>3104 FAIRFIELD AVE</b>	3870 BLACK ROCK TPK	<b>3126 FAIRFIELD AVE</b>	<b>3120 FAIRFIELD AVE</b>	42879 SPINKS FERRY RD	40 CLARKSON ST	694 COURTLAND AVE	8 MONTGOMERY ST #10	MAILING ADDRESS
BRIDGEPORT BRIDGEPORT	BRIDGEPORT SOUTHPORT	FAIRFIELD BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	LAKEWOOD	LAKEWOOD	LAKEWOOD	BRIDGEPORT	LAKEWOOD	LAKEWOOD	BRIDGEPORT	FAIRFIELD	BRIDGEPORT	BRIDGEPORT	LEESBURG	BRIDGEPORT	BRIDGEPORT	BRIDGEPORT	CITY
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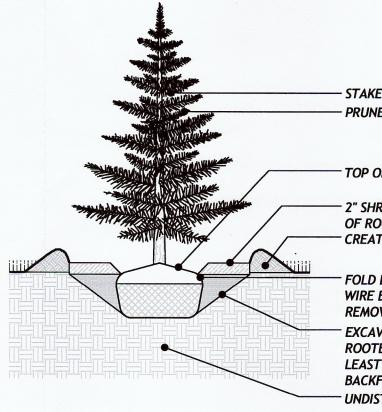


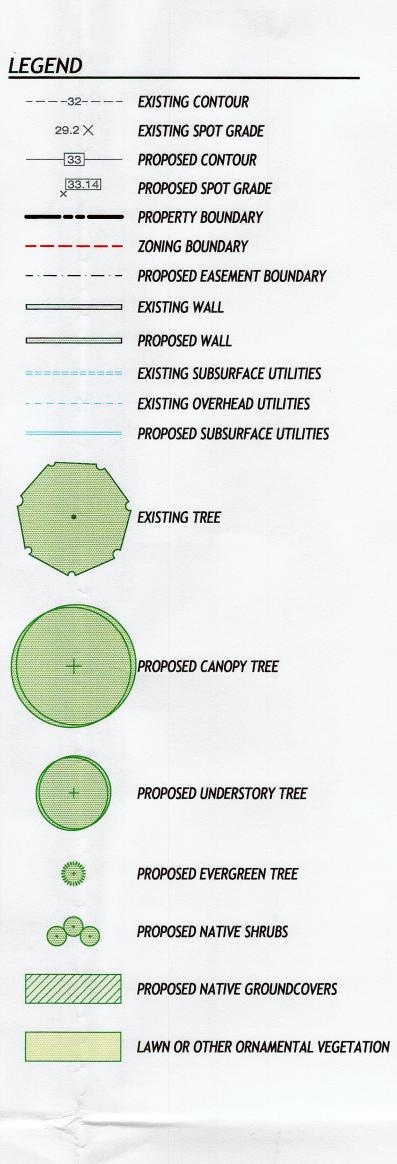
# TREE PLANTING DETAIL



NOT TO SCALE

# EVERGREEN TREE PLANTING DETAIL





NOT TO SCALE

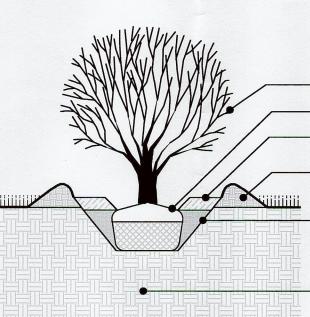
– STAKE & BRACE TREE IF DIRECTED BY L.A. – PRUNE AS DIRECTED BY L.A.

- TOP OF ROOTBALL 1"-2" ABOVE GRADE.

 2" SHREDDED BARK MULCH. KEEP OFF TOP OF ROOTBALL.
 CREATE SOIL SAUCER WITH TOPSOIL (6") MIN.

FOLD DOWN OR CUT & REMOVE BURLAP AND WIRE BASKET FROM SIDE OF ROOTBALL. REMOVE ALL NON-BIODEGRADEABLE WRAP. EXCAVATION AT BASE TO BE SAME AS ROOTBALL DIAMETER. TOP OF PIT TO BE AT LEAST 3X THE WIDTH OF THE ROOTBALL. BACKFILL WITH EXCAVATED MATERIAL.

# SHRUB PLANTING DETAIL



NOT TO SCALE

-PRUNE AS DIRECTED BY L.A.

-TOP OF ROOTBALL 1"-2" ABOVE GRADE. -2" SHREDDED BARK MULCH. KEEP OFF TOP OF ROOTBALL.

-CREATE SOIL SAUCER WITH TOPSOIL (6") MINIMUM. -EXCAVATION AT BASE TO BE SAME AS ROOTBALL DIAMETER. TOP OF PIT TO B

ROOTBALL DIAMETER. TOP OF PIT TO BE AT LEAST 3X THE WIDTH OF THE ROOTBALL. BACKFILL WITH EXCAVATED MATERIAL. -UNDISTURBED SUBGRADE

# **GENERAL NOTES**

1. UNLESS NOTED OTHERWISE, EXISTING AND ALL OTHER PROPOSED CONDITIONS INFORMATION TAKEN FROM A DRAWING PREPARED BY THE HUNTINGTON COMPANY, LLC.

2. PROPOSED PLANTING INFORMATION PROVIDED BY WILLIAM KENNY ASSOCIATES LLC.

# PLANT LIST

SYM.	QTY.		SCIENTIFIC NAME	COMMON NAME	SIZE	ROOT
PROPOSE	D NATI	E CANOPY	TREES			
QR	7	QUERCUS	RUBRA	RED OAK	4" CAL.	B&B
TOTAL	7					
PROPOSE	D NATIN	E UNDERST	ORY TREES			
AF	7	ACER X FR	EEMANII 'ARMSTRONG'	ARMSTRONG MAPLE	4" CAL.	B&B
TOTAL	7					
PROPOSE	ED EVER	GREEN TREE	s			
то	49	THUJA OC	CIDENTALIS	AMERICAN ARBORVITAE	6'-7' HT.	B&B
TOTAL	49					
PROPOSE	D NATI	/E SHRUBS				
CA	18	CLETHRA A	ALNIFOLIA 'SIXTEEN CANDLES'	SIXTEEN CANDLES SUMMERSWEET	2'-3' HT.	CONTAINER
HA	22	HYDRANGE	A ARBORESCENS 'INCREDIBALL'	INCREDIBALL SMOOTH HYDRANGEA	2'-3' HT.	CONTAINER
HL	11	HYDRANGE	A ARBORESCENS 'LIMETTA'	LIMETTA SMOOTH HYDRANGEA	2'-3' HT.	CONTAINER
IG	29	ILEX GLAB	RA 'DENSA'	DENSA INKBERRY	2'-3' HT.	CONTAINER
TOTAL	80					
PROPOSE	D NATIV	E GROUND	COVERS			
EP	87	ECHINACE	A PURPUREA 'POWWOW WHITE'	POWWOW WHITE CONEFLOWER	2 QUART	CONTAINER
JH	5	JUNIPERUS	HORIZONTALIS 'BAR HARBOR'	BAR HARBOR CREEPING JUNIPER	2 QUART	CONTAINER
PV	62	PANICUM	/IRGATUM 'CAPE BREEZE'	CAPE BREEZE SWITCHGRASS	2 QUART	CONTAINER
PS	85	PANICUM	/IRGATUM 'SHENANDOAH'	SHENANDOAH SWITCHGRASS	2 QUART	CONTAINER
RF	87	RUDBECKI	A FULGIDA 'EARLY BIRD GOLD'	BLACK-EYED SUSAN	2 QUART	CONTAINER
TOTAL:	326					

#### PLANTING NOTES

- 1. PROPOSED TREE AND SHRUB LOCATIONS TO BE ADJUSTED IN FIELD AS NEEDED BASED ON FIELD CONDITIONS.
- 2. PLANT SPACING FOR HERBACEOUS MATERIAL TO BE 24" O.C.
- 3. BOTANICAL NAMES SHALL PREVAIL OVER COMMON NAMES.
- 4. ALL PLANT MATERIAL SHALL BE NURSERY GROWN; NO COLLECTED MATERIALS SHALL BE ACCEPTED, UNLESS SPECIFICALLY INDICATED.
- 5. PLANTS SHALL CONFORM TO THE AMERICAN ASSOCIATION OF NURSERYMEN STANDARDS IN ALL WAYS INCLUDING DIMENSIONS.
- 6. THE LANDSCAPE ARCHITECT HAS THE RIGHT TO REJECT ANY PLANT MATERIALS UPON DELIVERY TO THE
- PROJECT. SELECTION BY THE LANDSCAPE ARCHITECT DOES NOT WAIVE THE RIGHT OF REJECTION.
  ALL REPLACEMENTS SHALL BE PLANTS OF THE SAME KIND AND SIZE AS SPECIFIED IN THE PLANT LIST OR AS NECESSARY TO MATCH SURVIVING PLANTS OF THE SAME PLANTING GROUP. ALL COSTS SHALL BE BORN BY THE LANDSCAPE CONTRACTOR EXCEPT FOR REPLACEMENTS RESULTING FROM LOSS OR DAMAGE DUE TO VANDALISM OR ACTS OF NEGLECT ON THE PART OF OTHERS, PHYSICAL DAMAGE, BY ANIMALS, VEHICLES, FIRE, ETC., AS MAY BE DETERMINED BY THE LANDSCAPE ARCHITECT.
- 8. ALL PLANT MATERIAL SHOULD BE PLACED, OR LOCATION STAKED, ON THE SITE AS SHOWN ON THE PLANTING PLAN PRIOR TO COMMENCEMENT OF PLANT EXCAVATION FOR THE LANDSCAPE ARCHITECT'S APPROVAL. THE CONTRACTOR MUST NOTIFY THE LANDSCAPE ARCHITECT OF ALL PLANTING OPERATIONS A MINIMUM OF 48 HOURS IN ADVANCE.
- 9. ALL PLANT MATERIALS SHALL BE BALLED AND BURLAPPED OR CONTAINER GROWN OR AS OTHERWISE SPECIFIED. NO CONSTRUCTED BALLS SHALL BE ACCEPTED. REMOVE SYNTHETIC 'BURLAP' AND SYNTHETIC TWINES AND ROPES. REMOVE TOP 1/3 OF METAL BASKETS FROM ROOT BALLS WHEN THE ROOT BALL HAS BEEN POSITIONED IN THE PLANTING PIT. PROVIDE SUPPORT AS NECESSARY TO PROTECT THE ROOT BALL FROM INJURY DURING THIS OPERATION.

# PLANTING PLAN

# PREPARED FOR: MAGNICO CONTRACTING LOCATION:

3125 FAIRFIELD AVENUE BRIDGEPORT, CONNECTICUT

DATE: 10/8/21 - 1 10/27/21 - 2 12/22/21 - 3 SCALE: AS NOTED REF NO. 5017

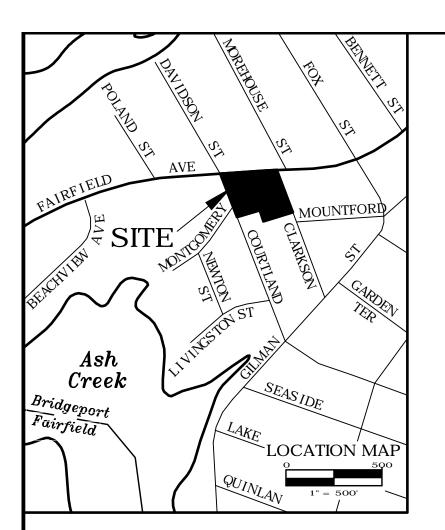




# WILLIAM KENNY ASSOCIATES

LANDSCAPE ARCHITECTURE = ECOLOGICAL SERVICES

1899 Bronson Road Fairfield CT 06824 203 366 0588 www.wkassociates.net



#### SOIL TESTING:

**Test Hole #1 (6/30/15)** Broken ledge @ 1.5 ft. Ledge @ 4 ft.

**Test Hole #2 (6/30/15)** Broken Rock Fill 8 ft. to grey silt/clay Total depth 9 ft.

**Test Hole #3 (6/30/15)** Ledge @ 3 ft.

**Test Hole #4 (6/30/15)** Ledge @ 6 ft. (east) Ledge @ 7 ft. (west)

**Test Hole #5 (6/30/15)** Total depth 7 to 8 ft.

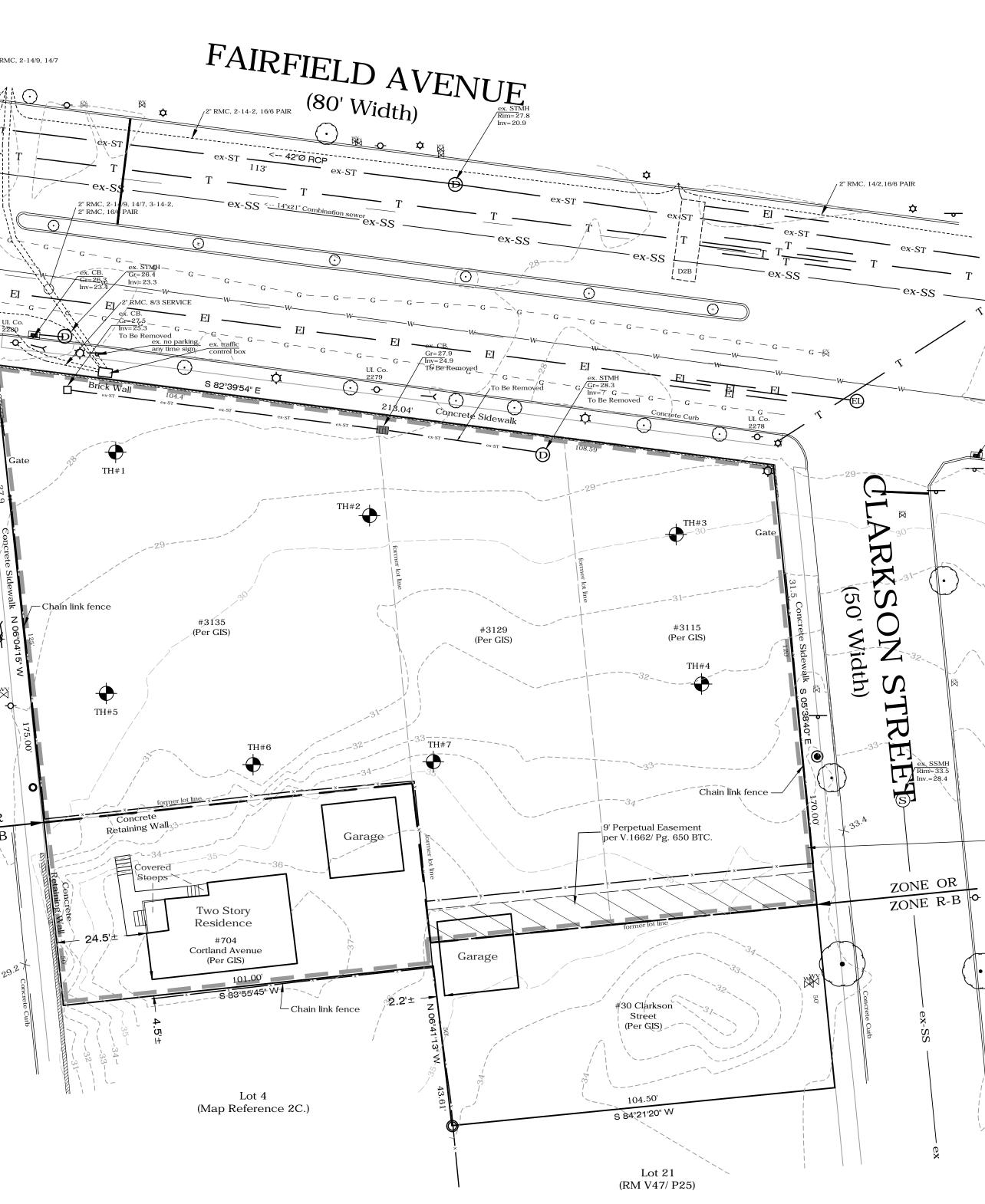
**Test Hole #6 (6/30/15)** Ledge @ 6 ft.

**Test Hole #7 (6/30/15)** Ledge @ grade ±

# $\begin{array}{l} \underline{Percolation Test:} \\ \hline 6/21/11 \\ Depth = 3.0 \ ft. \\ Pre-soak @ 10:00 \\ Time: Reading: Drop: \\ 11:05.....1.20 \ ft. \\ 11:15.....1.43 ".....0.23 \ ft. \\ 11:25.....1.53 "....0.10 " \\ 11:35.....1.62 "....0.09 " \\ 11:45.....1.69 "....0.07 " \\ 11:55.....1.75....0.06 " \\ 12:05.....1.81....0.06 \ ft. \\ Drop \ of \ 0.05 \ ft. \ in 10 \ minutes = \\ PERC \ RATE = 1 \ inch \ in 13 \ minutes \end{array}$

ex. STMH Gr=26.3 Inv=20.4(e) $Gr=25.9\pm$ $Inv=22.2\pm$ D=-5	x) G /G	7±         ex. SSMFI           Rim=26.9±         Inv.=19.5-ex           Inv.=19.5-ex         Inv.=           G         S         G           G         S         G           G         S         G           G         S         G           N         S         S <t< th=""><th>G - G - G - G - G - G - G - G - G - G -</th><th>ex-SS</th></t<>	G - G - G - G - G - G - G - G - G - G -	ex-SS
	(50' Width)	SS-x9 0 0 0 0 0	403' 15" Tile>SS-xa	27.9 Concrete Succes
	L B			DNE OR DNE R-B

	LEGEND
x	Chain Link Fence
	Picket Fence
$\mathbf{)}$	Iron Pin, Brass Plug
	Drill Hole
þ	Lamp Post
	Catch Basin
Ň	Water Valve
<i>k</i>	Fire Hydrant
3	Manhole
	Test Hole
T	



NOT VALID UNLESS EMBOSSED WITH SEAL OR FIXED WITH THE LIVE STAMP OF THE SIGNATORY TO MY KNOWLEDGE AND BELIEF, THIS MAP IS SUBSTANTIALLY CORRECT AS NOTED HEREON

Jason T. Spath Sr., L.S. #70136

## NOTES:

- 1. This survey and map has been prepared in accordance with the Sections 20-300b-1 through 20-300b-20 of the Regulations of Connecticut State Agencies "Minimum Standards for Survey and Maps in the State of Connecticut" as endorsed by the Connecticut Association of Land Surveyors, Inc. It is a Data Accumulation Plan based upon a Resurvey and conforms to Horizontal Accuracy Class A-2 and Topographical Class T-2.
- 2. Reference is made to the following maps:
- A. "Revised Map No. 3 of Property Belonging To The Estate Of Caroline Clarkson. Situated in Bridgeport, Conn" Novermber 1918, Revised March 1925. Scale 1"= 30'
- B. "Map of Fairfield Avenue Estates" September 1915 Prepared by Palmer and Goodell, Surveyors
- C. "Map of Property For Phoebe M. Clarkson, Bridgeport, Conn; Dec. 19, 1981"; Scale 1"=20' Prepared by The Huntington Company; Vol.47/ Pg.25 BTC.
- D. Block Maps from the Bridgeport Engineering Department.
- 3. Reference is made to the following deeds:
- A. Vol. 423/ Pg. 249 Building Restrictions (#704 Courtland Ave.)
- B. Vol. 1662/ Pg. 650 Perpetual Easement (Lots 22,23,24 RM V47/P25)
- 1. The underground utilities shown, if any, have been located from visible field survey information. The surveyor makes no guarantees that the underground utilities shown comprise all such utilities in the area either in service or abandoned. The surveyor further does not warrant that the underground utilities shown are in the exact location indicated. The surveyor has not physically located the underground utilities, unless specifically noted as such. It is the Contractor's responsibility to contact CALL BEFORE YOU DIG (CBYD) prior to commencement of any excavation, Dial 811 or 1-800-922-4455.

Location and Depths of underground utilites within the Proposed Pipe Crossing Area Have been provided by ACS Underground Solutions

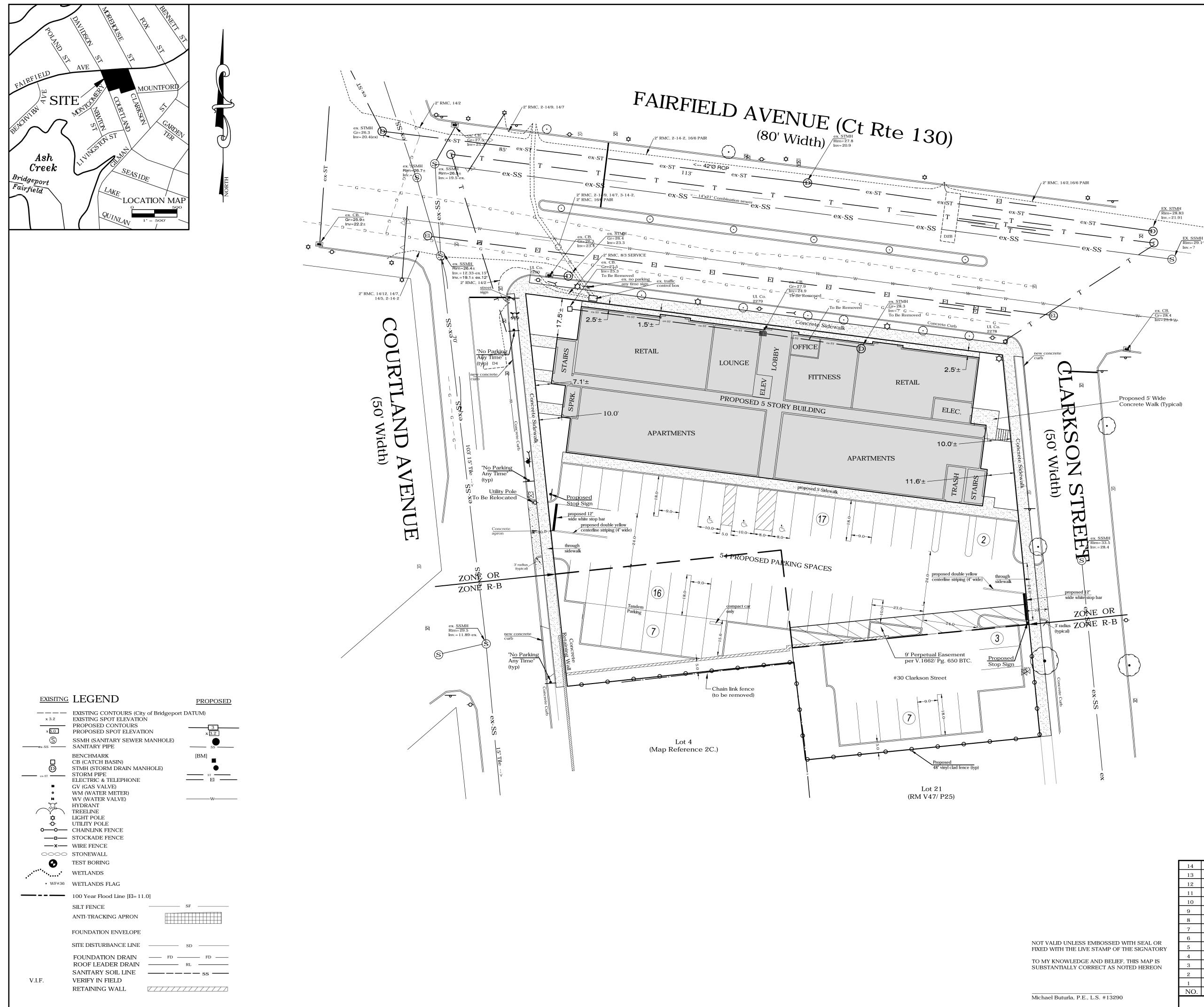
- 5. Property is located in FEMA Zone X. Per Flood Insurance Rate Map #09001C0438G, Effective Date: July 8, 2013; Panel 438 of 626.
- 6. Property is located in Zone OR and R-B.
- 7. Reference is hereby made to Connecticut General Statute 8-13a, as amended, with regards to existing structures three or more years old.
- 8. Total Lot Area = 35,704 S.F.±, 0.820 Ac.±

9. Closure 1/5000 or better.

10. Underground traffic control features shown per map entitled:
"State Of Connecticut Department Of Transportation Bureau Of Engineering & Hwy. Operations Division Of Traffic Engineering, Traffic Control Signal Layout, City Of Bridgeport, Route 130 (Fairfield Ave.) At Davidson Street And Cortland Ave.; Scale: 1"=40' ". Traffic Control Signal Plan For Intersection 015-341.

#### LIMIT OF PREVIOUSLY APPROVED SITE PLAN

13         12         11         10         9         8         7         6         5         4	12-22-21 8-23-21 12-17-19 11-22-19 10-16-19 9-18-19 10-28-14 9-08-14 8-18-14 7-31-14	Revise Building Modify Site Plan CT DOT Comments 12-10-19 Underground Utility Info Added CT DOT Comments 10-8-19 State of CT comments revise parking & details additional landscaping RC zoning table rev. parking & bldg.		/IAGNIC #312	CUMULAT PREPARED FOR O CONTI 5 FAIRFIELE ORT, CONN $0 = 1^{\circ} = 20^{\circ}$	RACTINO DAVE IECTICUT	
3 2	6-01-14 5-28-14	rev. parking & lot rev. parking & lot	DATE: JAN. 9, 2014	SCALE: 1"= 20'	DRAFTER: MSC	JOB NUMBER: 9205	JECT #: 9205
2 1 NO.	1-22-14 DATE	zoning table DESCRIPTION REVISIONS	HC	Co	HUNTINGTON COM onsulting Engineers & Linwood Avenue, Fa 203.259.1091	Surveyors airfield, CT	1/7



Development Standards	Zone OR Requi <b>r</b> ed	<b>Pr</b> opose <b>d</b>
<b>M</b> inimum Lot <b>Ar</b> ea	5,000 sf	35,704 sf
<b>M</b> inimum <b>W</b> i <b>d</b> th	35'	213±
Streetwall Minimum (Primary)	75%	83.4%
St <b>r</b> eetwall <b>M</b> aximum	100%	83.4%
Streetwall Minimum(Secondary	/) 30%	44.0%
St <b>r</b> eetwall <b>M</b> aximum	100%	48.1%
<b>M</b> inimum Buil <b>d</b> ing <b>S</b> etbacks		
Street Lot Line	0'	1.5±
Lot Line	0' / 5' *	NA
Rear Lot Line	None **	79.3'±
Maximum Building Setbacks		
Street Lot Line	10'	10.0'
<b>M</b> inimum Lot Cove <b>r</b> age	75%	84.9% (30,328 / 35,704
Maximum Lot Coverage	None	84.9% (30,328 / 35,704
Maximum Height	65 ft	59.4'±
-	5 Stories	5
Floor To Ceiling Height First Floor	12 ft	12'+
(Height based on information prov * 5' If side yard is utilized ** 20' If floor contains habitable sp		icant & Architect)

LANDSCAPING required 15%

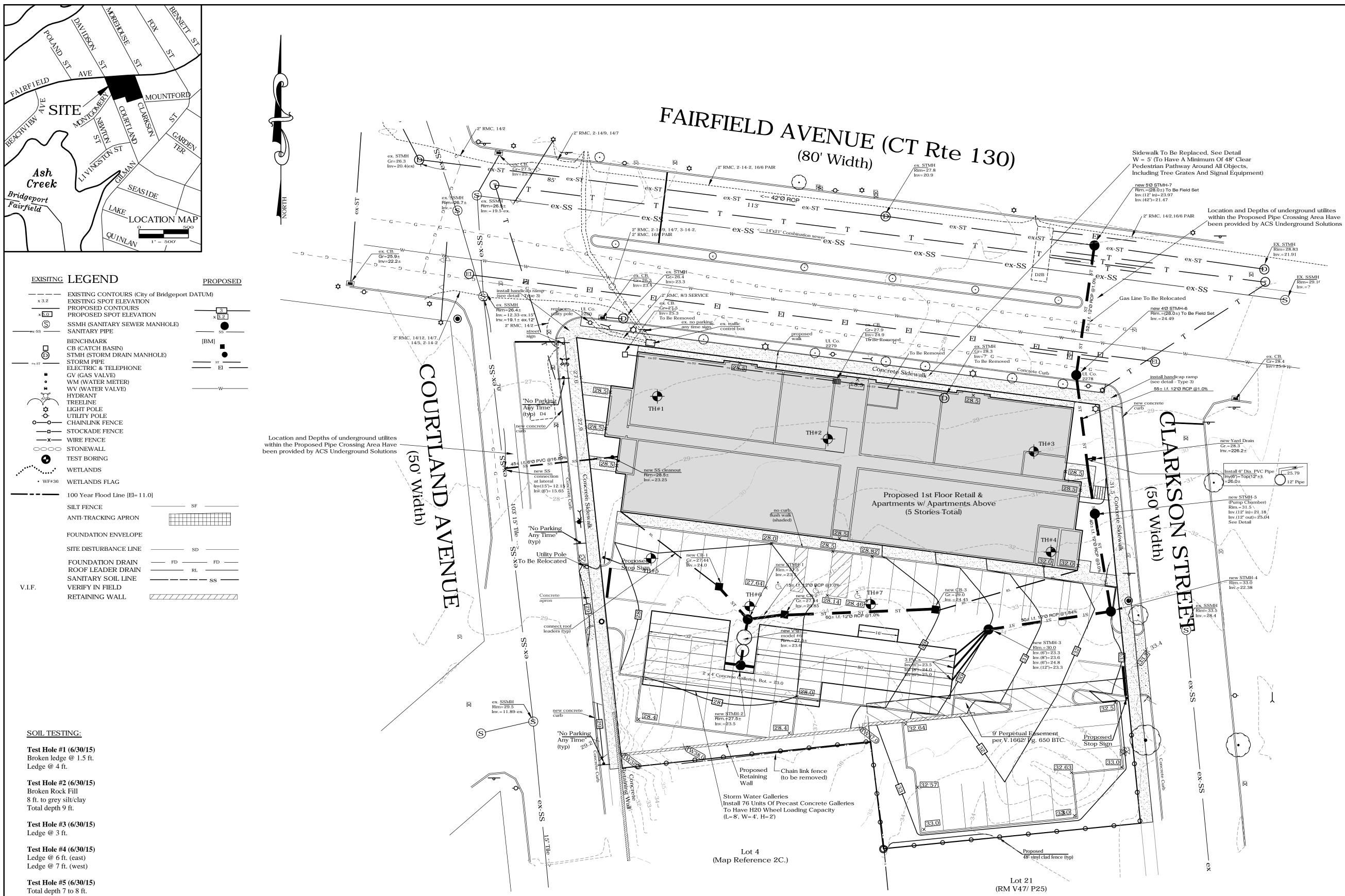
proposed 15.1% (5,376 / 35,704)

PARKING 52 Units Proposed

49 Parking Spaces 3 Handicap Spaces

Total = 52 Parking Spaces Proposed

AL OR VATORY	$     \begin{array}{r}       14 \\       13 \\       12 \\       11 \\       10 \\       9 \\       8 \\       7 \\       6 \\       5 \\       4 \\       4     \end{array} $	12-22-21 10-27-21 8-23-21 12-17-19 11-22-19 10-16-19 9-18-19 10-28-14 9-08-14 8-18-14 7-31-14	Revise Building Revise Parking Modify Site Plan CT DOT Comments 12-10-19 Underground Utility Info Added CT DOT Comments 10-8-19 State of CT comments revise parking & details additional landscaping RC zoning table rev. parking & bldg.		IAGNIC( #3125 F	YOUT PLA PREPARED FOR O CONTR CAIRFIELD A ORT, CONN $0 = 1^{\circ} = 20^{\circ}$	RACTING	۱ ۳	
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				HCO INFO: FB#49	90:25,482:29,557:1	[BPT-56 C52Q		E-5	93



Test Hole #6 (6/30/15) Ledge @ 6 ft.

Test Hole #7 (6/30/15) Ledge @ grade ±

Percolation Test:

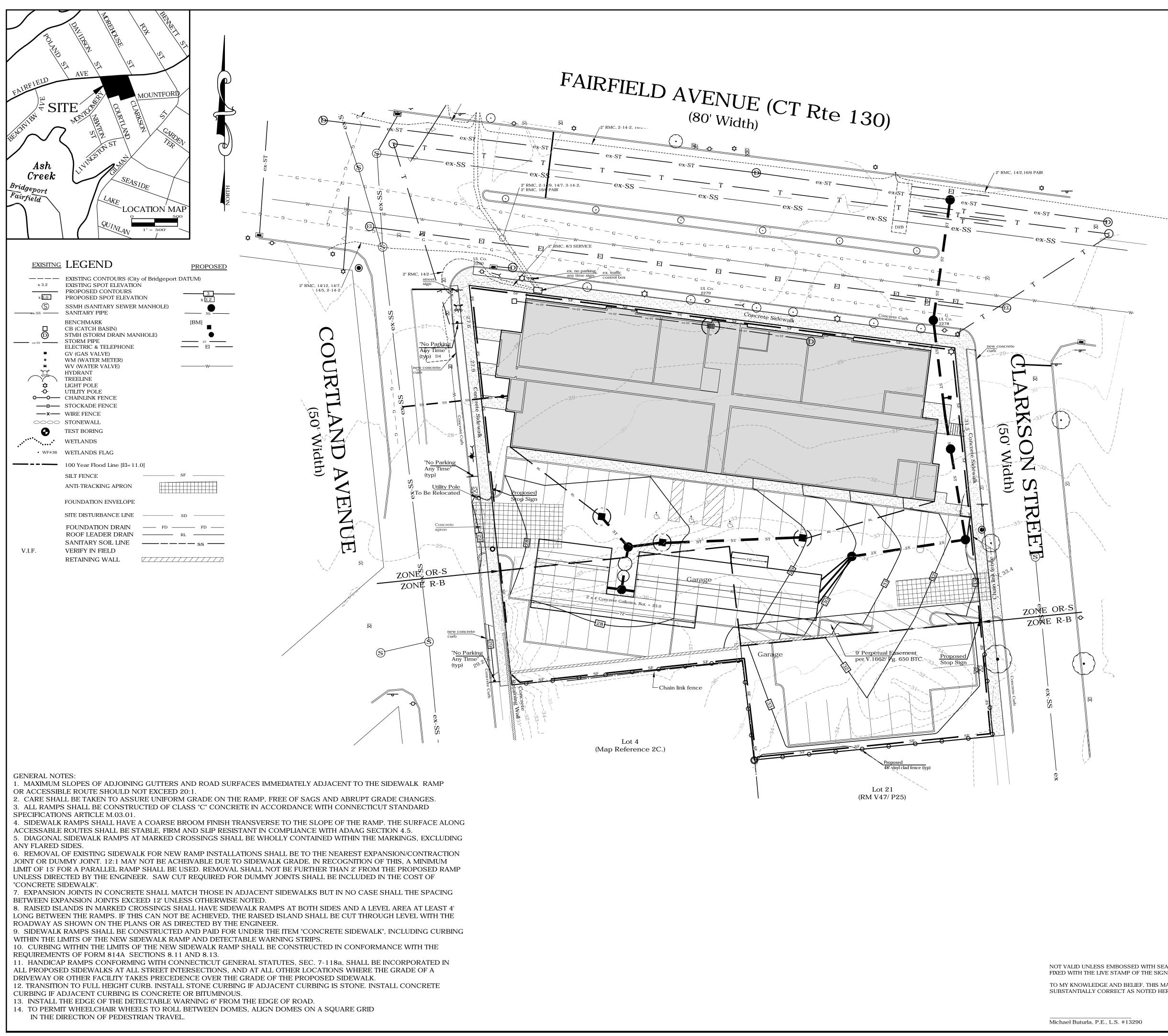
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## NOTES:

- 1. Trim trees along Fairfield Avenue to provide 7' clearance over sidewalk.
- 2. The underground utilities shown, if any, have been located from visible field survey information. The surveyor makes no guarantees that the underground utilities shown comprise all such utilities in the area either in service or abandoned. The surveyor further does not warrant that the underground utilities shown are in the exact location indicated. The surveyor has not physically located the underground utilities, unless specifically noted as such. It is the Contractor's responsibility to contact CALL BEFORE YOU DIG (CBYD) prior to commencement of any excavation, Dial 811 or 1-800-922-4455.
- 3. The permittee shall contact the Department's District Survey Unit Mr. Vincent Hanchuruck at (203) 389-3112 prior to any construction within State Right Of Way.
- 4. The permittee will be responsible for all engineering costs should the CTDOT boundary/ survey markers be disturbed or damaged.
- 5. In the event the Department determines the subject CDOT boundary/survey markers need to be replaced due to the proposed development, the Department will furnish new Monuments, which the permittee will be required to install under the direction of a Connecticut licensed surveyor.
- 6. The CDOT boundary / survey markers shall be verified and accepted by the District 3 survey unit prior to releasing the encroachment permit bond.
- 7. The Department of Transportation will secure a Drainage Connection Concurrence for the proposed drainage connection. The actual Drainage Connection Concurrence document will be finalized during the permit issuance phase for the property owner's signature. The completed document shall be recorded in the town land records. A certified copy of the recording must be received by Neil Creem, District 3 Drainage Engineer, Pond Lily Avenue, New Haven, CT 06515 prior to the release of the bond for the project.

			REVISIONS		30	3 Linwood Avenue, F 203.259.1091			1 1
	1 NO.	1-22-14 DATE	zoning table DESCRIPTION		C	HUNTINGTON CO onsulting Engineers &	2 Surveyors	2	/7
	2	5-28-14	rev. parking & lot	JAN. 9, 2014	1"=20'	whj	9205	9205	
AP IS REON	3	6-01-14	rev. parking & lot	DATE:	SCALE:	DRAFTER:	JOB NUMBER:	PROJECT	#:
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	12	8-23-21	Modify Site Plan					r	
	13	10-27-21	Revise Parking	1		PREPARED FOR			
]	14	12-22-21	Revise Building	1	S	SITE PLAN	V		



Sediment and Erosion Control Notes

1. Prior to the start of construction, a preconstruction meeting with the engineer is required

2. Actual locations and applications of erosion control devices shall be determined in the field prior to the start of construction based on the erosion and sediment control strategy. The strategy will require the contractor to follow the general sequence of construction, provide appropriate controls such as structural practices, maintenance, and stabilization practices along with the proper discharge of dewatering wastewaters. The contractor must follow the general permit for the discharge of stormwater

and dewatering wastewaters associated with construction activities.

3. Limits of disturbance shall be flagged in the field and verified prior to initiation of construction.

4. Erosion and sediment control devices shall be installed prior to any site or building demolition at the site. All erosion and sediment control measures shall be constructed in accordance with the standard and specifications of the State of Ct. Dep "Guidelines for Erosion and Sediment Control "Handbook, January, 1985, or as amended. 5. All sediment and erosion control measures shall be installed and functioning prior to any site disturbance. Additional measures may be required during

the course of construction and shall be implemented as needed. No activity is to begin until the site monitor has been notified. All sediment and erosion

control measures are to be inspected prior to a heavy rain, immediately after and at least daily during prolonged rains. 6. All graded areas with slopes steeper than 3 horizontal to 1 vertical shall be stabilized with jute netting.

a) land grading:

i) areas to be filled shall be cleared, grubbed and stripped of unsuitable material.

ii) all fills shall be compacted as required to reduce erosion slippage. Settlement, subsidence. Or other related problems. iii)fill material shall be free of brush, rubbish, rocks, logs, stumps, building debris and other unsuitable materials that would interfere with or prevent construction of satisfactory fills.

7. When all graded areas are permanently stabilized. Remove all erosion and sediment controls. Remove trapped sediment

8. It shall be the responsibility of the Owner and the site development contractor to ensure proper implementation of the soil erosion and sediment controls as shown on this plan; and shall include but not be limited to installation and maintenance of control measures, informing all parties of such requirements and notification of any transfer of this responsibility to other parties.

9. Any disturbed area and piles planned to be left more than 14 days will have to be seeded or mulched immediately. Recommended seed mixture: Future 2000 by the Chas C. Hart containing the following varieties of perennial Ryegrass: Fiesta ii, Blazer ii, Dasher ii, and Express. A seeding rate of 5-7 pounds per 1,000 square feet is recommended.

10. When all surfaces are permanently stabilized, any remaining sediment and erosion control devices shall be removed and all trapped sediment shall be removed. All catch basin sumps shall be cleaned.

11.Construction activities at the project site will result in emissions of fugitive dust to the atmosphere. The quantity of fugitive dust generated will be

controlled but is dependent upon weather conditions. Fugitive dust particles have a greater propensity to become airborne during dry and breezy meteorological conditions. Construction activities at the site which will result in the generation of fugitive dust include which will result in the generation of fugitive dust include grading, material loading and unloading, material storage piles and construction traffic. The contractor will implement the following reasonable precautions during construction to minimize the generation of fugitive dust:

a) use water for dust control of active construction areas, active unpaved roads, and other surfaces which can give rise to airborne dust. A typical practice to be followed during site grading will be to follow the earth moving equipment with a water truck to immediately wet the new disturbed area. b)apply seed for a vegetative cover on storage piles, especially those that will remain dormant for an extended period.

c)apply the binder course of paving material to site drives and parking lots as soon as feasible during construction.

d)the contractor must clean/sweep daily all on-site paved roads and that portion of any surrounding roads which are used by construction traffic, for the duration of the project.

e)institute a maximum on site speed limit of 10 miles per hour.

f)the contractor is responsible for dust control during the construction process. The construction manager shall inspect the site to assure dust is adequately controlled. If the construction manager or owners representative feels dust control measures are not adequate the contractor shall be required to increase these measures as directed by the construction manager.

12.All construction activities shall comply with the City of Bridgeport Zoning regulations.

13.Dewatering procedures shall be conducted in a manner that insures no dewatering waste water is directly discharged into any wetland or waterbody. Dewatering wastewaters must be discharged in a manner which will not cause erosion and scouring or contain suspended solids in amounts which could reasonably be expected to cause pollution of the waters of the state. The measures shall be conducted in accordance with the dewatering plan submitted by the contractor as part of the contract documents. Dewatering wastewaters shall be discharged in a manner to minimize the discoloration of the receiving waters. Unless otherwise specifically approved, all dewatering wastewaters shall be infiltrated into the ground.

14. A stockpile of sediment and erosion controls shall be kept on site at all times. This will consist of at least 24 hay bales, under cover, extra stone for the anti-tracking apron, at least 100 feet of silt fence and 100 square yards of non-woven filter fabric additional measures may be required by the site monitor. These measures are to be installed by the request date. Replace construction entrance when the capacity of the apron has reached the 50% full volume. 15.Sediment removed from control structures will be disposed of in a manner which is consistent with the intent of these plans.

16.Where construction activities have permanently ceased or have temporarily been suspended for more than seven days, or when final grades are reached in any portion of the site, stabilization practices shall be implemented within three days.

17. The contractor is responsible for stormwater discharges and must submit a revised general permit registration to Connecticut Department of Environmental Protection prior to the start of construction.

18. The contractor must prepare a plan which conforms to the stormwater pollution control plan approved by the

Connecticut Department of Environmental Protection. The plan must be approved the A/E and will be prepared at the contractors own expense. The contractor must sign and cause to be signed by each appropriate subcontractor, the "certification statement" required by the general permit. 19. The contractor, during construction, shall inspect the site in conformance with the general permit, including and inspection at least once every seven days and within 24 hours of the end of a storm that is 0.5 inch or greater.

General Sequence of Construction

1. A copy of "Connecticut Guidelines for Soil and Sediment Control" should be on site for reference. 2. The limits of the new construction and limits of grading are to be staked by a licensed land surveyor.

3. Clear the proposed driveway to the grading limits. It is strongly recommended that the wood and brush chips be saved for sediment and erosion control. Brush and trees less than 6" in diameter may be chipped for use as mulch.

4. The construction entrance(s) shall be installed at the locations as shown on the plans. The sediment control system (silt fence. hay bales, temporary swales shall be placed at the locations as shown on the plans. Install an anti-tracking apron per attached detail. The limits of construction are to be clearly marked whether with silt or barrier fence or some other approved means. This applies to the silt and barrier fence, staked hay bales, the anti-tracking apron, stone dams and other protection which might be required due to site conditions.

5. All trees and brush in the area of the new grading shall be cut.

6. a. Stockpile areas for topsoil and extra material are to be ringed, on the downhill side with silt fence, staked hay bales and another approved system of containment. Piles to be left over two weeks should be seeded with a quick grow grass mix. This is to control erosion by both rain and wind. 7. Proceed with cuts and fills for access driveway, maintaining and adding any additional sediment and erosion controls which might be needed due to field conditions and pending weather. Rough grade proposed driveway and stabilize area.

8. Proceed with cuts and fills for parking area and building site. Rough grade proposed parking area and stabilize

9. The loam shall be stripped and stockpiled in a level area on the site. Stumps shall be removed and disposed of at a legal landfill off-site. The loam stockpiles shall be ringed with silt fence. These rings shall be maintained during the period that materials are stored. The earth excavation shall be done to bring the roadway, structures, shoulders and slope areas to subgrade levels. The slopes shall be stabilized with temporary vegetation (vt) as soon as possible after the completion of the earthwork.

10. Demolish existing structures and appurtenances.

11. Begin new building foundation construction. Use graded parking area as staging area for building construction.

12. As building construction proceeds construct stormwater detention structures and municipal utilities and appurtenances.

13. Install drainage structures. As drainage structures are completed they must be protected with hay bales, silt fence, silt sacks or other approved means. If it is necessary to dewater the area, it must be done in an approved manner. This could be achieved by pumping into a portable sediment control container, into an approved sediment basin, filter bags or by other acceptable means. If any turbidity occurs, which affects the regulated area, the pumping is to

cease immediately. 14. The drainage pipe shall be laid to the grades and elevations as shown on the plans starting from the downstream section first. The catch basin frames shall be adjusted to the finish grade elevations as shown on the plans.

15. The catch basins shall be protected with silt fencing or hay bales as shown on the plans.

16. The gravel base shall be placed in the roadway in accordance with the plans and specifications.

17. The first course of pavement and the curbing shall be placed.

18. The topsoil and seed shall be applied to the shoulders and all disturbed slope areas. 19. The second course of pavement shall be placed.

20. As soon as possible, disturbed areas are to be stabilized. On a temporary basis this could mean temporary Seeding, hay mulch, wood chips, netting or whatever method site condition might dictate.

21. Maintenance of all sediment and erosion controls is to be ongoing. Replacement and repairs are to be done immediately.

22. Complete cuts and fills, final grade, pave, and install curbing per specifications.

23. Regrade and restore stockpile and all other disturbed areas.

24. Remove all sediment and erosion controls once the site has been deemed stable

Temporary vegetation schedule

Provide not less than the following quantities of specified materials.

1. 4" topsoil

2. 135 lbs. of lime per 1000 sq. Ft.

3. 7.5 lbs. of commercial fertilizer per 1000 sq, ft.

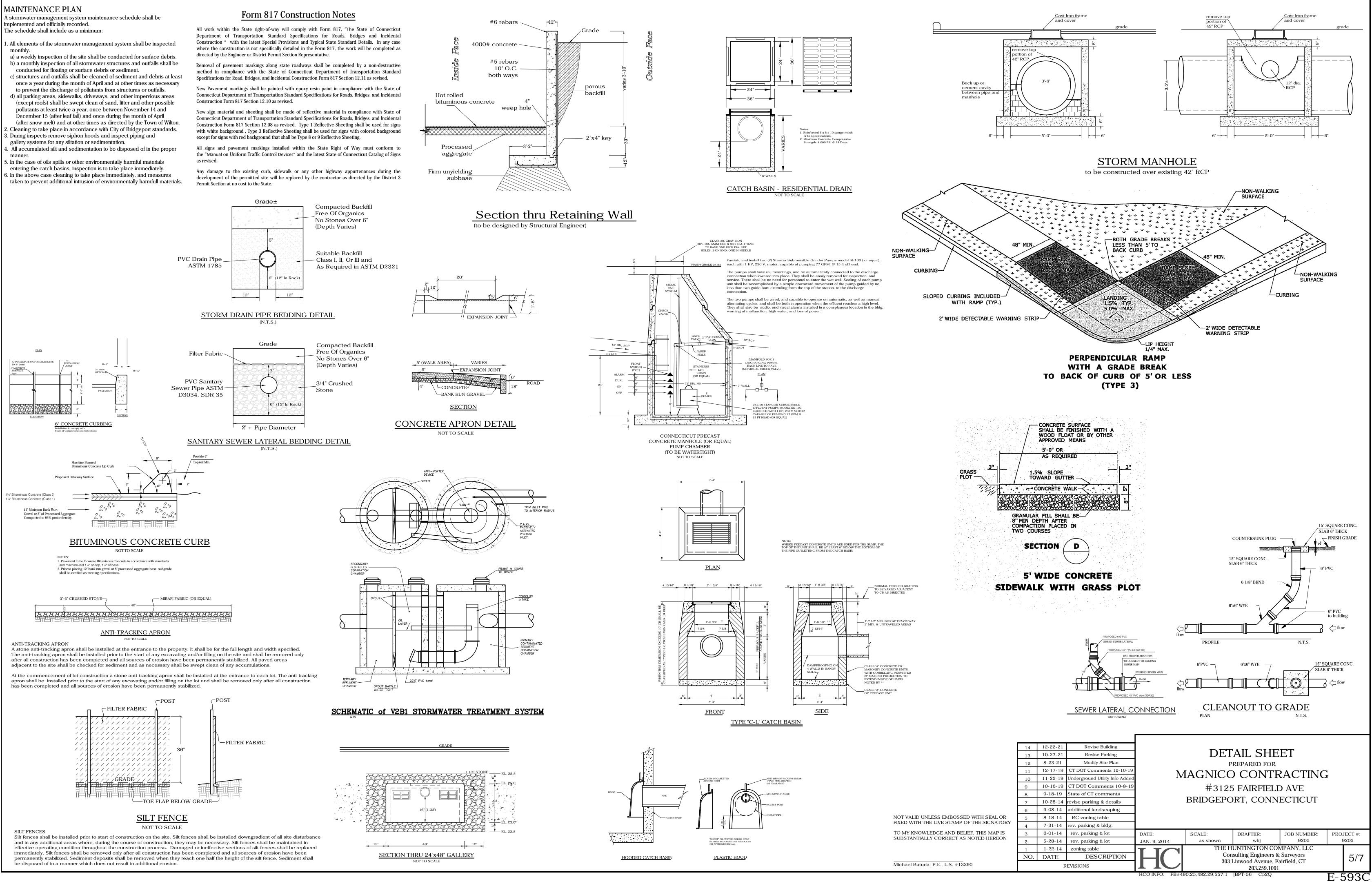
4. Seed:

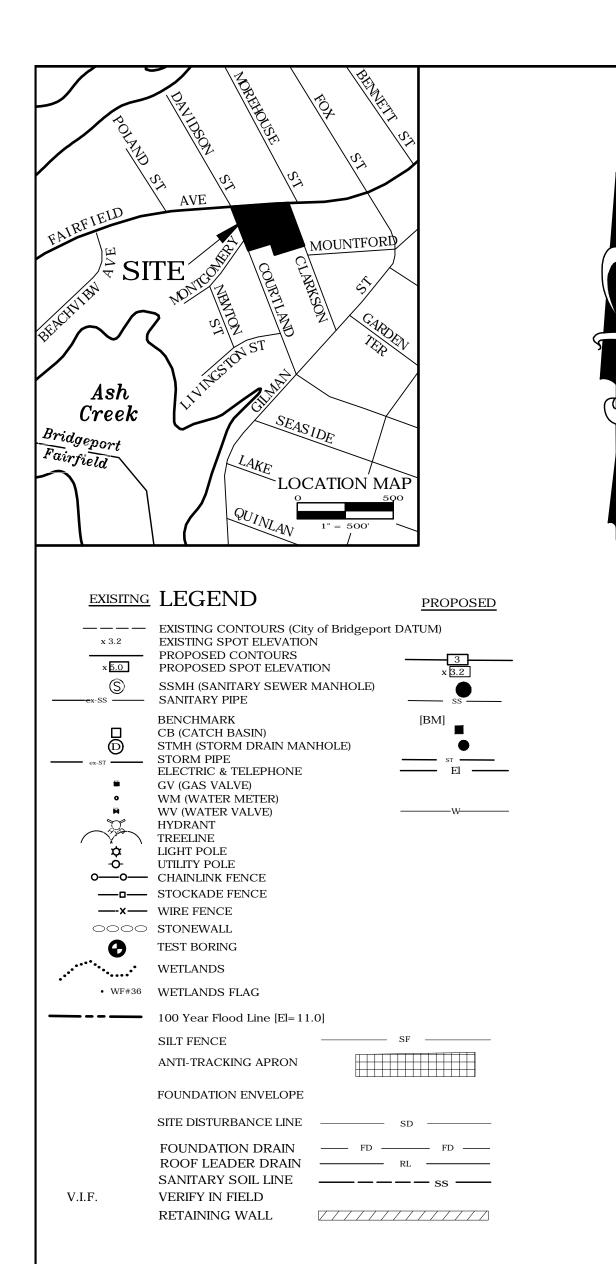
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	12	8-23-21	Modify Site Plan					١				
	11	12-17-19	CT DOT Comments 12-17-19		MAGNICO CONTRACTING #3125 FAIRFIELD AVE							
	10	11-22-19	Underground Utility Info Added									
	9	10-16-19	CT DOT Comments 10-8-19									
	8	9-18-19	State of CT comments		BRIDGEPC	DRT, CONN	ECTICUT					
	7	10-28-14	revise parking & details									
	6	9-08-14	additional landscaping		20	0 1" 20' 20	40					
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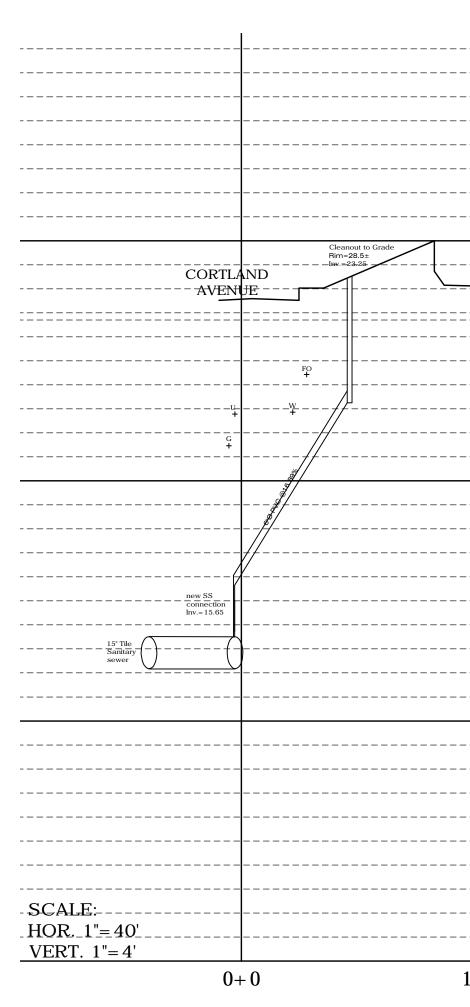
implemented and officially recorded.

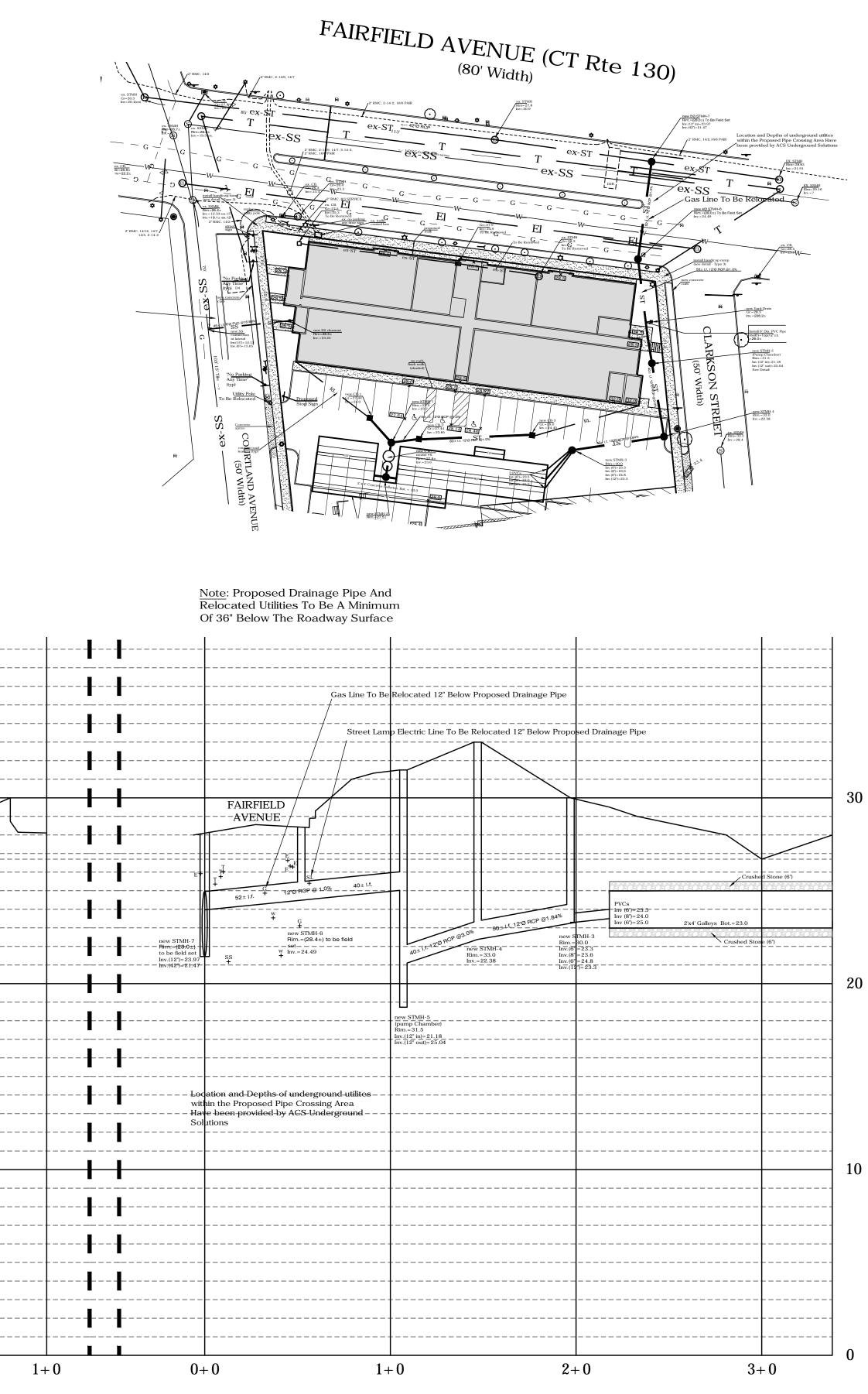
monthly

- conducted for floating or surface debris or sediment.
- once a year during the month of April and at other times as necessary to prevent the discharge of pollutants from structures or outfalls. (except roofs) shall be swept clean of sand, litter and other possible
- December 15 (after leaf fall) and once during the month of April (after snow melt) and at other times as directed by the Town of Wilton.
- gallery systems for any siltation or sedimentation.
- manner.









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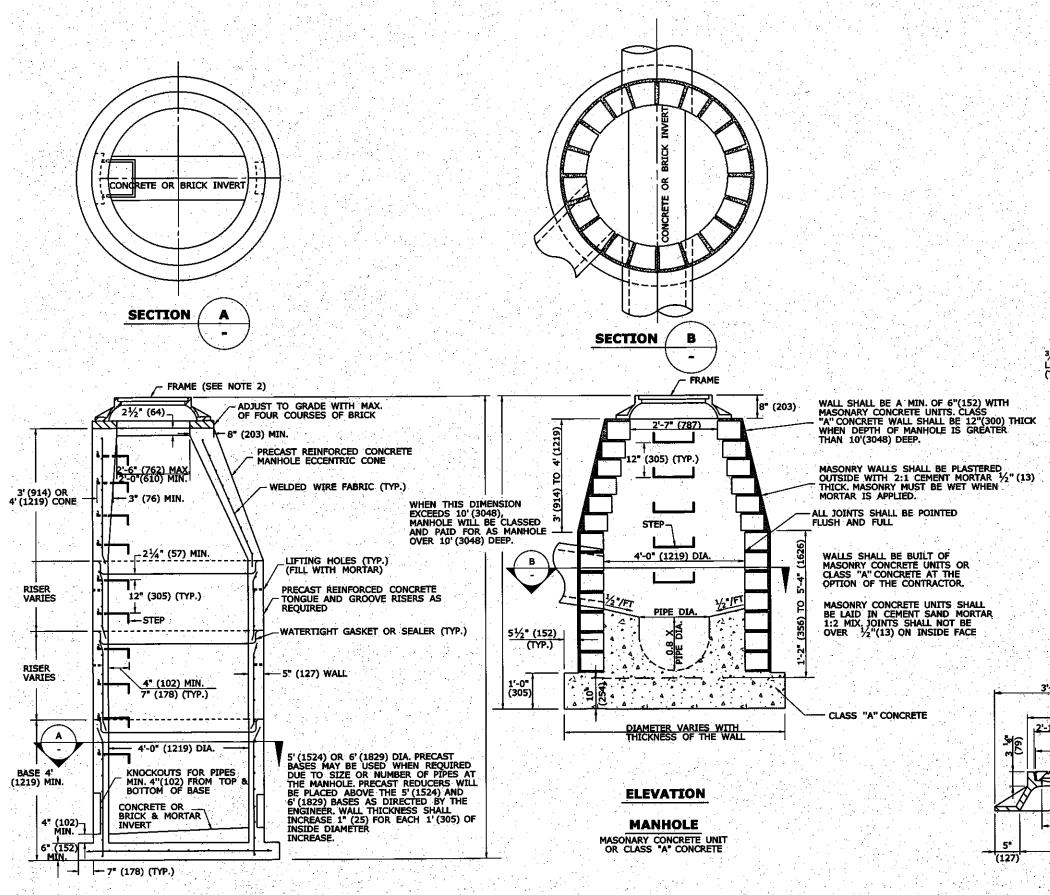
#### NOTE:

1. The underground utilities shown, if any, have been located from visible field survey information. The surveyor makes no guarantees that the underground utilities shown comprise all such utilities in the area either in service or abandoned. The surveyor further does not warrant that the underground utilities shown are in the exact location indicated. The surveyor has not physically located the underground utilities, unless specifically noted as such. It is the Contractor's responsibility to contact CALL BEFORE YOU DIG (CBYD) prior to commencement of any excavation, Dial 811 or 1-800-922-4455.

Location and Depths of underground utilites within the Proposed Pipe Crossing Area Have been provided by ACS Underground Solutions

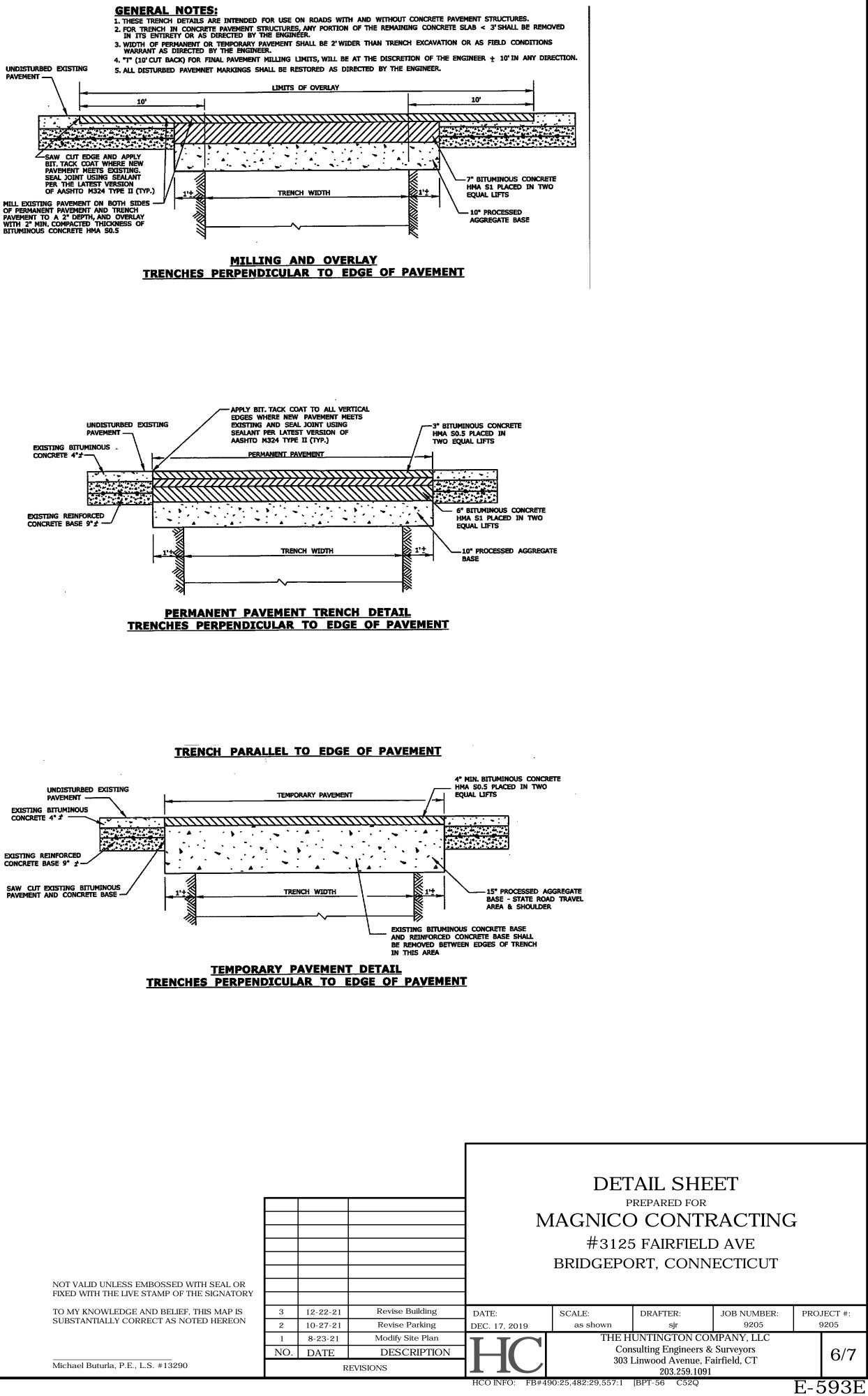
 Underground traffic control features shown per map entitled: "State Of Connecticut Department Of Transportation Burea Of Engineering & Hwy. Operations Division Of Traffic Engineering, Traffic Control Signal Layout, City Of Bridgeport, Route 130 (Fairfield Ave.) At Davidson Street And Cortland Ave.; Scale: 1"=40' ". Traffic Control Signal Plan For Intersection 015-341.

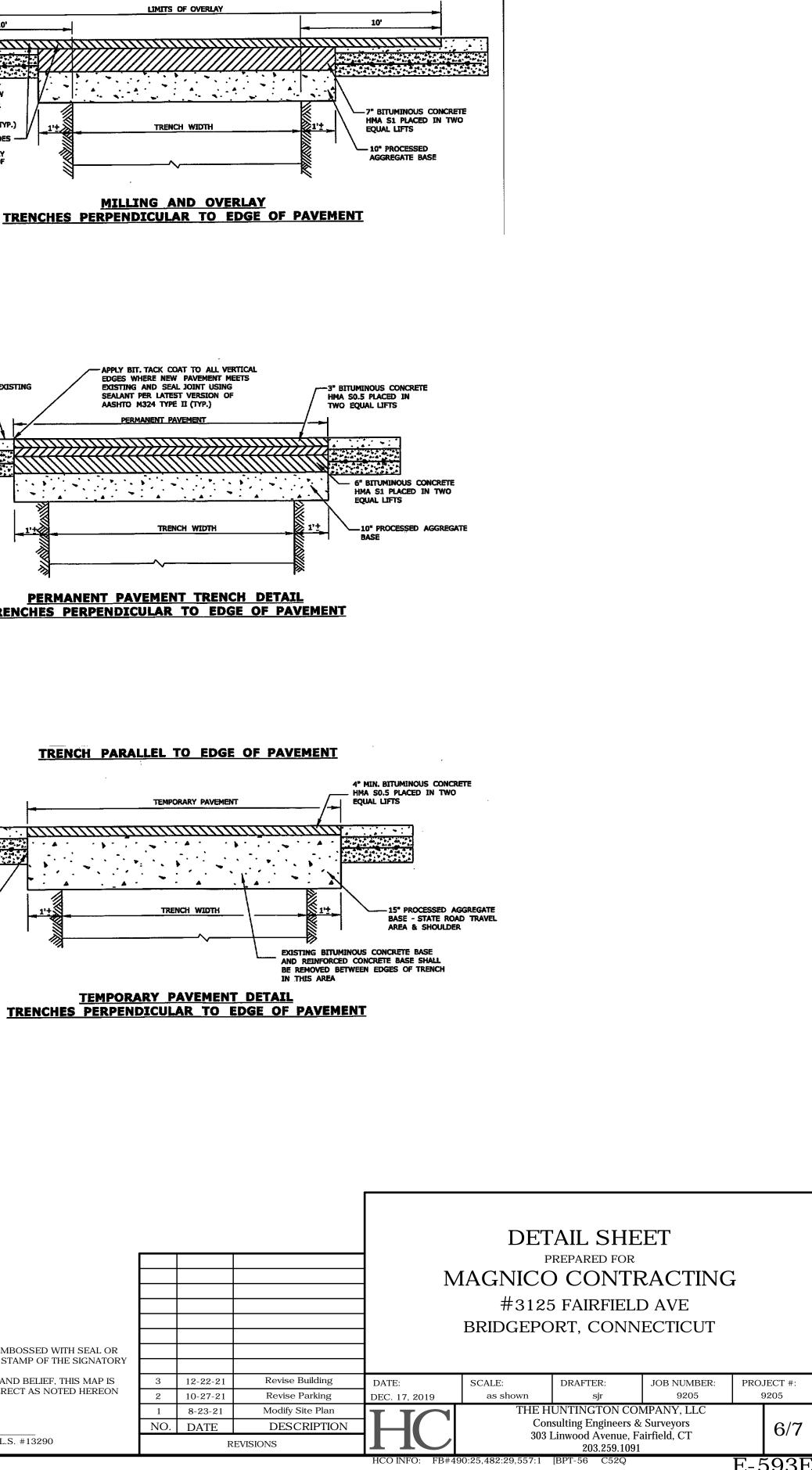
	14	12-22-21	Revise Building		PLAN	V & PROI	FILE				
	13	10-27-21	Revise Parking		F	PREPARED FOR					
	12	8-23-21	Modify Site Plan	Ν		) CONTE		۲			
	11	12-17-19	CT DOT Comments 12-10-19	1 <b>V</b> .	IAGNICC	JUUNIF		T			
	10	11-22-19	Underground Utility Info Added		#3125 FAIRFIELD AVENUE						
	9	10-16-19	CT DOT Comments 10-8-19								
	8	9-18-19	State of CT comments	l I	BRIDGEPC	ORT, CONN	ECTICUT				
	7	10-28-14	revise parking & details								
EAL OR	6	9-08-14	additional landscaping		40	0 40	80				
EAL OR GNATORY	5	8-18-14	RC zoning table			0 1" = 40' 40					
MAP IS	4	7-31-14	rev. parking & bldg.								
EREON	3	6-01-14	rev. parking & lot	DATE:	SCALE:	DRAFTER:	JOB NUMBER:	PROJ	JECT #:		
	2	5-28-14	rev. parking & lot	JAN. 9, 2014	1"=40'	whj	9205	ę	9205		
	1	1-22-14	zoning table	TT		IUNTINGTON CON					
	NO.	DATE	DESCRIPTION			nsulting Engineers &			7/7		
		F	REVISIONS		303	Linwood Avenue, Fa 203.259.1091	airfield, C I		• • •		
				HCO INFO: FB#49	0:25,482:29,557:1	[BPT-56 C52Q		<u><u> </u></u>	593D		

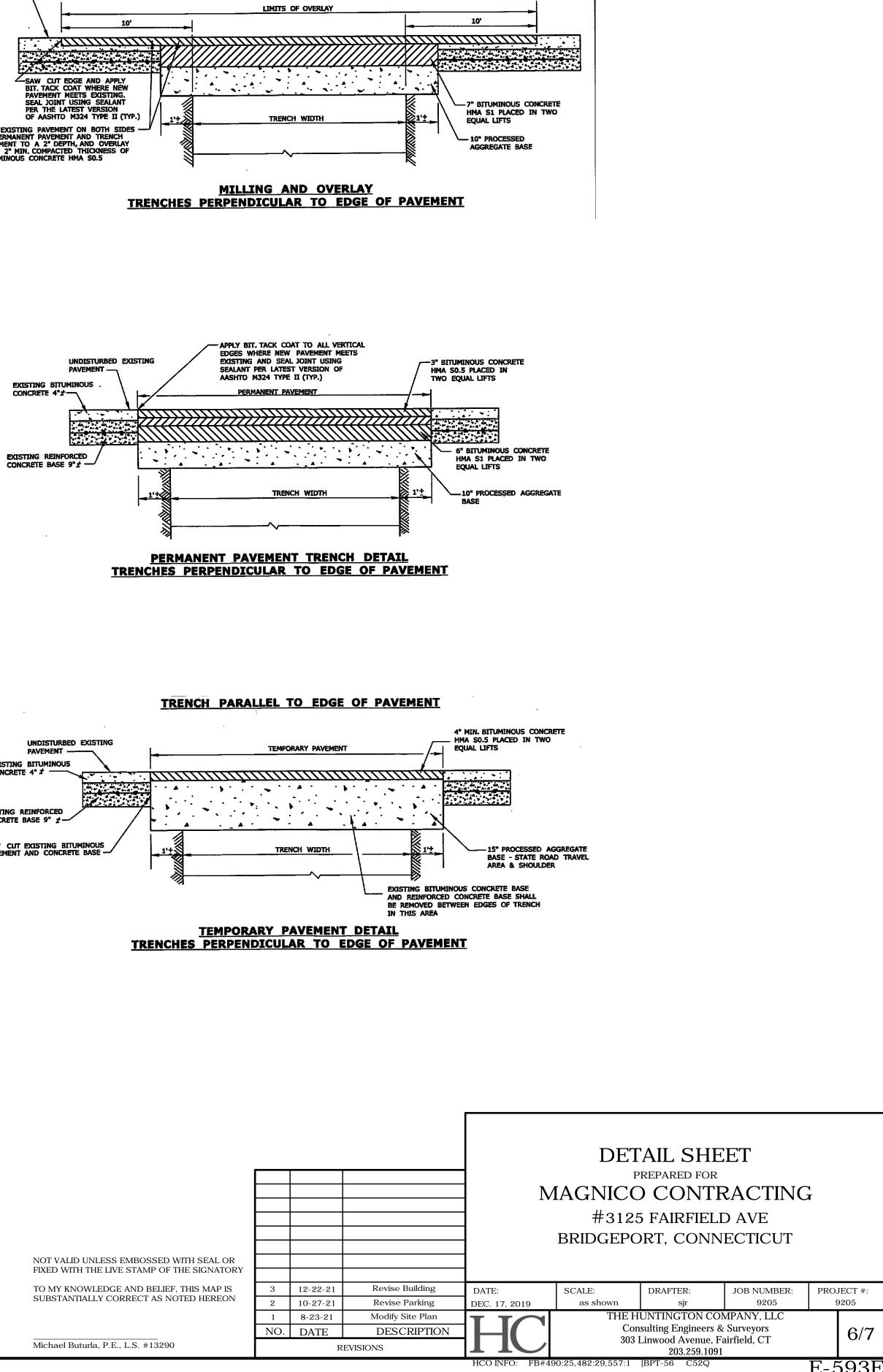


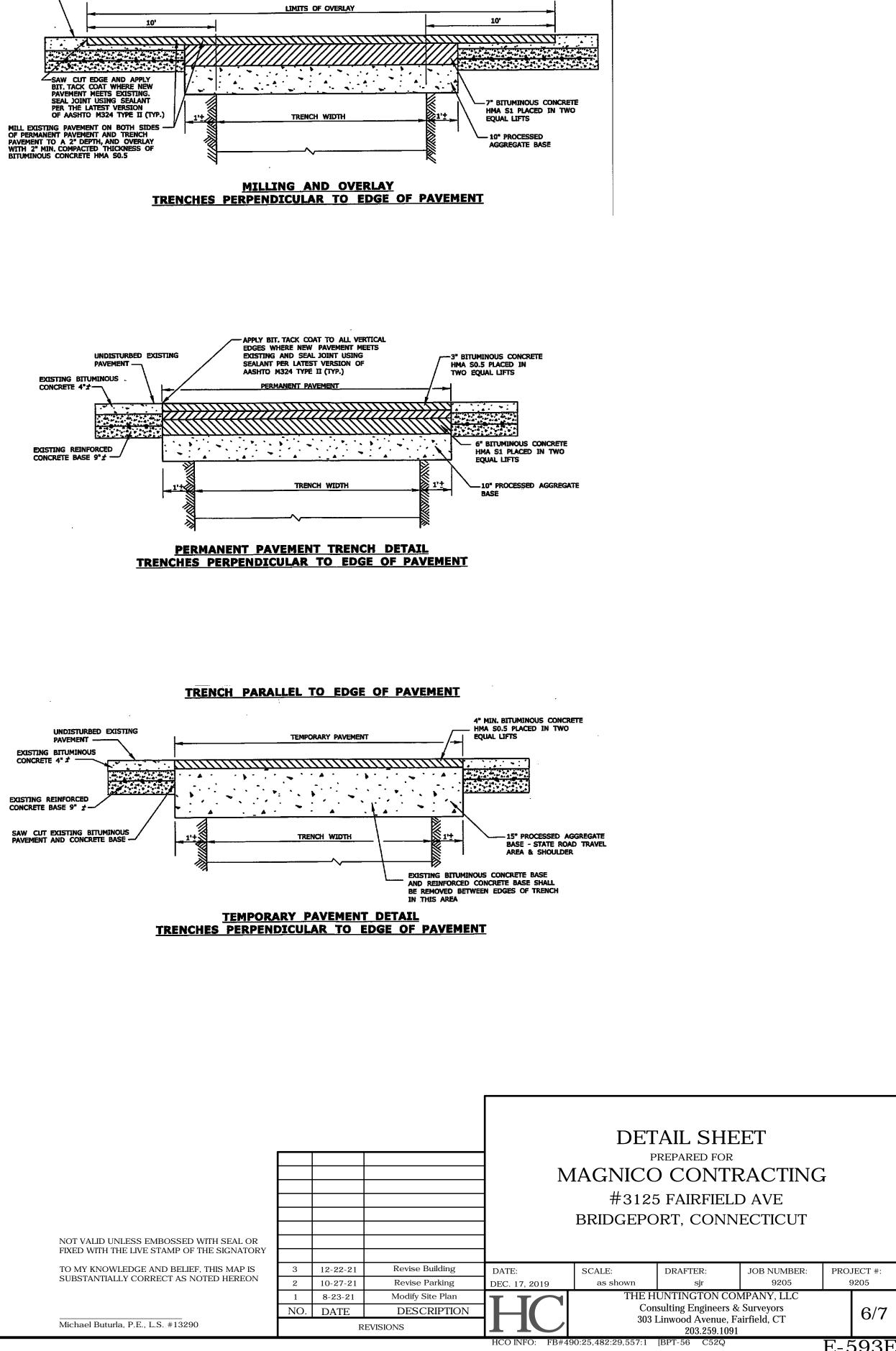
ELEVATION MANHOLE

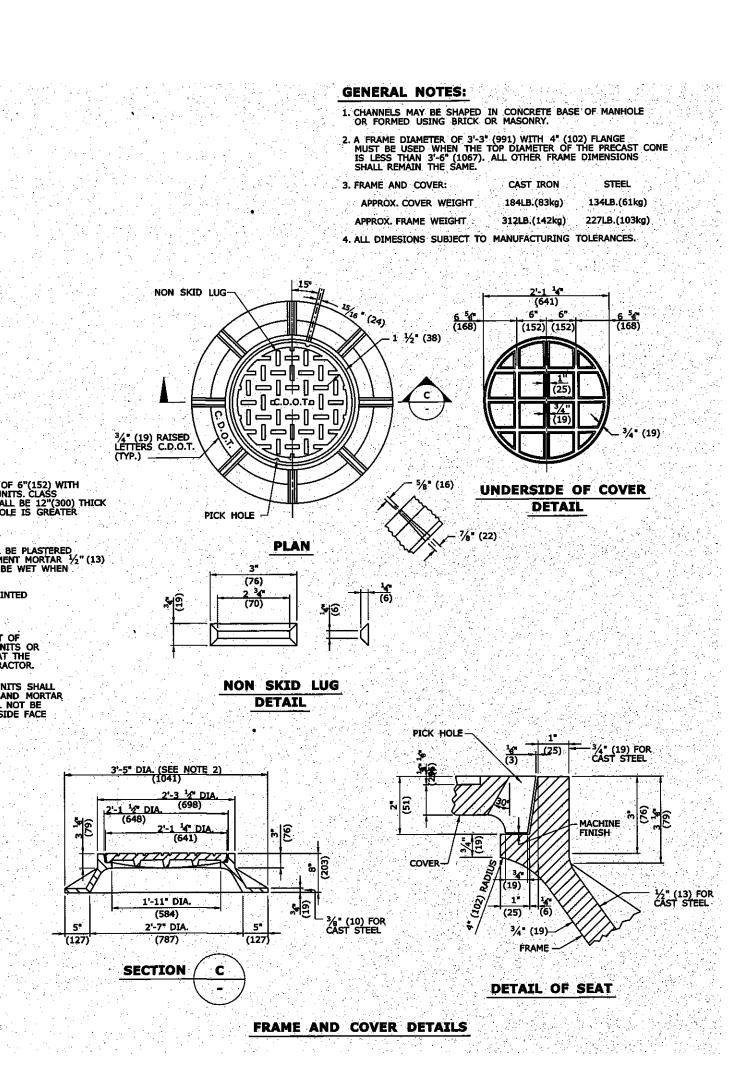
REINFORCED PRECAST CONCRETE UNIT

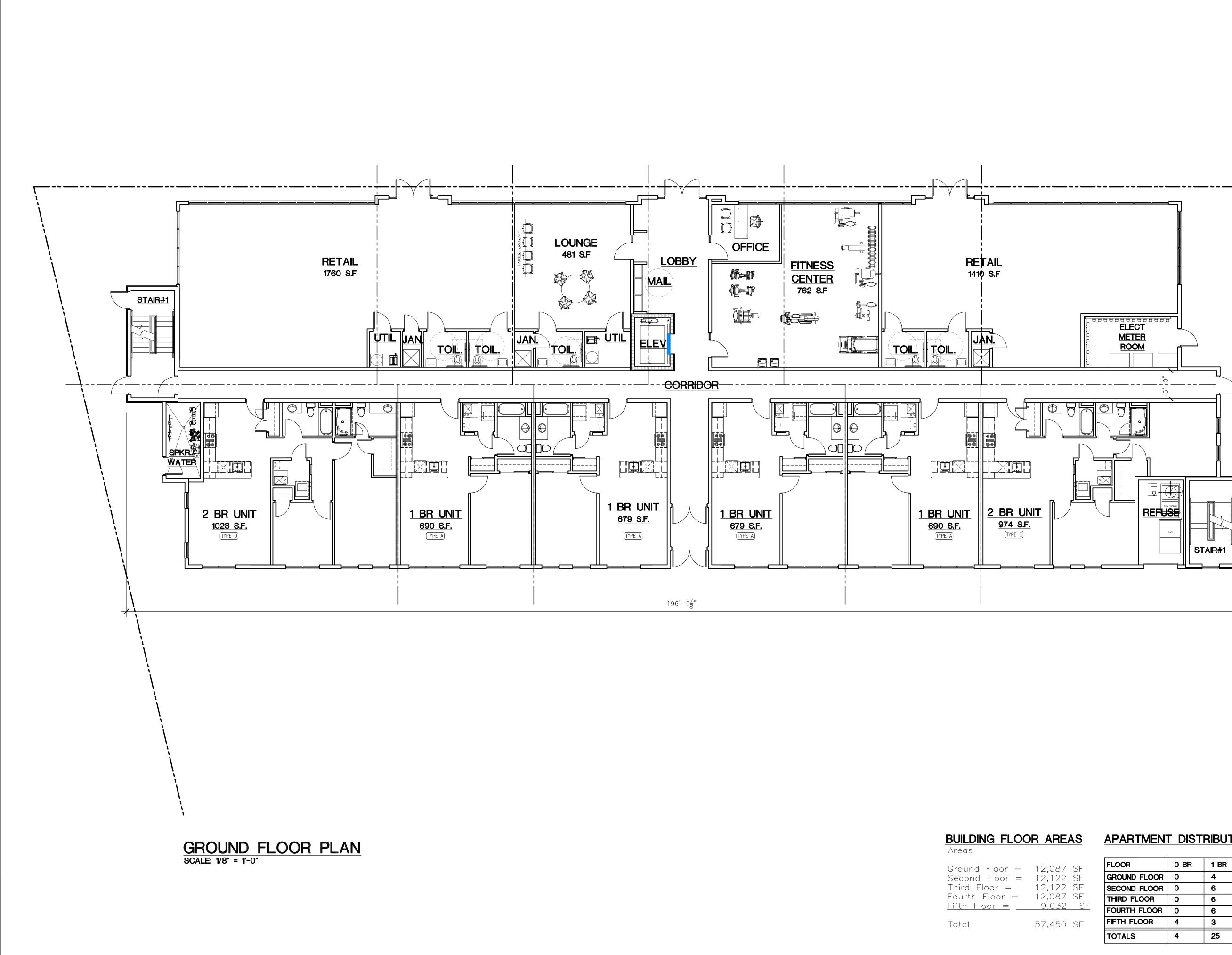












#### **BUILDING FLOOR AREAS** APARTMENT DISTRIBUTION

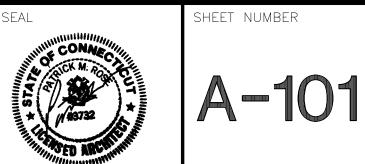
Г							
W1 3 5	Resentwood Avenue, Fairfield, ct 06825 berentwood Avenue, Fairfield, ct 06825 ct: (203)610-6262 • FAX: (203)610-6404						
NO. 1 2 3 4	REVISIONSBYDATEDESCRIPTIONMF8–10–21CLIENT REVIEWMF8–23–21ZONING SUBMISSIONMF10–27–21REVISE MATERIALSMF12–2–21REVISED BUILDING						
5	MF       12-29-21       ZONING SUBMISSION         Image: Submission state s						
PR	PROJECT TITLE MIXED-USE BUILDING						
	3115-3129 FAIRFIELD AVE. BRIDGEPORT, CT						



MAGNICO CONTRACTING 276 S. HOPE CHAPEL ROAD JACKSON, NJ 08527

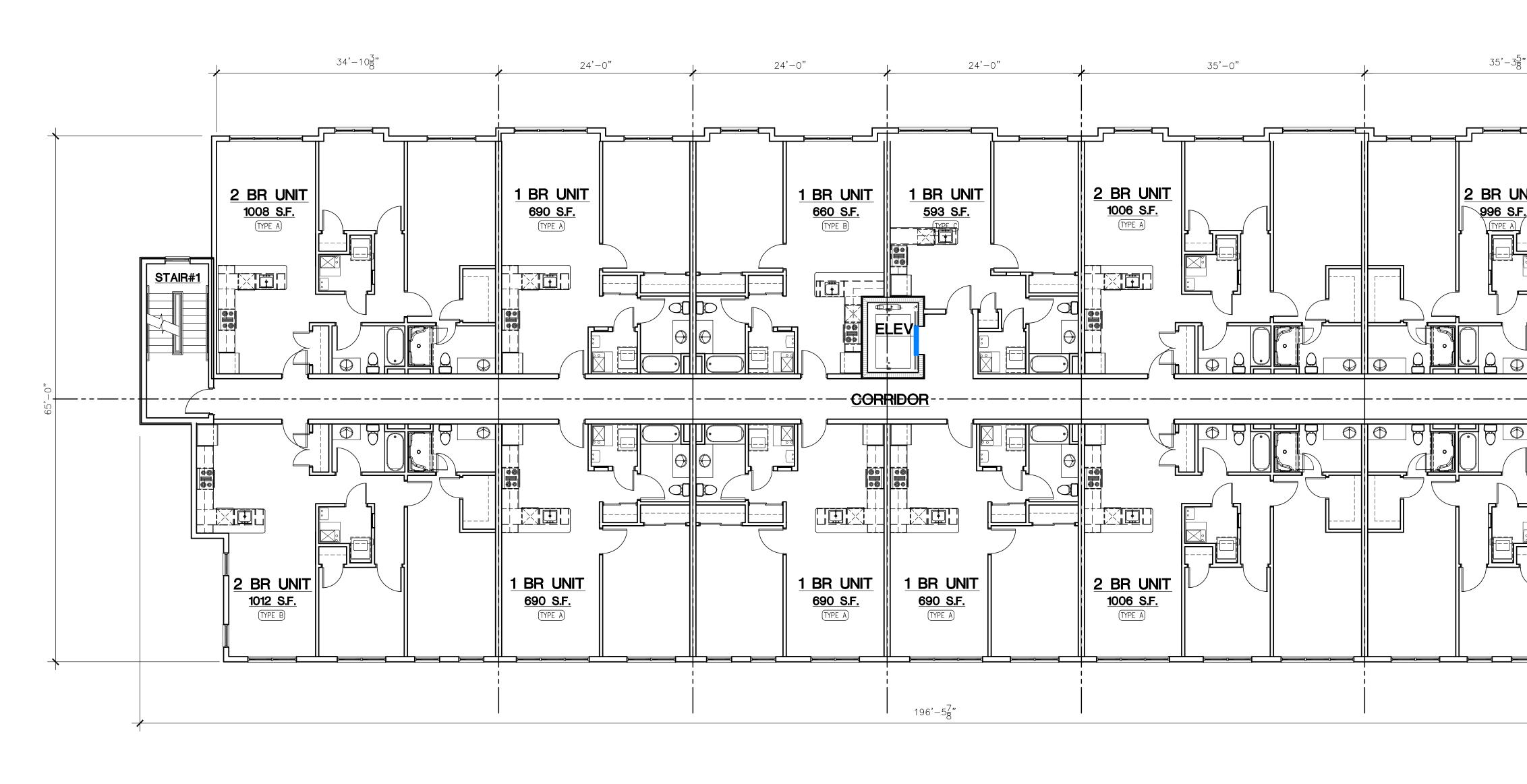
SHEET TITLE FIRST FLOOR PLAN

DESIGNED BY: MMF	SCALE: AS NOTED
DRAWN BY: MMF	DATE: 8-10-21
CHECKED BY: PMR	PROJECT NUMBER: 2613
CAD FILE: R:/2613/ARCH	



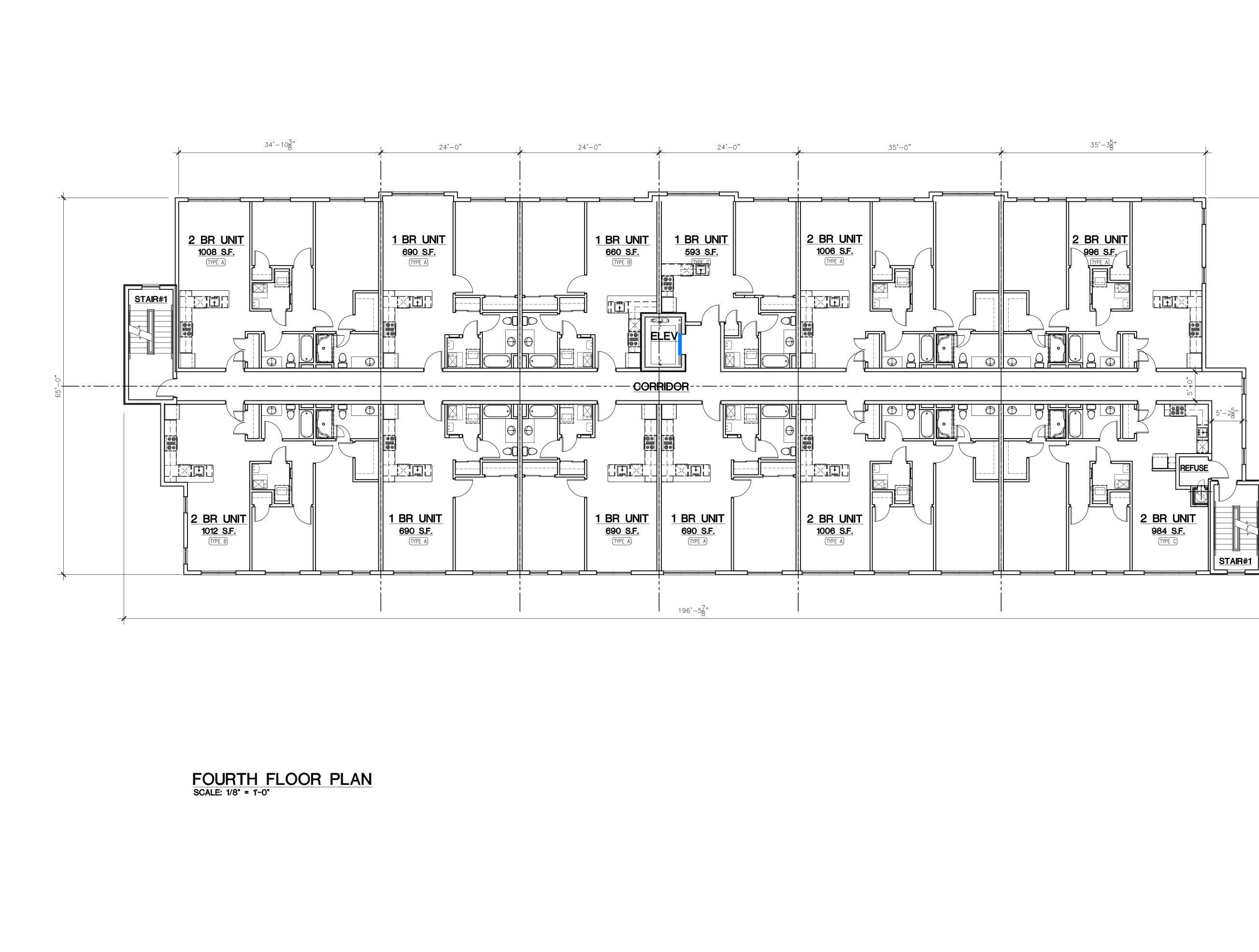
<u>ELECT</u> METER ROOM REFUSE **STAIR#1** 

7 SF	FLOOR	0 BR	1 BR	2 BR	TOTALS
2 SF	GROUND FLOOR	0	4	2	6
2 SF	SECOND FLOOR	0	6	6	12
'SF	THIRD FLOOR	0	6	6	12
<u>2 SF</u>	FOURTH FLOOR	0	6	6	12
) SF	FIFTH FLOOR	4	3	3	10
	TOTALS	4	25	23	52

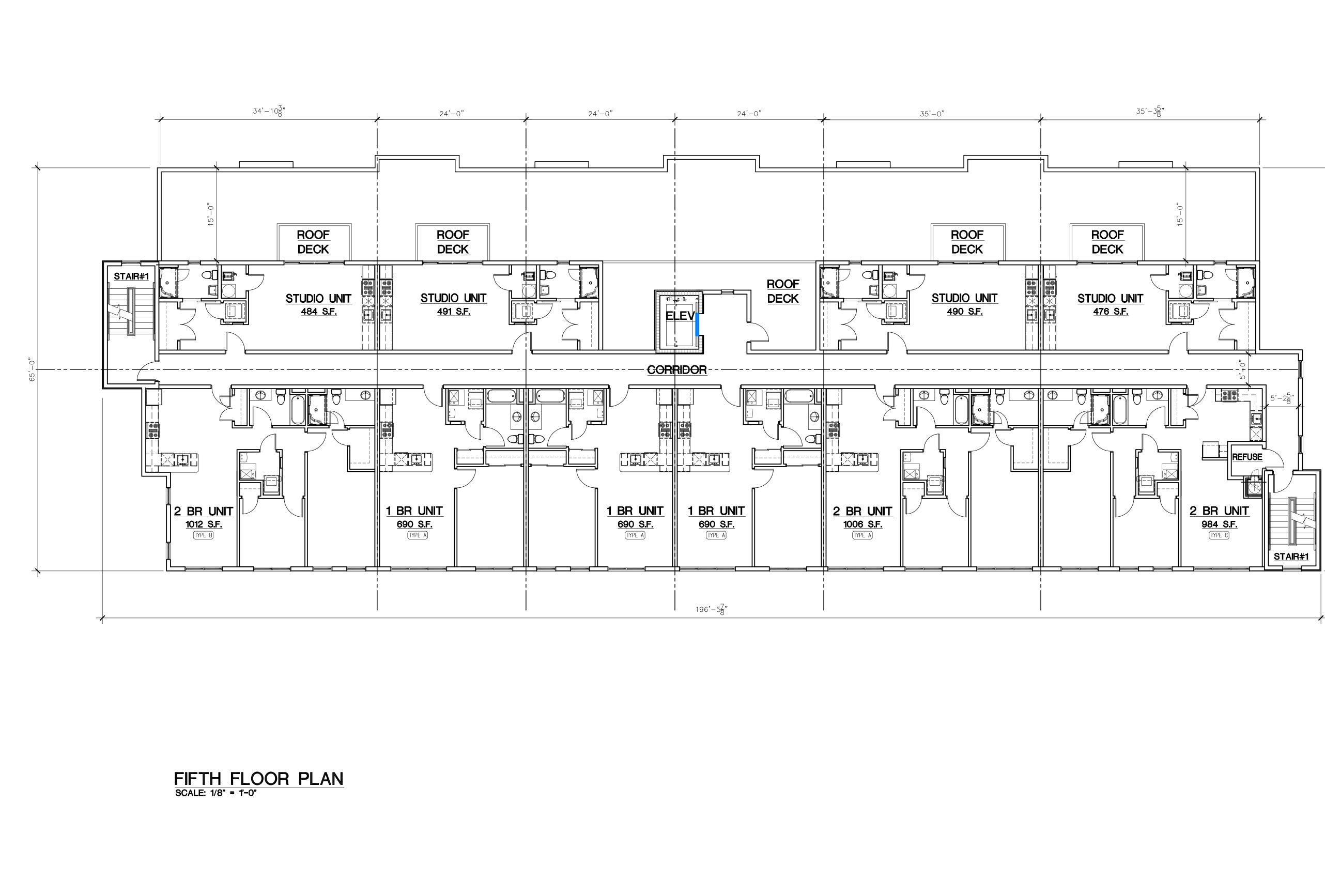




	WWW.RDSETISD.CDM         35 BRENTWOOD AVENUE, FAIRFIELD, CT 06825         TEL: (203)610-6262 • FAX: (203)610-6404
	REVISIONSNO.BYDATEDESCRIPTION1MF8–10–21CLIENT REVIEW2MF8–23–21ZONING SUBMISSION3MF10–27–21REVISE MATERIALS4MF12–2–21REVISED BUILDING5MF12–29–21ZONING SUBMISSION4M12–29–21ZONING SUBMISSION5MF12–29–21ZONING SUBMISSION6M117M1181191191191191191191191191191191191911919119 <t< th=""></t<>
Image: Constraint of the second s	PROJECT TITLE MIXED-USE BUILDING 3115-3129 FAIRFIELD AVE. BRIDGEPORT, CT
	Prepared For: MAGNICO CONTRACTING 276 S. HOPE CHAPEL ROAD JACKSON, NJ 08527
	DESIGNED BY: MMF DRAWN BY: MMF CHECKED BY: PMR CHECKED BY: PMR PROJECT NUMBER: 2613 CAD FILE: R:/2613/ARCH SEAL SHEET NUMBER A-1022



	<image/> <section-header><text></text></section-header>
	REVISIONS         NO.       BY       DATE       DESCRIPTION         1       MF       8–10–21       CLIENT REVIEW         2       MF       8–23–21       ZONING SUBMISSION         3       MF       10–27–21       REVISE MATERIALS         4       MF       12–2–21       REVISED BUILDING         5       MF       12–29–21       ZONING SUBMISSION         4       MF       12–29–21       ZONING SUBMISSION         5       MF       12–10       1         4       MF       12–10       1         5       MF       12–10       1         6       MF       12–10       1         6       MF       12–10       1         7       MF       12–10       1         8       MF       12–10       1         9       MF       12–10       1         10       1       1       1         11       1       1       1         12       1       1       1         13       1       1       1         14       1       1       1         15       1
5'-25"	MIXED-USE BUILDING 3115-3129 FAIRFIELD AVE. BRIDGEPORT, CT
	Prepared For: MAGNICO CONTRACTING 276 S. HOPE CHAPEL ROAD JACKSON, NJ 08527
	<b>FIFTH FLOOR PLAN</b> DESIGNED BY: MMFSCALE: AS NOTEDDRAWN BY: MMFDATE: 8-10-21CHECKED BY: PMRPROJECT NUMBER: 2613
	CAD FILE: R:/2613/ARCH SEAL SHEET NUMBER A 103



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REVISIONSNO.BYDATEDESCRIPTION1MF8-10-21CLIENT REVIEW2MF8-23-21ZONING SUBMISSION3MF10-27-21REVISE MATERIALS4MF12-2-21REVISED BUILDING5MF12-29-21ZONING SUBMISSION4MF12-20-21REVISED BUILDING5MF12-20-21ZONING SUBMISSION4MF12-20-21ZONING SUBMISSION5MF12-20-21ZONING SUBMISSION6MF12-20-21ZONING SUBMISSION7MMM8MMM9MM9MM9MM9MM9MM9MM9MM9M9MM9M9MM9M9MM9M<
PROJECT TITLE     PROJECT TITLE     MIXED-USE     BUILDING     3115-3129 FAIRFIELD AVE.   BRIDGEPORT, CT
Prepared For: MAGNICO CONTRACTING 276 S. HOPE CHAPEL ROAD JACKSON, NJ 08527
DESIGNED BY: MMFSCALE: AS NOTEDDRAWN BY: MMFDATE: 8-10-21CHECKED BY: PMRPROJECT NUMBER: 2613CAD FILE: R:/2613/ARCHSHEET NUMBERSEALSHEET NUMBERImage: Comparison of the second of the





			REVISIONS
NO.	BY	DATE	DESCRIPTION
	MF	8-10-21	CLIENT REVIEW
1		0 07 04	ZONING SUBMISSION
1	MF	8-23-21	
	MF MF	8-23-21	REVISE MATERIALS
2			REVISE MATERIALS REVISED BUILDING

PROJECT TITLE

# MIXED-USE BUILDING

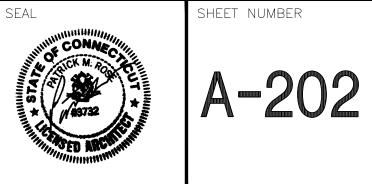
# 3115-3129 FAIRFIELD AVE. BRIDGEPORT, CT

Prepared For:

MAGNICO CONTRACTING 276 S. HOPE CHAPEL ROAD JACKSON, NJ 08527

# SHEET TITLE EXTERIOR ELEVATIONS

DESIGNED BY: MMF	SCALE: AS NOTED
DRAWN BY: MMF	DATE: 8-10-21
CHECKED BY: PMR	PROJECT NUMBER: 2613
CAD FILE: R:/2613/ARCH	



SHEET NUMBER

	File No.
	PETITION TO THE PLANNING & ZONING COMMISSION CITY OF BRIDGEPORT, CONNECTICUT
1.	NAME OF PETITIONER:ALPA SHAH
2.	Is the Petitioner's name Trustee of Record? Yes NoX
	If yes, a sworn statement disclosing the Beneficiary shall accompany this application upon filing. Address of Property: 3171 FAIRFIELD AVE CT 06605
	(number) (street) (state) (zip code)
4.	Assessor's Map Information: Block No. 106 Lot No. 13 A
5.	Amendments to Zoning Regulations: (indicate) Article: Liquor Control Section: 12 - 10
	(Attach copies of Amendment)
6.	Description of Property (Metes & Bounds):
7.	
8.	Existing Zone Classification: <u>Restaurant</u>
	Zone Classification requested: Liquor Store
5.	Describe Proposed Development of Property: Retail Store (Liquor, wine
	Beer).
	Approval(s) requested: Change of the from Restaurant
	to Liquor Store.
	Signature: Shah Jalpa R. Date: 10/29/21
	Print Name: Jalpa Shah
	f signed by Agent, state capacity (Lawyer, Developer, etc.) Signature:
	Print Name:
	Aailing Address:
	Phone: Cell: Fax:
ł	-mail Address:
9	Fee received Date: Clerk:
	THIS PETITION MUST BE SUBMITTED IN PERSON AND WITH COMPLETED CHECKLIST
VE	
	Completed Site / Landscape Plan
LE	Written Statement of Development of Development
-	Cert. of Incorporation & Organization and First Report (Corporations & LLC's)
_	Scolu Cappel And Owner's Signature Date
	Print Owner's Name Owner's Signature Date
R	ev. 6/18/2016

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January 25, 2022 City of Bridgeport Planning and Zoning

**RE: Package Store Application** 

Dear Sir/Madam,

We would like to request zoning approval for package store at 3171 Fairfield Ave, Bridgeport, CT 06605.

5 K.,

Sincerely

Shain Jalpa R.

Jalpa Shah 203-502-9479 Ravish1511@gmail.com ETIENNE DEAN & JEAN 000008 MONTGOMERY ST BRIDGEPORT, CT 06605

TOMASIO MIGUEL A & ROSEMARIE 003170 FAIRFIELD AVE BRIDGEPORT, CT 06605

NEMETZ ANTHONY J 143 BAROS ST FAIRFIELD, CT 06430

704 COURTLAND AVENUE LLC 704 COURTLAND AVENUE BRIDGEPORT, CT 06605

3171 FAIRFIELD AVENUE LLC 003255 FAIRFIELD AVE BRIDGEPORT, CT 06605 OPENSHAW KENNETH 25 DALE RD TRUMBULL, CT 06611

FORMATO JOSEPH 3870 BLACK ROCK TPK FAIRFIELD, CT 06825

GREGA THOMAS E. & TIMOTHY 38 MONTGOMERY STREET BRIDGEPORT, CT 06605

PEKAR MARGE 000679 COURTLAND AVE BRIDGEPORT, CT 06605

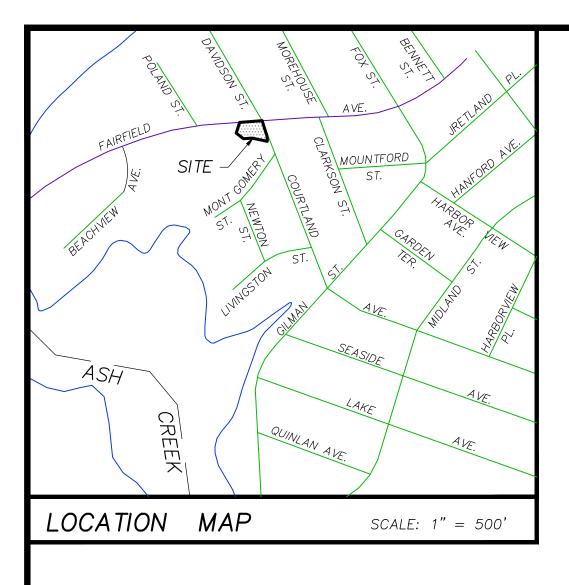
T C M REALTY COMPANY LLC 003255 FAIRFIELD AVE BRIDGEPORT, CT 06605 BASJAH JOHN & FORTUNATA 694 COURTLAND AVE BRIDGEPORT, CT 06605

DEPIANO LYNDA 102 JAMES ST FAIRFIELD, CT 06430

FORMATO JOSEPH 3870 BLACK ROCK TPK FAIRFIELD, CT 06825

3115 FAIRFIELD AVENUE LLC 3135 FAIRFIELD AVENUE BRIDGEPORT, CT 06605

Property Owner's List



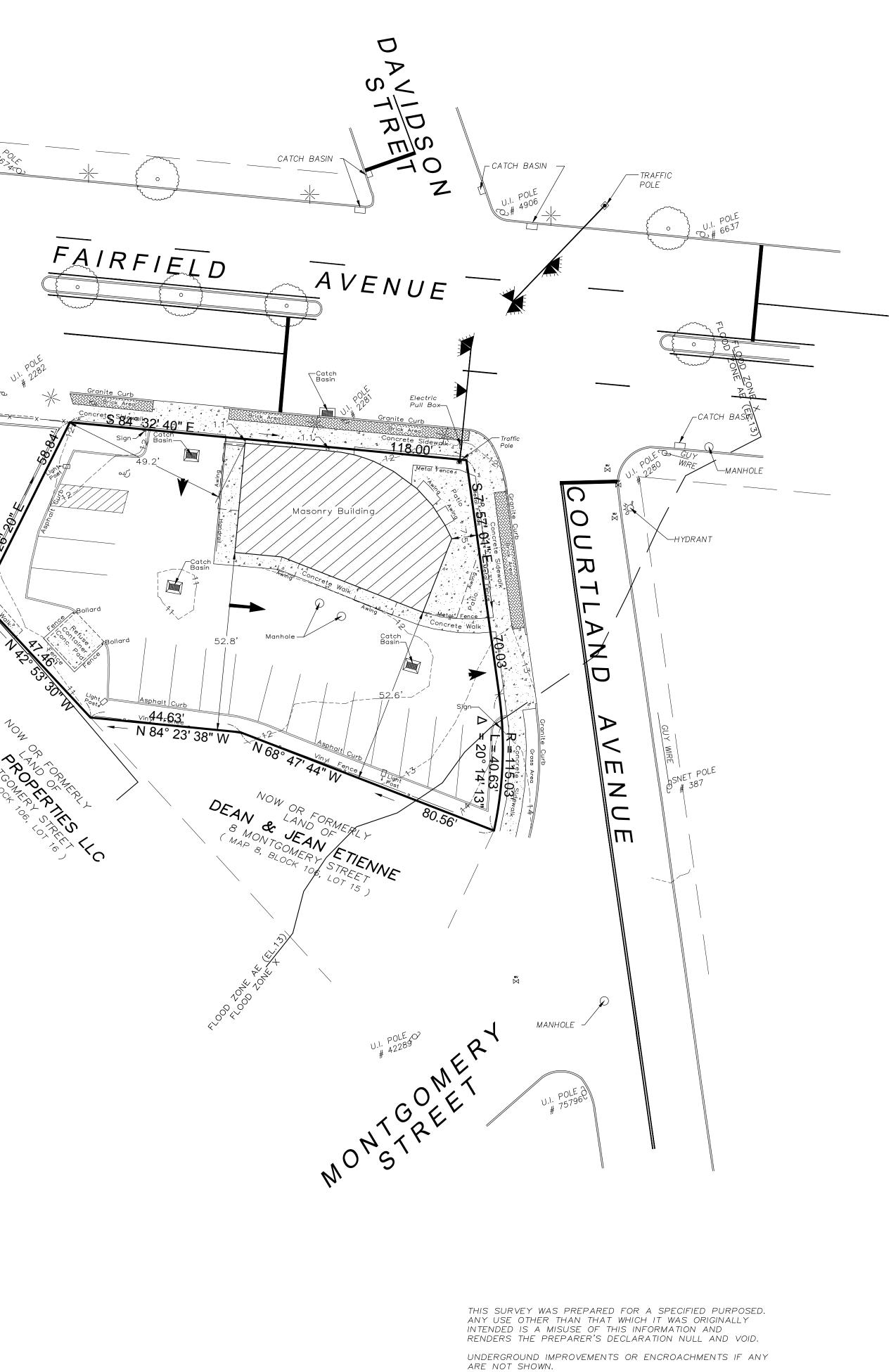


- 1) THIS SURVEY AND MAP HAS BEEN PREPARED IN ACCORDANCE WITH THE REGULATIONS OF CONNECTICUT STATE AGENCIES, SECTIONS 20–300b–1 THROUGH 20–300b–20, AND THE "MINIMUM STANDARDS FOR SURVEYS AND MAPS IN THE STATE OF CONNECTICUT" AS ADOPTED BY THE CONNECTICUT ASSOCIATION OF LAND SURVEYORS, INC., ON SEPTEMBER 26, 1996.
- 2) THE TYPE OF SURVEY PERFORMED IS A BUILDING LOCATION MAP, ZONING. THE MAIN PURPOSE OF THIS SURVEY IS TO SHOW THE INFORMATION NECESSARY TO ENABLE DETERMINATION OF COMPLIANCE OR NON-COMPLIANCE WITH THE APPLICABLE ZONING SETBACK REQUIREMENTS. AS SUCH, IT MAY BE NECESSARY TO DEPICT ONLY A PORTION OF THE PROPERTY. THE ONLY IMPROVEMENT THAT NEED BE SHOWN IS THE EXISTING OR PROPOSED BUILDING RELATING TO THE MATTER OF ZONING COMPLIANCE BEING ADDRESSED.
- 3) THE BOUNDARY DETERMINATION IS BASED UPON THE DEPENDENT RESURVEY METHOD.
- THE SURVEY CONFORMS TO HORIZONTAL CLASS A-2 ACCURACY STANDARDS. VERTICAL DATA CONFORMS TO A CLASS T-2 STANDARDS, REFERS TO 4) NAVD '88 DATUM.
- 5) THE SUBJECT PROPERTY IS SHOWN AS TAX LOT 13A, BLOCK 106 ON ASSESSOR'S MAP 8.
- 6) THE SUBJECT PROPERTY IS LOCATED IN 'ORS' ZONE. 7) THE SUBJECT PROPERTY IS OWNED BY 3171 FAIRFIELD AVENUE LLC REFER TO RECORD DEED VOL. 6238, PG. 198 ON FILE IN
- THE OFFICE OF THE BRIDGEPORT TOWN CLERK.
- 9) THE SUBJECT PROPERTY IS LOCATED IN FLOOD ZONE AE (EL 13) AS SHOWN ON FEMA MAP ENTITLED "FIRM FLOOD INSURANCE RATE MAP FAIRFIELD COUNTY, CONNECTICUT ( ALL JURISDICTIONS) PANEL 438 OF 626 CONTAINS: COMMUNITY CITY OF BRIDGEPORT, NUMBER 090002, PANEL 0438, MAP NUMBER 09001C0438G, MAP REVISED JULY 8, 2013".

MAP REFERENCE:

RECORD MAP VOL. 50 PG. 217, B.L.R. ENTITLED "TOPOGRAPHICAL SURVEY OF PROPERTY IN BRIDGEPORT, CONN. FOR: THE THE ARNOLD CO. INC. SCALE 1" = 20 DATE DECEMBER 18, 1985" BY THE FULLER & COMPANY INC. FILE APRIL 27, 1987.

POUL P - 101 P - 101 P - 101 P - 101 P



THE SURVEY AND DECLARATION SHOWN HEREON IS NULL AND VOID WITHOUT THE LICENSED SURVEYORS LIVE SIGNATURE OR EMBOSSED SEAL.

RENDER'S ANY DECLARATION SHOWN HEREON NULL AND VOID.

UNAUTHORIZED ALTERATIONS OR ADDITIONS TO THIS SURVEY, WHICH BEARS THE SURVEYORS STAMP OR SEAL,

EMBOSSED SEAL

THE DECLARATION SHOWN RUNS TO THE PERSON, OR PERSONS

FOR WHOM THE SURVEY WAS PREPARED FOR. THE DECLARATION IS NOT TRANSFERABLE.

<u>AREA = 12,645 SQ. FT.</u> \_or 0.2902 AC.

			1		
DEVELOPMENT STANDARDS	OR Z		Existing Conditions	Proposed Conditions	As-built Condi
LOT	Minimum	Maximum			
Lot width	35 feet	None	118.0 feet		
Lot depth	None	None	None		
Lot area	5,000 sf	None	12,645 sf		
Lot coverage	75 %	None	85.95 %		
STREET WALL			]		
As a percent of frontage (Primary)	75 %	100 %	61.02 %		
As a percent of frontage (Secondary)	30 %	100 %	21.19 %		
As a percent of frontage	N/A	N/A	N/A		
(Tertiary)	Note	e 8	Note 8		
Primary frontage	0' 0'	10' 10'	1.1' 7.5'		
FROM STREET LINE		10'	1 1 1'		
Secondary frontage	0'	10'	7.5'		
Tertiary frontage	N/A	N/A	N/A		
	Note 9	& 10	Note 9 & 10		
YARD			]		
Side yard (see Note 2)	O feet or 5 feet if side yard is utilized	1 foot for each floor of building height not to exceed 14 feet	49.2'		
Rear yard	O feet or 20 feet if floor contains habitable space	None	52.6'		
			,	·	
OTHER STANDARDS					
Landscaped area as a percent of lot	15 %	25 %	14.05 %		
Floor to ceiling height of first story	12 feet	None	_		
Public Access Easement	NOTE 3	NOTE 3	NOTE 3		
	NOTE	E 11	NOTE 11		

ZONING/IMPROVEMENT SURVEY MAP OF PROPERTY	
PREPARED FOR	
<u> </u>	
3171 FAIRFIELD AVENUE	
BRIDGEPORT, CONNECTICUT	
SCALE: 1" = 20' DECEMBER 14, 2021	
GRAPHIC SCALE 10 0 5 10 20 40	
WALTER H. SKIDD - LAND SURVEYOR LLC	OF CONVER
To the best of my knowledge and belief this map is substantially correct as noted hereon.	$\begin{array}{c} \mathbf{x} \\ $
WALTER H. SKIDD, L.S. Conn. Reg. # 14663 1992 STRATFIELD ROAD — FAIRFIELD, CONN. TELEPHONE (203) 373–0401	AND SURVEYOR

